## Contents

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Important: Version 0.5.8 is released!
Mac version is supported!
OpenSeesPy has been official in DesignSafe.
The latest version of this document can be found at https://openseespydoc.readthedocs.io/en/latest/.

Note: Please send any questions to github issues.
You are very welcome to contribute to OpenSeesPy with new command documents and examples by sending pull requests through github pulls.

OpenSeesPy is a Python 3 interpreter of OpenSees. A minimum script is shown below:

```python
# import OpenSeesPy
import openseespy.opensees as ops

# wipe model
ops.wipe()

# create model
ops.model('-ndm', ndm, '-ndf', ndf)
```

Most of OpenSeesPy commands have the same syntax and arguments as the OpenSees Tcl commands. The conversion from Tcl to Python is easy and straightforward as demonstrated with commands below.
1.1 Installation

1. **PyPi**
2. **DesignSafe**

1.1.1 PyPi

Install Python

- Install Anaconda

Install OpenSeesPy

- To install
  ```bash
  python -m pip install openseespy
  python -m pip install --user openseespy
  ```

- To upgrade
python -m pip install --upgrade openseespy
python -m pip install --user --upgrade openseespy

Import OpenSeesPy

```python
import openseespy.opensees as ops
```

1.1.2 DesignSafe

OpenSeesPy has been official in DesignSafe.

1. OpenSeesPy in DesignSafe
2. Paraview in DesignSafe

OpenSeesPy in DesignSafe

Follow steps below to run OpenSeesPy in DesignSafe.

Tip:

- Go To DesignSafe website and click register
• Register an account for free and log in

**Tip:**

• Land on your own portal and go to workspace
Tip:

- In your workspace, select Jupyter, launch it, and start my server
Tip:

- Now you should be in the Jupyter Notebook
- Go to mydata and always save your data under this folder
Tip:

- In mydata folder, select New and then Terminal

Tip:

- The version on DesignSafe is not the latest.
- To update to the latest:
  - Type `pip install --user --upgrade openseespy` in the terminal
  - Check the installation by running `ipython` and `import openseespy.opensees as ops`
  - Everytime you start a new session, you have to update openseespy (this is not ideal, but it works for now.
- Now you may close the terminal by closing the browser tab.
Tip:

- Back to mydata folder, select New and then Python 3

Tip:

- Now you can write OpenSeesPy script in Jupyter Notebook, run the script, and show results
- To show figure in the Notebook, you should include %matplotlib inline at the beginning
Paraview in DesignSafe

DesignSafe provides paraview for viewing OpenSeesPy results:

Tip:
- Make sure the steps in *OpenSeesPy in DesignSafe* are completed.

Tip:
- Go to *Workspace* and select *Visualization* and *Paraview*. 
Tip:

- In the Job Submission windows, you should select Working Directory, Maximum job runtime, Job name, and Node Count, and click Run.
Tip:

- You can see the job status on the right
Tip:

- Wait until see this windows and connect to Paraview
Your interactive session has started!

To connect to your interactive session, click the button below.
To end the job, quit the application (e.g. MATLAB) within the session.
Your files may take some time to appear in your archive directory after the job has completed.

Tip:

- Now open the pvd file in Paraview
Tip:

- Initially, you see nothing
- Check the Pipeline Browser
- Click on the eye left to the file in Pipeline Browser

Tip:

- Change the Solid Color to other variables
- Change Surface to Surface With Edges
1.2 Compilation

Following is the OpenSees Compilation using QT by

Stevan Gavrilovic github

PhD Candidate
University of British Columbia

Original instructions can be found at here.

1.2.1 A Qt build environment for OpenSees

OpenSeesQt

A Qt build environment for OpenSees – Open System For Earthquake Engineering Simulation Pacific Earthquake Engineering Research Center(http://opensees.berkeley.edu).

Qt is an open source, cross-platform development environment that is free for many uses. Please see the license.
1.2.2 Dependency

The purpose of this project is to create a package that will allow aspiring developers to get started on writing code without having to worry about the compilation environment. A program as large as OpenSees relies on many third-party libraries, often referred to as dependencies. It can be a daunting task assembling, compiling, and linking these libraries. Many times, these libraries depend on other libraries, and so on. The current list of dependencies includes:

- MUMPS (5.1.2)
- Scalapack (2.0.2.13)
- UMFPACK (5.7.7) contained in Suite-Sparse (5.3.0)
- SUPERLU (5.2.1)
- SUPERLUMT (3.0)
- SUPERLUDIST (5.1.0)
- Openblas (0.3.5)
- Parmetis (4.0.3)
- ARPACK (3.6.3)
- Libevent (2.1.8)
- GCC(8.2.0)
- TCL (8.6.9)** Tcl only
- Python(3.7.2)** OpenSeesPy only

Please ensure that your project complies with each library’s licensing requirements.

Another feature of this build platform is modularity. Developers can select from a list of build options that can be turned on and off as required. The basic configuration builds the structural analysis core. Other build options include parallel processing, reliability, and the particle finite element method (PFEM) modules. Python and Tcl interpreters are additional build options. These options are located in single configuration file called qmake.conf. By default, the environment is configured to build the core along with the parallel processing module. Other options can be turned on by deleting the ‘#’ symbol that precedes the option; this includes the option into the build environment.

---

**Note:** This build environment comes with pre-compiled dependencies. Although this makes getting started easier, the caveat is that your build environment (compiler version) must match that of the environment used to compile the dependencies. The supported environments are listed below. There are no guarantees that it will work with other build environments. In other words, make sure your compiler type and version (i.e., clang-1000.11.45.5) matches the version below listed under the heading ‘Supported Build Environments’. Otherwise, bad things might happen. Also, this project is still a work in progress. Currently, only building in OS X is supported. Windows support will be added shortly. Moreover, not all build options are supported. For example, compiling with fortran is not supported.

---

**Note:** This project uses qmake and Qt Creator. qmake is a build tool for compiling and linking applications. Qt Creator is a free IDE (interactive development environment) that bundles code writing/editing and application building within one program. Qt Creator uses project (.pro) files. The project files contain all information required by qmake to build an application.
1.2.3 Getting started:

1. Download and install Qt open source from https://www.qt.io/download The version of the Qt library is not important in this case since the library is not used in the OpenSees project (although I use Qt in other projects and recommend it)

2. Download the OpenSeesQt source code into a folder of your choice (https://github.com/steva44/OpenSees/archive/master.zip)

3. In the directory containing the code, double click on the ‘OpenSees.pro’ file. If compiling OpenSeesPy, open the ‘OpenSeesPy.pro’ file. The .pro files are project files that will automatically open the project in Qt Creator.

4. Select a build configuration, for example ‘Desktop Qt 5.12.1 clang 64bit’. The project will automatically configure itself. You only have to do this once.

5. The left-hand pane should now display the project directory structure. In the left-hand pane, under the heading qmake, open the ‘qmake.conf’ file. Review and select additional build options, if any. Note that this is still a work in progress and not all build options are supported.

6. Click on the ‘start’ button in the bottom lefthand corner of Qt Creator to compile. Clicking on the small computer symbol above the start button allows for switching between the debug and release deploy configurations. The release deployment results in faster program execution but it does not allow for debugging or stepping through the code. The start button with the bug symbol opens the debugger.

7. Go and have a coffee, it will take a few minutes to finish compiling!

1.2.4 Building OpenSeesPy:

OpenSeesPy builds OpenSees as a library object that can be used within Python.

Steps: Follow steps 1-4 under the heading getting started above.

1. The left-hand pane should now display the project directory structure. In the left-hand pane, under the heading qmake, open the ‘qmake.conf’ file. Under the heading #INTERPRETERS, uncomment the _PYTHON option by removing the ‘#’ symbol. Everything else should be configured automatically going forward. Python automatically compiles with the reliability, parallel, and PFEM modules.

2. The last few lines at the end of the ‘OpenSeesPy.pro’ file contain the location of the Python framework. Update this so that it matches the location of Python on your build system.

3. Click on the ‘start’ button in the bottom lefthand corner of Qt Creator to start compiling. Clicking on the small computer symbol allows for switching between the debug and release deploy configurations. The release deployment results in faster program execution but it does not allow for debugging or stepping through the code. Build in release mode if using OpenSees as a library in a Python project.

4. Go and have a coffee, it will take a few minutes to finish compiling!

5. After successful compilation, the library will be in the ‘bin’ folder. The bin folder is located in the ‘build’ folder which is created, by default, one directory higher than the OpenSeesQt source code. The name of the build folder should look something like this: build-OpenSeesPy-Desktop_Qt_5_12_1_clang_64bit-Debug

6. OS X only

OS X automatically prepends a ‘lib’ to the library file. Remove this ‘lib’ and rename the file to be ‘opensees.dylib’ Next, a symbolic link is required for a Python project to import the library. To create a symbolic link, cd the directory containing the OpenSees library in terminal and run the following command to create a symbolic link:

```
ln -s opensees.dylib opensees.so
```
There should now be a .so (shared object) file in addition to the .dylib file. Finally, copy both the .dylib and the .so ‘link’ into your python environment folder to import it into your project. Directions for using OpenSeesPy can be found at the project website: https://openseespydoc.readthedocs.io/en/latest/index.html

1.2.5 Supported Build Environments:

OSX

Build Environment:
- OSX 10.14.3 (Mojave)
- Qt 5.12.1
- Qt Creator 4.8.1

Compiler:
- Apple LLVM version 10.0.0 (clang-1000.11.45.5)
- Target: x86_64-apple-darwin18.2.0
- Thread model: posix 64-BIT architecture

To find the version of clang on your computer, type the following in terminal:

```
clang --version
```

Note: This project comes with pre-built libraries for everything except Python. Therefore, you do not have to go through the trouble of building any libraries unless you are using a special build system or you want to experiment. The precompiled library files are located in the ‘OpenSeesLibs’ folder. In the event that you are feeling adventurous and you want to compile the libraries on your own, instructions are given below for each library, for each operating system. After successful compilation, note the installation directory. This directory contains the locations of the ‘include’ and ‘lib’ folders for that library. If replacing or adding new libraries, the file paths should be updated in the ‘OpenSeesLibs.pri’ file. This is required so that the compiler knows where to find the header files and to link the libraries to your project.

OSX

On OSX, the dependencies are built/installed with Homebrew. Homebrew is a free and open-source software package management system that simplifies the installation of software on Apple’s macOS operating system and Linux. Homebrew maintains its own folder within /usr/local/ directory aptly named the 'Cellar':

```
/usr/local/Cellar/
```

Each dependency installed through Homebrew will have its own subfolder within the Cellar directory. Each subfolder contains that dependencies ‘include’ and ‘lib’ folders.

1.2.6 MUMPS

MUltifrontal Massively Parallel sparse direct Solver, or MUMPS, is a sparse direct solver used for parallel solving of a system of equations.

Installing MUMPS via brew: Dominique Orban has written a Homebrew formula (http://brew.sh) for Mac OSX users. Homebrew MUMPS is now available via the OpenBLAS tap. Build instructions are as follows:

In terminal, copy and paste each command individually and execute:
brew tap dpo/openblas
brew tap-pin dpo/openblas
brew options mumps # to discover build options
brew install mumps [options...]

The options can be left blank, i.e., with default options so the last line will look like:

brew install mumps

Mumps requires the following dependencies that will automatically be installed:

- Scalapack

1.2.7 OpenMPI

OpenMPI is a high performance message passing library (https://www.open-mpi.org/)

Installing OpenMPI via brew: In terminal, copy and paste the following command and execute:

brew install open-mpi

OpenMPI requires the following dependencies that will automatically be installed:

- GCC (GNU compiler collection)
- libevent (Asynchronous event library: https://libevent.org/)

1.2.8 UMFPACK

UMFPACK is a set of routines for solving unsymmetric sparse linear systems of the form Ax=b, using the Unsymmetric MultiFrontal method (Matrix A is not required to be symmetric). UMFPACK is part of suite-sparse library in homebrew/science

In terminal, copy and paste each command individually and execute:

brew tap homebrew/science
brew install suite-sparse

UMFPACK requires the following dependencies that will automatically be installed:

- Metis (‘METIS’ is a type of GraphPartitioner and numberer - An Unstructured Graph Partitioning And Sparse Matrix Ordering System’, developed by G. Karypis and V. Kumar at the University of Minnesota.

1.2.9 SUPERLU

SUPERLU is a general purpose library for the direct solution of large, sparse, nonsymmetric systems of linear equations. The library is written in C and is callable from either C or Fortran program. It uses MPI, OpenMP and CUDA to support various forms of parallelism.

Installing SUPERLU via brew In terminal, copy and paste the following command and execute:

brew install superlu

Should install by default with option --with-openmp enabled. Open MP is needed for parallel analysis.

SUPERLU requires the following dependencies that will automatically be installed:
• GCC (GNU compiler collection)
• openblas (In scientific computing, OpenBLAS is an open source implementation of the BLAS API with many hand-crafted optimizations for specific processor types)

1.2.10 SUPERLUMT
SUPERLU but for shared memory parallel machines. Provides Pthreads and OpenMP interfaces.
Installing SUPERLUMT via brew: In terminal, copy and paste the following command and execute:

```
brew install superlu_mt
```

SUPERLUMT requires the following dependencies that will automatically be installed:
• openblas

1.2.11 SUPERLUDIST
SUPERLU but for distributed memory parallel machines. Supports manycore heterogeneous node architecture: MPI is used for interprocess communication, OpenMP is used for on-node threading, CUDA is used for computing on GPUs.
Installing SUPERLUDIST via brew: In terminal, copy and paste the following command and execute:

```
brew install superlu_dist
```

SUPERLUDIST requires the following dependencies that will automatically be installed:
• GCC (GNU compiler collection)
• openblas (In scientific computing, OpenBLAS is an open source implementation of the BLAS API with many hand-crafted optimizations for specific processor types)
• OpenMPI (a high performance message passing library (https://www.open-mpi.org/))
• Parmetis (MPI library for graph/mesh partitioning and fill-reducing orderings)

1.2.12 LAPACK (SCALAPACK)
The Linear Algebra PACKage, or LAPACK, is written in Fortran 90 and provides routines for solving systems of simultaneous linear equations, least-squares solutions of linear systems of equations, eigenvalue problems, and singular value problems. The associated matrix factorizations (LU, Cholesky, QR, SVD, Schur, generalized Schur) are also provided, as are related computations such as reordering of the Schur factorizations and estimating condition numbers. Dense and banded matrices are handled, but not general sparse matrices. In all areas, similar functionality is provided for real and complex matrices, in both single and double precision.

LAPACK is given as a system library in OSX, you may have to update the locations of your system library in ‘OpenSeesLibs.pri’

1.2.13 BLAS
The BLAS (Basic Linear Algebra Subprograms) are routines that provide standard building blocks for performing basic vector and matrix operations.
OpenSeesPy Documentation, Release 0.4.2019.7

BLAS is given as a system library in OSX, you may have to update the locations of your system library in ‘OpenSeesLibs.pri’

1.2.14 ARPACK

ARPACK contains routines to solve large scale eigenvalue problems

Installing ARPACK via brew: In terminal, copy and paste the following command and execute:

```
brew install arpack
```

ARPACK requires the following dependencies that will automatically be installed:

- GCC (GNU compiler collection)
- openblas (In scientific computing, OpenBLAS is an open source implementation of the BLAS API with many hand-crafted optimizations for specific processor types)

1.2.15 GCC

Many of the dependencies require fortran (there is still a lot of legacy fortran code floating around in the engineering world). On OSX, I found the best solution is to use the pre-bundled fortran capabilities in the GNU compiler collection or GCC. In addition to its fortran capabilities, GCC is a dependency for many other libraries.

Installing GCC via brew: In terminal, copy and paste the following command and execute:

```
brew install GCC
```

1.2.16 PYTHON

Python is an interpreted, high-level, general-purpose programming language. It is used in OpenSees as an interpreter in the OpenSeesPy version. In OpenSeesPy, Python version 3 is used.

Installing PYTHON via brew:

```
brew install python
```

1.2.17 MISC. NOTES

For the SUPERLU library. The file supermatrix.h throws an undefined error for the type int_t. It is actually defined in the file slu_ddefs.h, but for some reason the compiler is not linking the two. Add the following line, copied from slu_ddefs.h to supermatrix.h around line 17:

```
typedef int int_t; /* default */
```

1.3 Change Log

- Version 0.5.4
  - Support Mac
  - Support Python3.8 on Windows and Linux
• **Version 0.5.3**
  - Fix bug in LimitState UniadialMaterial
  - Automatic trimming spaces for string inputs
  - Some output commands return lists instead of ints, such as nodeDisp etc.

• **Version 0.5.2**
  - Add package openseespy.postprocessing
  - Add setStartNodeTag command
  - modalDamping: bug fixes
  - Add Steel02Fatigue material
  - Add Concrete02IS material
  - Add HardeningMaterial2 material
  - Add hystereticBackone command
  - Add stiffnessDegradation command
  - Add strengthDegradation command
  - Add unloadingRule command

• **Version 0.4.2019.7**
  - Parallel: the Linux version is enabled with parallel capability
  - Python stream: add no echo
  - Mesh: add CorotTruss
  - TriMesh: can create line elements
  - QuadMesh: can create line and triangular elements
  - Python inputs: more flexible input types
  - Commands: add ExplicitDifference integrator

• **Version 0.3.0**
  - Add logFile command
  - Add partial uniform load fo ForceBeamColumn
  - Add ShellDKGT element
  - Add ‘-V’ option in Newmark and HHT
  - Fix bugs in wipe and Mesh
  - Various PFEM updates
  - Update to OpenSees 3.0.3

• **Version 0.2.0** (8a3d622)
  - OpenSeesPy now can print messages and errors in Jupyter Notebook and other Windows based Python applications
  - Add setParameter command
  - Add nodeDOFs command
– Add setNumThread and getNumThread commands in a multi-threaded environment
– Add logFile command
– printA and prinbB can return matrix and vector as lists
– Fix bugs in updateMaterialStage
– PM4Sand improvements
– Add CatenaryCable element to OpenSeesPy

• Version 0.1.1 (f9f45fe)
  – Update to OpenSees 3.0.2

• Version 0.0.7 (b75db21)
  – Add “2D wheel-rail” element
  – PVD recorder allows to set a path
  – Add “sdfResponse” function for single dof dynamic analysis
  – Fix a bug in Joint2D
  – Fix typo in UCSD UP elements
  – Fix bugs in PressureIndependMultiYield
  – Add JSON print options to some materials and elements

• Version 0.0.6 (cead6e8)
  – Add “nonlinearBeamColumn” element for backward compatatability
  – Add “updateMaterialStage” function
  – Add “RCCircular” secton
  – Add “quadr” patch for backward compatibility
  – Fix bugs in “Steel01Thermal” material
  – Fix bugs in Truss
  – Fix bugs in eleNodes function
  – Fix bugs in ZeroLength element
  – Fix bugs in FiberSection2d
  – Fix bugs in PFEMLinSOE

• Version 0.0.5 (215c63d)
  – Update to OpenSees 3.0.0

1.4 Model Commands

The model or domain in OpenSees is a collection (an aggregation in object-oriented terms) of elements, nodes, single- and multi-point constraints and load patterns. It is the aggregation of these components which define the type of model that is being analyzed.

1. model command
2. element commands
3. node command
4. sp constraint commands
5. mp constraint commands
6. timeSeries commands
7. pattern commands
8. mass command
9. region command
10. rayleigh command
11. block commands
12. beamIntegration commands
13. uniaxialMaterial commands
14. nDMaterial commands
15. section commands
16. frictionModel commands
17. geomTransf commands

1.4.1 model command

```python
model ('basic', '-ndm', ndm, '-ndf', ndf=ndm*(ndm+1)/2)
```

Set the default model dimensions and number of dofs.

| ndm (int) | number of dimensions (1,2,3) |
| ndf (int) | number of dofs (optional) |

1.4.2 element commands

```python
element (eleType, eleTag, *eleNodes, *eleArgs)
```

Create a OpenSees element.

| eleType (str) | element type |
| eleTag (int)  | element tag. |
| eleNodes (list (int)) | a list of element nodes, must be preceded with *. |
| eleArgs (list) | a list of element arguments, must be preceded with *. |

For example,

```python
eleType = 'truss'
eleTag = 1
eleNodes = [iNode, jNode]
eleArgs = [A, matTag]
```

```python
element(eleType, eleTag, *eleNodes, *eleArgs)
```

The following contain information about available eleType:
Zero-Length Element

1. `zeroLength Element`
2. `zeroLengthND Element`
3. `zeroLengthSection Element`
4. `CoupledZeroLength Element`
5. `zeroLengthContact Element`
6. `zeroLengthContactNTS2D`
7. `zeroLengthInterface2D`
8. `zeroLengthImpact3D`

**zeroLength Element**

`element ('zeroLength', eleTag, *eleNodes, '-mat', *matTags, '-dir', *dirs[, '-doRayleigh', rFlag=0][, '-orient', *vecx, *vecyp])`

This command is used to construct a zeroLength element object, which is defined by two nodes at the same location. The nodes are connected by multiple UniaxialMaterial objects to represent the force-deformation relationship for the element.

<table>
<thead>
<tr>
<th>Parameter</th>
<th>Description</th>
</tr>
</thead>
<tbody>
<tr>
<td><code>eleTag (int)</code></td>
<td>unique element object tag</td>
</tr>
<tr>
<td><code>eleNodes (list(int))</code></td>
<td>a list of two element nodes</td>
</tr>
<tr>
<td><code>matTags (list(int))</code></td>
<td>a list of tags associated with previously-defined UniaxialMaterials</td>
</tr>
<tr>
<td><code>dirs (list(int))</code></td>
<td>a list of material directions:</td>
</tr>
<tr>
<td></td>
<td>• 1,2,3 - translation along local x,y,z axes, respectively;</td>
</tr>
<tr>
<td></td>
<td>• 4,5,6 - rotation about local x,y,z axes, respectively</td>
</tr>
<tr>
<td><code>vecx (list(float))</code></td>
<td>a list of vector components in global coordinates defining local x-axis (optional)</td>
</tr>
<tr>
<td><code>vecyp (list(float))</code></td>
<td>a list of vector components in global coordinates defining vector yp which lies in the local x-y plane for the element. (optional)</td>
</tr>
<tr>
<td><code>rFlag (float)</code></td>
<td>optional, default = 0</td>
</tr>
</tbody>
</table>

- `rFlag = 0` NO RAYLEIGH DAMPING (default)
- `rFlag = 1` include rayleigh damping

**Note:** If the optional orientation vectors are not specified, the local element axes coincide with the global axes. Otherwise the local z-axis is defined by the cross product between the vectors x and yp vectors specified on the command line.

**See also:**

**Notes**
zeroLengthND Element

element ("zeroLengthND", eleTag, *eleNodes, matTag[, uniTag][, '-orient', *vecx, vecyp ])

This command is used to construct a zeroLengthND element object, which is defined by two nodes at the same location. The nodes are connected by a single NDMaterial object to represent the force-deformation relationship for the element.

<table>
<thead>
<tr>
<th>Parameter</th>
<th>Description</th>
</tr>
</thead>
<tbody>
<tr>
<td>eleTag (int)</td>
<td>unique element object tag</td>
</tr>
<tr>
<td>eleNodes (list (int))</td>
<td>a list of two element nodes</td>
</tr>
<tr>
<td>matTag (int)</td>
<td>tag associated with previously-defined ndMaterial object</td>
</tr>
<tr>
<td>uniTag (int)</td>
<td>tag associated with previously-defined UniaxialMaterial object which may be used to represent uncoupled behavior orthogonal to the plane of the NDMaterial response. SEE NOTES 2 and 3.</td>
</tr>
<tr>
<td>vecx (list (float))</td>
<td>a list of vector components in global coordinates defining local x-axis (optional)</td>
</tr>
<tr>
<td>vecyp (list (float))</td>
<td>a list of vector components in global coordinates defining vector yp which lies in the local x-y plane for the element. (optional)</td>
</tr>
</tbody>
</table>

Note:

1. The zeroLengthND element only represents translational response between its nodes
2. If the NDMaterial object is of order two, the response lies in the element local x-y plane and the UniaxialMaterial object may be used to represent the uncoupled behavior orthogonal to this plane, i.e. along the local z-axis.
3. If the NDMaterial object is of order three, the response is along each of the element local axes.
4. If the optional orientation vectors are not specified, the local element axes coincide with the global axes. Otherwise the local z-axis is defined by the cross product between the vectors x and yp vectors specified on the command line.
5. The valid queries to a zero-length element when creating an ElementRecorder object are ‘force’, ‘deformation’, and ‘material matArg1 matArg2 ...’

See also:

Notes

zeroLengthSection Element

element ("zeroLengthSection", eleTag, *eleNodes, secTag[, '-orient', *vecx, *vecyp ][, '-doRayleigh', rFlag ])

This command is used to construct a zero length element object, which is defined by two nodes at the same location. The nodes are connected by a single section object to represent the force-deformation relationship for the element.
eleTag (int) unique element object tag

eleNodes (list (int)) a list of two element nodes

secTag (int) tag associated with previously-defined Section object

vecx (list (float)) a list of vector components in global coordinates defining local x-axis (optional)

vecyp (list (float)) a list of vector components in global coordinates defining vector yp which lies in the local x-y plane for the element. (optional)

rFlag (float) optional, default = 0
- rFlag = 0 NO RAYLEIGH DAMPING (default)
- rFlag = 1 include rayleigh damping

See also:

Notes

**CoupledZeroLength Element**

element (‘CoupledZeroLength’, eleTag, *eleNodes, dirn1, dirn2, matTag[, rFlag=1])

eleTag (int) unique element object tag

eleNodes (list (int)) a list of two element nodes

matTag (float) tags associated with previously-defined UniaxialMaterial

dirn1 dirn2 (int) the two directions, 1 through ndof.

rFlag (float) optional, default = 0
- rFlag = 0 NO RAYLEIGH DAMPING (default)
- rFlag = 1 include rayleigh damping

See also:

Notes

**zeroLengthContact Element**

element (‘zeroLengthContact2D’, eleTag, *eleNodes, Kn, Kt, mu, ‘-normal’, Nx, Ny)

This command is used to construct a zeroLengthContact2D element, which is Node-to-node frictional contact element used in two dimensional analysis and three dimensional analysis:

eleTag (int) unique element object tag

eleNodes (list (int)) a list of a slave and a master nodes

Kn (float) Penalty in normal direction

Kt (float) Penalty in tangential direction

mu (float) friction coefficient
**openSeesPy Documentation, Release 0.4.2019.7**

**element** ('zeroLengthContact3D', eleTag, *eleNodes, Kn, Kt, mu, c, dir)

This command is used to construct a zeroLengthContact3D element, which is Node-to-node frictional contact element used in two dimensional analysis and three dimensional analysis:

<table>
<thead>
<tr>
<th>Parameter</th>
<th>Description</th>
</tr>
</thead>
<tbody>
<tr>
<td>eleTag (int)</td>
<td>unique element object tag</td>
</tr>
<tr>
<td>eleNodes (list (int))</td>
<td>a list of a slave and a master nodes</td>
</tr>
<tr>
<td>Kn (float)</td>
<td>Penalty in normal direction</td>
</tr>
<tr>
<td>Kt (float)</td>
<td>Penalty in tangential direction</td>
</tr>
<tr>
<td>mu (float)</td>
<td>friction coefficient</td>
</tr>
<tr>
<td>c (float)</td>
<td>cohesion (not available in 2D)</td>
</tr>
<tr>
<td>dir (int)</td>
<td>Direction flag of the contact plane (3D), it can be:</td>
</tr>
<tr>
<td></td>
<td>• 1 Out normal of the master plane pointing to +X direction</td>
</tr>
<tr>
<td></td>
<td>• 2 Out normal of the master plane pointing to +Y direction</td>
</tr>
<tr>
<td></td>
<td>• 3 Out normal of the master plane pointing to +Z direction</td>
</tr>
</tbody>
</table>

See also:

Notes

**zeroLengthContactNTS2D**

**element** ('zeroLengthContactNTS2D', eleTag, '-sNdNum', sNdNum, '-mNdNum', mNdNum, '-Nodes', *Nodes, Kn, kt, phi)

<table>
<thead>
<tr>
<th>Parameter</th>
<th>Description</th>
</tr>
</thead>
<tbody>
<tr>
<td>eleTag (int)</td>
<td>unique element object tag</td>
</tr>
<tr>
<td>sNdNum (int)</td>
<td>Number of Slave Nodes</td>
</tr>
<tr>
<td>mNdNum (int)</td>
<td>Number of Master nodes</td>
</tr>
<tr>
<td>Nodes (list (int))</td>
<td>Slave and master node tags respectively</td>
</tr>
<tr>
<td>Kn (float)</td>
<td>Penalty in normal direction</td>
</tr>
<tr>
<td>Kt (float)</td>
<td>Penalty in tangential direction</td>
</tr>
<tr>
<td>phi (float)</td>
<td>Friction angle in degrees</td>
</tr>
</tbody>
</table>

Note:

1. The contact element is node-to-segment (NTS) contact. The relation follows Mohr-Coulomb frictional law: \( T = N \times \tan(\phi) \), where \( T \) is the tangential force, \( N \) is normal force across the interface and \( \phi \) is friction angle.

2. For 2D contact, slave nodes and master nodes must be 2 DOF and notice that the slave and master nodes must be entered in counterclockwise order.

3. The resulting tangent from the contact element is non-symmetric. Switch to the non-symmetric matrix solver if convergence problem is experienced.

4. As opposed to node-to-node contact, predefined normal vector for node-to-segment (NTS) element is not required because contact normal will be calculated automatically at each step.

5. contact element is implemented to handle large deformations.

See also:

1.4. Model Commands
Notes

### zeroLengthInterface2D

**element** (`'zeroLengthInterface2D', eleTag, '-sNdNum', sNdNum, '-mNdNum', mNdNum, '-dof', sdof, mdo, 
'-Nodes', *Nodes, Kn, kt, phi)

<table>
<thead>
<tr>
<th>Parameter</th>
<th>Description</th>
</tr>
</thead>
<tbody>
<tr>
<td>eleTag (int)</td>
<td>unique element object tag</td>
</tr>
<tr>
<td>sNdNum (int)</td>
<td>Number of Slave Nodes</td>
</tr>
<tr>
<td>mNdNum (int)</td>
<td>Number of Master nodes</td>
</tr>
<tr>
<td>sdof, mdo (int)</td>
<td>Slave and Master degree of freedom</td>
</tr>
<tr>
<td>Nodes (list (int))</td>
<td>Slave and master node tags respectively</td>
</tr>
<tr>
<td>Kn (float)</td>
<td>Penalty in normal direction</td>
</tr>
<tr>
<td>Kt (float)</td>
<td>Penalty in tangential direction</td>
</tr>
<tr>
<td>phi (float)</td>
<td>Friction angle in degrees</td>
</tr>
</tbody>
</table>

**Note:**

1. The contact element is node-to-segment (NTS) contact. The relation follows Mohr-Coulomb frictional law: 
   \[ T = N \times \tan(\phi) \], where \( T \) is the tangential force, \( N \) is normal force across the interface and \( \phi \) is friction angle.
2. For 2D contact, slave nodes and master nodes must be 2 DOF and notice that the slave and master nodes must be entered in counterclockwise order.
3. The resulting tangent from the contact element is non-symmetric. Switch to the non-symmetric matrix solver if convergence problem is experienced.
4. As opposed to node-to-node contact, predefined normal vector for node-to-segment (NTS) element is not required because contact normal will be calculated automatically at each step.
5. Contact element is implemented to handle large deformations.

**See also:**

Notes

### zeroLengthImpact3D

**element** (`'zeroLengthImpact3D', eleTag, *eleNodes, direction, initGap, frictionRatio, Kt, Kn, Kn2, Delta_y, 
cohesion)

This command constructs a node-to-node zero-length contact element in 3D space to simulate the impact/pounding and friction phenomena.
**eleTag** (int)  
unique element object tag  

**eleNodes** (list (int))  
a list of a slave and a master nodes  

**direction** (int)  
- 1 if out-normal vector of master plane points to +X direction  
- 2 if out-normal vector of master plane points to +Y direction  
- 3 if out-normal vector of master plane points to +Z direction  

**initGap** (float)  
Initial gap between master plane and slave plane  

**frictionRatio** (float)  
Friction ratio in two tangential directions (parallel to master and slave planes)  

**Kt** (float)  
Penalty in two tangential directions  

**Kn** (float)  
Penalty in normal direction (normal to master and slave planes)  

**Kn2** (float)  
Penalty in normal direction after yielding based on Hertz impact model  

**Delta_y** (float)  
Yield deformation based on Hertz impact model  

**cohesion** (float)  
Cohesion, if no cohesion, it is zero  

**Note:**  
1. This element has been developed on top of the “zeroLengthContact3D”. All the notes available in “zeroLengthContact3D” wiki page would apply to this element as well. It includes the definition of master and slave nodes, the number of degrees of freedom in the domain, etc.  
2. Regarding the number of degrees of freedom (DOF), the end nodes of this element should be defined in 3DOF domain. For getting information on how to use 3DOF and 6DOF domain together, please refer to OpenSees documentation and forums or see the zip file provided in the EXAMPLES section below.  
3. This element adds the capabilities of “ImpactMaterial” to “zeroLengthContact3D.”  
4. For simulating a surface-to-surface contact, the element can be defined for connecting the nodes on slave surface to the nodes on master surface.  
5. The element was found to be fast-converging and eliminating the need for extra elements and nodes in the modeling process.  

**See also:**  

**Notes**  

**Truss Elements**  

1. **Truss Element**  
2. **Corotational Truss Element**  

**Truss Element**  

This command is used to construct a truss element object. There are two ways to construct a truss element object:
element ('Truss', eleTag, *eleNodes, A, matTag[, '-rho', rho ][, '-cMass', cFlag ][, '-doRayleigh', rFlag ])
One way is to specify an area and a UniaxialMaterial identifier:

\[
element \left( \text{'TrussSection', eleTag, *eleNodes, A, secTag[, '-rho', rho ][, '-cMass', cFlag ][, '-doRayleigh', rFlag ]} \right)
\]
the other is to specify a Section identifier:

<table>
<thead>
<tr>
<th>eleTag (int)</th>
<th>unique element object tag</th>
</tr>
</thead>
<tbody>
<tr>
<td>eleNodes (list (int))</td>
<td>a list of two element nodes</td>
</tr>
<tr>
<td>A (float)</td>
<td>cross-sectional area of element</td>
</tr>
<tr>
<td>matTag (int)</td>
<td>tag associated with previously-defined UniaxialMaterial</td>
</tr>
<tr>
<td>secTag (int)</td>
<td>tag associated with previously-defined Section</td>
</tr>
<tr>
<td>rho (float)</td>
<td>mass per unit length, optional, default = 0</td>
</tr>
<tr>
<td>cFlag (float)</td>
<td>consistent mass flag, optional, default = 0</td>
</tr>
<tr>
<td>rFlag (float)</td>
<td>Rayleigh damping flag, optional, default = 0</td>
</tr>
</tbody>
</table>

\[• \text{cFlag} = 0 \text{ lumped mass matrix (default)}\]
\[• \text{cFlag} = 1 \text{ consistent mass matrix} \]
\[• \text{rFlag} = 0 \text{ NO RAYLEIGH DAMPING (default)}\]
\[• \text{rFlag} = 1 \text{ include Rayleigh damping} \]

Note:
1. The truss element DOES NOT include geometric nonlinearities, even when used with beam-columns utilizing P-Delta or Corotational transformations.
2. When constructed with a UniaxialMaterial object, the truss element considers strain-rate effects, and is thus suitable for use as a damping element.
3. The valid queries to a truss element when creating an ElementRecorder object are 'axialForce,' 'forces,' 'localForce', deformations,' 'material matArg1 matArg2...,' 'section sectArg1 sectArg2...' There will be more queries after the interface for the methods involved have been developed further.

See also:
Notes

Corotational Truss Element

This command is used to construct a corotational truss element object. There are two ways to construct a corotational truss element object:

element ('corotTruss', eleTag, *eleNodes, A, matTag[, '-rho', rho ][, '-cMass', cFlag ][, '-doRayleigh', rFlag ])
One way is to specify an area and a UniaxialMaterial identifier:

\[
element \left( \text{'corotTrussSection', eleTag, *eleNodes, secTag[, '-rho', rho ][, '-cMass', cFlag ][, '-doRayleigh', rFlag ]} \right)
\]
the other is to specify a Section identifier:
eleTag (int) | unique element object tag  
---|---  
eleNodes (list (int)) | a list of two element nodes  
A (float) | cross-sectional area of element  
matTag (int) | tag associated with previously-defined UniaxialMaterial  
secTag (int) | tag associated with previously-defined Section  
rho (float) | mass per unit length, optional, default = 0  
cFlag (float) | consistent mass flag, optional, default = 0  
| • cFlag = 0 lumped mass matrix (default)  
| • cFlag = 1 consistent mass matrix  
rFlag (float) | Rayleigh damping flag, optional, default = 0  
| • rFlag = 0 NO RAYLEIGH DAMPING (default)  
| • rFlag = 1 include Rayleigh damping

Note:

1. When constructed with a UniaxialMaterial object, the corotational truss element considers strain-rate effects, and is thus suitable for use as a damping element.

2. The valid queries to a truss element when creating an ElementRecorder object are ‘axialForce,’ ‘stiff,’ deformations,’ ‘material matArg1 matArg2…,’ ‘section sectArg1 sectArg2…’ There will be more queries after the interface for the methods involved have been developed further.

3. CorotTruss DOES NOT include Rayleigh damping by default.

See also:

Notes

Beam-Column Elements

1. Elastic Beam Column Element
2. Elastic Beam Column Element with Stiffness Modifiers
3. Elastic Timoshenko Beam Column Element
4. Beam With Hinges Element
5. dispBeamColumn
6. forceBeamColumn
7. nonlinearBeamColumn
8. Flexure-Shear Interaction Displacement-Based Beam-Column Element
9. MVLEM - Multiple-Vertical-Line-Element-Model for RC Walls
10. SFI MVLEM - Cyclic Shear-Flexure Interaction Model for RC Walls
Elastic Beam Column Element

This command is used to construct an elasticBeamColumn element object. The arguments for the construction of an
elastic beam-column element depend on the dimension of the problem, ndm:

\[
\text{element} \left( \text{elasticBeamColumn}', \text{eleTag}, \text{*eleNodes}, \text{A, E, I_z, transfTag}, \text{',-mass', massDens} \right], \text{',-cMass'} \right]
\]

For a two-dimensional problem

\[
\text{element} \left( \text{elasticBeamColumn}', \text{eleTag}, \text{*eleNodes}, \text{A, E, G, J, I_y, I_z, transfTag}, \text{',-mass', massDens} \right], \text{',-cMass'} \right]
\]

For a three-dimensional problem

<table>
<thead>
<tr>
<th>Parameter</th>
<th>Description</th>
</tr>
</thead>
<tbody>
<tr>
<td>eleTag (int)</td>
<td>unique element object tag</td>
</tr>
<tr>
<td>eleNodes (list (int))</td>
<td>a list of two element nodes</td>
</tr>
<tr>
<td>A (float)</td>
<td>cross-sectional area of element</td>
</tr>
<tr>
<td>E (float)</td>
<td>Young’s Modulus</td>
</tr>
<tr>
<td>G (float)</td>
<td>Shear Modulus</td>
</tr>
<tr>
<td>J (float)</td>
<td>torsional moment of inertia of cross section</td>
</tr>
<tr>
<td>I_z (float)</td>
<td>second moment of area about the local z-axis</td>
</tr>
<tr>
<td>I_y (float)</td>
<td>second moment of area about the local y-axis</td>
</tr>
<tr>
<td>transfTag (int)</td>
<td>identifier for previously-defined coordinate-transformation (CrdTransf) object</td>
</tr>
<tr>
<td>massDens (float)</td>
<td>element mass per unit length (optional, default = 0.0)</td>
</tr>
<tr>
<td>'-cMass' (str)</td>
<td>to form consistent mass matrix (optional, default = lumped mass matrix)</td>
</tr>
</tbody>
</table>

See also:

Notes

Elastic Beam Column Element with Stiffness Modifiers

This command is used to construct a ModElasticBeam2d element object. The arguments for the construction of an
elastic beam-column element with stiffness modifiers is applicable for 2-D problems. This element should be used for
modelling of a structural element with an equivalent combination of one elastic element with stiffness-proportional
damping, and two springs at its two ends with no stiffness proportional damping to represent a prismatic section. The
modelling technique is based on a number of analytical studies discussed in Zareian and Medina (2010) and Zareian
and Krawinkler (2009) and is utilized in order to solve problems related to numerical damping in dynamic analysis of
frame structures with concentrated plasticity springs.

\[
\text{element} \left( \text{ModElasticBeam2d}', \text{eleTag}, \text{*eleNodes}, \text{A, E, I_z, K_{11}, K_{33}, K_{44}, transfTag}, \text{',-mass', massDens} \right], \text{',-cMass'} \right]
\]

<table>
<thead>
<tr>
<th>Parameter</th>
<th>Description</th>
</tr>
</thead>
<tbody>
<tr>
<td>eleTag (int)</td>
<td>unique element object tag</td>
</tr>
<tr>
<td>eleNodes (list (int))</td>
<td>a list of two element nodes</td>
</tr>
<tr>
<td>A (float)</td>
<td>cross-sectional area of element</td>
</tr>
<tr>
<td>E (float)</td>
<td>Young’s Modulus</td>
</tr>
<tr>
<td>I_z (float)</td>
<td>second moment of area about the local z-axis</td>
</tr>
<tr>
<td>K_{11} (float)</td>
<td>stiffness modifier for translation</td>
</tr>
<tr>
<td>K_{33} (float)</td>
<td>stiffness modifier for translation</td>
</tr>
<tr>
<td>K_{44} (float)</td>
<td>stiffness modifier for rotation</td>
</tr>
<tr>
<td>transfTag (int)</td>
<td>identifier for previously-defined coordinate-transformation (CrdTransf) object</td>
</tr>
<tr>
<td>massDens (float)</td>
<td>element mass per unit length (optional, default = 0.0)</td>
</tr>
<tr>
<td>'-cMass' (str)</td>
<td>to form consistent mass matrix (optional, default = lumped mass matrix)</td>
</tr>
</tbody>
</table>
Elastic Timoshenko Beam Column Element

This command is used to construct an ElasticTimoshenkoBeam element object. A Timoshenko beam is a frame member that accounts for shear deformations. The arguments for the construction of an elastic Timoshenko beam element depend on the dimension of the problem, ndm:


For a two-dimensional problem:


For a three-dimensional problem:

<table>
<thead>
<tr>
<th>\textbf{eleTag (int)}</th>
<th>unique element object tag</th>
</tr>
</thead>
<tbody>
<tr>
<td>\textbf{eleNodes (list (int))}</td>
<td>a list of two element nodes</td>
</tr>
<tr>
<td>\textbf{E (float)}</td>
<td>Young’s Modulus</td>
</tr>
<tr>
<td>\textbf{G (float)}</td>
<td>Shear Modulus</td>
</tr>
<tr>
<td>\textbf{A (float)}</td>
<td>cross-sectional area of element</td>
</tr>
<tr>
<td>\textbf{Jx (float)}</td>
<td>torsional moment of inertia of cross section</td>
</tr>
<tr>
<td>\textbf{Iy (float)}</td>
<td>second moment of area about the local y-axis</td>
</tr>
<tr>
<td>\textbf{Iz (float)}</td>
<td>second moment of area about the local z-axis</td>
</tr>
<tr>
<td>\textbf{Avy (float)}</td>
<td>Shear area for the local y-axis</td>
</tr>
<tr>
<td>\textbf{Avz (float)}</td>
<td>Shear area for the local z-axis</td>
</tr>
<tr>
<td>\textbf{transfTag (int)}</td>
<td>identifier for previously-defined coordinate-transformation (CrdTransf) object</td>
</tr>
<tr>
<td>\textbf{massDens (float)}</td>
<td>element mass per unit length (optional, default = 0.0)</td>
</tr>
<tr>
<td>\textbf{'-cMass' (str)}</td>
<td>to form consistent mass matrix (optional, default = lumped mass matrix)</td>
</tr>
</tbody>
</table>

Notes

Beam With Hinges Element

This command is used to construct a \textit{forceBeamColumn} element object, which is based on the non-iterative (or iterative) flexibility formulation. The locations and weights of the element integration points are based on so-called plastic hinge integration, which allows the user to specify plastic hinge lengths at the element ends. Two-point Gauss integration is used on the element interior while two-point Gauss-Radau integration is applied over lengths of 4LpI and 4LpJ at the element ends, viz. “modified Gauss-Radau plastic hinge integration”. A total of six integration points are used in the element state determination (two for each hinge and two for the interior).

Users may be familiar with the beamWithHinges command format (see below); however, the format shown here allows for the simple but important case of using a material nonlinear section model on the element interior. The previous beamWithHinges command constrained the user to an elastic interior, which often led to unconservative estimates of the element resisting force when plasticity spread beyond the plastic hinge regions in to the element interior.

The advantages of this new format over the previous beamWithHinges command are:

- Plasticity can spread beyond the plastic hinge regions
- Hinges can form on the element interior, e.g., due to distributed member loads
To create a beam element with hinges, one has to use a `forceBeamColumn` element with following `beamIntegration()`.

**Note:**
- 'HingeRadau' – two-point Gauss-Radau applied to the hinge regions over 4LpI and 4LpJ (six element integration points)
- 'HingeRadauTwo' – two-point Gauss-Radau in the hinge regions applied over LpI and LpJ (six element integration points)
- 'HingeMidpoint' – midpoint integration over the hinge regions (four element integration points)
- 'HingeEndpoint' – endpoint integration over the hinge regions (four element integration points)

**See also:**
For more information on the behavior, advantages, and disadvantages of these approaches to plastic hinge integration, see


The primary advantages of HingeRadau are
- The user can specify a physically meaningful plastic hinge length
- The largest bending moment is captured at the element ends
- The exact numerical solution is recovered for a linear-elastic prismatic beam
- The characteristic length is equal to the user-specified plastic hinge length when deformations localize at the element ends

while the primary disadvantages are
- The element post-yield response is too flexible for strain-hardening section response (consider using HingeRadauTwo)
- The user needs to know the plastic hinge length a priori (empirical equations are available)

### dispBeamColumn

**element** (`'dispBeamColumn'`, `eleTag`, `iNode`, `jNode`, `transfTag`, `integrationTag`, `'cMass'`, `'mass'`, `mass=0.0`)  
Create a ForceBeamColumn element.

<table>
<thead>
<tr>
<th>Parameter</th>
<th>Description</th>
</tr>
</thead>
<tbody>
<tr>
<td><code>eleTag</code> (int)</td>
<td>tag of the element</td>
</tr>
<tr>
<td><code>iNode</code> (int)</td>
<td>tag of node i</td>
</tr>
<tr>
<td><code>jNode</code> (int)</td>
<td>tag of node j</td>
</tr>
<tr>
<td><code>transfTag</code> (int)</td>
<td>tag of transformation</td>
</tr>
<tr>
<td><code>integrationTag</code> (int)</td>
<td>tag of <code>beamIntegration()</code></td>
</tr>
<tr>
<td><code>'cMass'</code></td>
<td>to form consistent mass matrix (optional, default = lumped mass matrix)</td>
</tr>
<tr>
<td><code>mass</code> (float)</td>
<td>element mass density (per unit length), from which a lumped-mass matrix is formed (optional)</td>
</tr>
</tbody>
</table>
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forceBeamColumn

**element** (‘forceBeamColumn’, eleTag, iNode, jNode, transfTag, integrationTag, -iter, maxIter=10, tol=1e-12, -mass, mass=0.0)

Create a ForceBeamColumn element.

<table>
<thead>
<tr>
<th>Parameter</th>
<th>Description</th>
</tr>
</thead>
<tbody>
<tr>
<td>eleTag (int)</td>
<td>tag of the element</td>
</tr>
<tr>
<td>iNode (int)</td>
<td>tag of node i</td>
</tr>
<tr>
<td>jNode (int)</td>
<td>tag of node j</td>
</tr>
<tr>
<td>transfTag (int)</td>
<td>tag of transformation</td>
</tr>
<tr>
<td>integrationTag (int)</td>
<td>tag of beamIntegration()</td>
</tr>
<tr>
<td>maxIter (float)</td>
<td>maximum number of iterations to undertake to satisfy element compatibility (optional)</td>
</tr>
<tr>
<td>tol (float)</td>
<td>tolerance for satisfaction of element compatibility (optional)</td>
</tr>
<tr>
<td>mass (float)</td>
<td>element mass density (per unit length), from which a lumped-mass matrix is formed (optional)</td>
</tr>
</tbody>
</table>

nonlinearBeamColumn

**element** (‘nonlinearBeamColumn’, eleTag, iNode, jNode, numIntgrPts, secTag, transfTag, -iter, maxIter=10, tol=1e-12, -mass, mass=0.0, -integration, intType)

Create a nonlinearBeamColumn element. This element is for backward compatibility.

<table>
<thead>
<tr>
<th>Parameter</th>
<th>Description</th>
</tr>
</thead>
<tbody>
<tr>
<td>eleTag (int)</td>
<td>tag of the element</td>
</tr>
<tr>
<td>iNode (int)</td>
<td>tag of node i</td>
</tr>
<tr>
<td>jNode (int)</td>
<td>tag of node j</td>
</tr>
<tr>
<td>numIntgrPts (int)</td>
<td>number of integration points.</td>
</tr>
<tr>
<td>secTag (int)</td>
<td>tag of section</td>
</tr>
<tr>
<td>transfTag (int)</td>
<td>tag of transformation</td>
</tr>
<tr>
<td>maxIter (float)</td>
<td>maximum number of iterations to undertake to satisfy element compatibility (optional)</td>
</tr>
<tr>
<td>tol (float)</td>
<td>tolerance for satisfaction of element compatibility (optional)</td>
</tr>
<tr>
<td>mass (float)</td>
<td>element mass density (per unit length), from which a lumped-mass matrix is formed (optional)</td>
</tr>
<tr>
<td>intType (str)</td>
<td>integration type (optional, default is 'Lobatto')</td>
</tr>
<tr>
<td></td>
<td>• 'Lobatto'</td>
</tr>
<tr>
<td></td>
<td>• 'Legendre'</td>
</tr>
<tr>
<td></td>
<td>• 'Radau'</td>
</tr>
<tr>
<td></td>
<td>• 'NewtonCotes'</td>
</tr>
<tr>
<td></td>
<td>• 'Trapezoidal'</td>
</tr>
</tbody>
</table>

Flexure-Shear Interaction Displacement-Based Beam-Column Element

This command is used to construct a dispBeamColumnInt element object, which is a distributed-plasticity, displacement-based beam-column element which includes interaction between flexural and shear components.

**element** (‘dispBeamColumnInt’, eleTag, *eleNodes, numIntgrPts, secTag, transfTag, cRot ['-mass', mass-Dens])
**eleTag** (int) | unique element object tag  
--- | ---  
**eleNodes** (list (int)) | a list of two element nodes  
**numIntgrPts** (int) | number of integration points along the element.  
**secTag** (int) | identifier for previously-defined section object  
**transfTag** (int) | identifier for previously-defined coordinate-transformation (CrdTransf) object  
**cRot** (float) | identifier for element center of rotation (or center of curvature distribution). Fraction of the height distance from bottom to the center of rotation (0 to 1)  
**massDens** (float) | element mass density (per unit length), from which a lumped-mass matrix is formed (optional, default=0.0)

**See also:**

**Notes**

**MVLEM - Multiple-Vertical-Line-Element-Model for RC Walls**

The MVLEM element command is used to generate a two-dimensional Multiple-Vertical-Line-Element-Model (MVLEM; Vulcano et al., 1988; Orakcal et al., 2004, Kolozvari et al., 2015) for simulation of flexure-dominated RC wall behavior. A single model element incorporates six global degrees of freedom, three of each located at the center of rigid top and bottom beams, as illustrated in Figure 1a. The axial/flexural response of the MVLEM is simulated by a series of uniaxial elements (or macro-fibers) connected to the rigid beams at the top and bottom (e.g., floor) levels, whereas the shear response is described by a shear spring located at height ch from the bottom of the wall element (Figure 1a). Shear and flexural responses of the model element are uncoupled. The relative rotation between top and bottom faces of the wall element occurs about the point located on the central axis of the element at height ch (Figure 1b). Rotations and resulting transverse displacements are calculated based on the wall curvature, derived from section and material properties, corresponding to the bending moment at height ch of each element (Figure 1b). A value of c=0.4 was recommended by Vulcano et al. (1988) based on comparison of the model response with experimental results.

```python
element('MVLEM', eleTag, Dens, *eleNodes, m, c, -thick', *Thicknesses, -width', *Widths, -rho', *Reinforcing_ratios, -matConcrete', *Concrete_tags, -matSteel', *Steel_tags, -matShear', Shear_tag)
```

**eleTag** (int) | unique element object tag  
--- | ---  
**Dens** (float) | Wall density  
**eleNodes** (list (int)) | a list of two element nodes  
**m** (int) | Number of element macro-fibers  
**c** (float) | Location of center of rotation from the iNode, c = 0.4 (recommended)  
**Thicknesses** (list (float)) | a list of m macro-fiber thicknesses  
**Widths** (list (float)) | a list of m macro-fiber widths  
**Reinforcing_ratios** (list (float)) | a list of m reinforcing ratios corresponding to macro-fibers; for each fiber: \( \rho_i = A_{s,i} / A_{gross,i} \) (1 < i < m)  
**Concrete_tags** (list (int)) | a list of m uniaxialMaterial tags for concrete  
**Steel_tags** (list (int)) | a list of m uniaxialMaterial tags for steel  
**Shear_tag** (int) | Tag of uniaxialMaterial for shear material
SFI MVLEM - Cyclic Shear-Flexure Interaction Model for RC Walls

The SFI_MVLEM command is used to construct a Shear-Flexure Interaction Multiple-Vertical-Line-Element Model (SFI-MVLEM, Kolozvari et al., 2015a, b, c), which captures interaction between axial/flexural and shear behavior of RC structural walls and columns under cyclic loading. The SFI_MVLEM element (Figure 1) incorporates 2-D RC panel behavior described by the Fixed-Strut-Angle-Model (nDMaterial FSAM; Ulugtekin, 2010; Orakcal et al., 2012), into a 2-D macroscopic fiber-based model (MVLEM). The interaction between axial and shear behavior is captured at each RC panel (macro-fiber) level, which further incorporates interaction between shear and flexural behavior at the SFI_MVLEM element level.

\[ \text{element}(\text{"}, \text{eleTag}, \text{*eleNodes}, m, c, \text{','-thick'}, \text{*Thicknesses}, \text{','-width'}, \text{*Widths}, \text{','-mat'}, \text{*Material_tags}) \]

<table>
<thead>
<tr>
<th>Parameter</th>
<th>Description</th>
</tr>
</thead>
<tbody>
<tr>
<td>eleTag (int)</td>
<td>unique element object tag</td>
</tr>
<tr>
<td>eleNodes (list (int))</td>
<td>a list of two element nodes</td>
</tr>
<tr>
<td>m (int)</td>
<td>Number of element macro-fibers</td>
</tr>
<tr>
<td>c (float)</td>
<td>Location of center of rotation with from the iNode, ( c = 0.4 ) (recommended)</td>
</tr>
<tr>
<td>Thicknesses (list (float))</td>
<td>a list of m macro-fiber thicknesses</td>
</tr>
<tr>
<td>Widths (list (float))</td>
<td>a list of m macro-fiber widths</td>
</tr>
<tr>
<td>Material_tags (list (int))</td>
<td>a list of m macro-fiber nDMaterial1 tags</td>
</tr>
</tbody>
</table>

See also:

Notes

Joint Elements

1. **BeamColumnJoint Element**
2. **ElasticTubularJoint Element**
3. **Joint2D Element**

**BeamColumnJoint Element**

This command is used to construct a two-dimensional beam-column-joint element object. The element may be used with both two-dimensional and three-dimensional structures; however, load is transferred only in the plane of the element.

\[ \text{element}('\text{beamColumnJoint'}, \text{eleTag}, \text{*eleNodes}, \text{Mat1}, \text{Mat2}, \text{Mat3}, \text{Mat4}, \text{Mat5}, \text{Mat6}, \text{Mat7}, \text{Mat8}, \text{Mat9}, \text{Mat10}, \text{Mat11}, \text{Mat12}, \text{Mat13}[\text{, eleHeightFac=1.0, eleWidthFac=1.0}]) \]
ElasticTubularJoint Element

This command is used to construct an ElasticTubularJoint element object, which models joint flexibility of tubular joints in two dimensional analysis of any structure having tubular joints.

```python
element('ElasticTubularJoint', eleTag, *eleNodes, Brace_Diameter, Brace_Angle, E, Chord_Diameter, Chord_Thickness, Chord_Angle)
```

<table>
<thead>
<tr>
<th>Parameter</th>
<th>Description</th>
</tr>
</thead>
<tbody>
<tr>
<td>eleTag (int)</td>
<td>unique element object tag</td>
</tr>
<tr>
<td>eleNodes (list (int))</td>
<td>a list of two element nodes</td>
</tr>
<tr>
<td>Brace_Diameter (float)</td>
<td>outer diameter of brace</td>
</tr>
<tr>
<td>Brace_Angle (float)</td>
<td>angle between brace and chord axis $0 &lt; \text{Brace_Angle} &lt; 90$</td>
</tr>
<tr>
<td>E (float)</td>
<td>Young’s Modulus</td>
</tr>
<tr>
<td>Chord_Diameter (float)</td>
<td>outer diameter of chord</td>
</tr>
<tr>
<td>Chord_Thickness (float)</td>
<td>thickness of chord</td>
</tr>
<tr>
<td>Chord_Angle (float)</td>
<td>angle between chord axis and global x-axis $0 &lt; \text{Chord_Angle} &lt; 180$</td>
</tr>
</tbody>
</table>

See also:

Notes
Joint2D Element

This command is used to construct a two-dimensional beam-column-joint element object. The two dimensional beam-column joint is idealized as a parallelogram shaped shear panel with adjacent elements connected to its mid-points. The midpoints of the parallelogram are referred to as external nodes. These nodes are the only analysis components that connect the joint element to the surrounding structure.

\[ \text{element ("Joint2D", eleTag, *eleNodes[, Mat1, Mat2, Mat3, Mat4 ], MatC, LrgDspTag[, '-damage', DmgTag  ])[", '-damage', Dmg1 Dmg2 Dmg3 Dmg4 DmgC ])} \]

<table>
<thead>
<tr>
<th>eleTag</th>
<th>unique element object tag</th>
</tr>
</thead>
<tbody>
<tr>
<td>eleNodes</td>
<td>a list of five element nodes = [nd1, nd2, nd3, nd4, ndC]. ndC is the central node of beam-column joint. (the tag ndC is used to generate the internal node, thus, the node should not exist in the domain or be used by any other node)</td>
</tr>
<tr>
<td>Mat1</td>
<td>uniaxial material tag for interface rotational spring at node 1. Use a zero tag to indicate the case that a beam-column element is rigidly framed to the joint. (optional)</td>
</tr>
<tr>
<td>Mat2</td>
<td>uniaxial material tag for interface rotational spring at node 2. Use a zero tag to indicate the case that a beam-column element is rigidly framed to the joint. (optional)</td>
</tr>
<tr>
<td>Mat3</td>
<td>uniaxial material tag for interface rotational spring at node 3. Use a zero tag to indicate the case that a beam-column element is rigidly framed to the joint. (optional)</td>
</tr>
<tr>
<td>Mat4</td>
<td>uniaxial material tag for interface rotational spring at node 4. Use a zero tag to indicate the case that a beam-column element is rigidly framed to the joint. (optional)</td>
</tr>
<tr>
<td>MatC</td>
<td>uniaxial material tag for rotational spring of the central node that describes shear panel behavior</td>
</tr>
<tr>
<td>LrgDspTag</td>
<td>an integer indicating the flag for considering large deformations: * 0 - for small deformations and constant geometry * 1 - for large deformations and time varying geometry * 2 - for large deformations ,time varying geometry and length correction</td>
</tr>
<tr>
<td>DmgTag</td>
<td>damage model tag</td>
</tr>
<tr>
<td>Dmg1</td>
<td>damage model tag for Mat1</td>
</tr>
<tr>
<td>Dmg2</td>
<td>damage model tag for Mat2</td>
</tr>
<tr>
<td>Dmg3</td>
<td>damage model tag for Mat3</td>
</tr>
<tr>
<td>Dmg4</td>
<td>damage model tag for Mat4</td>
</tr>
<tr>
<td>DmgC</td>
<td>panel damage model tag</td>
</tr>
</tbody>
</table>

See also:

Notes

Link Elements

1. Two Node Link Element
Two Node Link Element

This command is used to construct a twoNodeLink element object, which is defined by two nodes. The element can have zero or non-zero length. This element can have 1 to 6 degrees of freedom, where only the transverse and rotational degrees of freedom are coupled as long as the element has non-zero length. In addition, if the element length is larger than zero, the user can optionally specify how the P-Delta moments around the local x- and y-axis are distributed among a moment at node i, a moment at node j, and a shear couple. The sum of these three ratios is always equal to 1. In addition the shear center can be specified as a fraction of the element length from the iNode. The element does not contribute to the Rayleigh damping by default. If the element has non-zero length, the local x-axis is determined from the nodal geometry unless the optional x-axis vector is specified in which case the nodal geometry is ignored and the user-defined orientation is utilized. It is important to recognize that if this element has zero length, it does not consider the geometry as given by the nodal coordinates, but utilizes the user-defined orientation vectors to determine the directions of the springs.

```
element('twoNodeLink', eleTag, *eleNodes, '-mat', *matTags, '-dir', *dirs, '-orient', *vecx, *vecy, '-pDelta', *Mratio, '-shearDist', *sDratios, '-doRayleigh', '-mass', m)
```

<table>
<thead>
<tr>
<th>Parameter</th>
<th>Description</th>
</tr>
</thead>
<tbody>
<tr>
<td>eleTag (int)</td>
<td>unique element object tag</td>
</tr>
<tr>
<td>eleNodes (list (int))</td>
<td>a list of two element nodes</td>
</tr>
<tr>
<td>matTags (list (int))</td>
<td>a list of tags associated with previously-defined UniaxialMaterial objects</td>
</tr>
<tr>
<td>dirs (list (int))</td>
<td>a list of material directions:</td>
</tr>
<tr>
<td></td>
<td>• 2D-case: 1, 2 - translations along local x,y axes; 3 - rotation about local z axis</td>
</tr>
<tr>
<td></td>
<td>• 3D-case: 1, 2, 3 - translations along local x,y,z axes; 4, 5, 6 - rotations about local x,y,z axes</td>
</tr>
<tr>
<td>vecx (list (float))</td>
<td>vector components in global coordinates defining local x-axis (optional)</td>
</tr>
<tr>
<td>vecy (list (float))</td>
<td>vector components in global coordinates defining local y-axis (optional)</td>
</tr>
<tr>
<td>Mratios (list (float))</td>
<td>P-Delta moment contribution ratios, size of ratio vector is 2 for 2D-case and 4 for 3D-case (entries: [My_iNode, My_jNode, Mz_iNode, Mz_jNode]) My_iNode + My_jNode &lt;= 1.0, Mz_iNode + Mz_jNode &lt;= 1.0. Remaining P-Delta moments are resisted by shear couples. (optional)</td>
</tr>
<tr>
<td>sDratios (list (float))</td>
<td>shear distances from iNode as a fraction of the element length, size of ratio vector is 1 for 2D-case and 2 for 3D-case. (entries: [dy_iNode, dz_iNode])(optional, default = [0.5, 0.5])</td>
</tr>
<tr>
<td>'-doRayleigh' (str)</td>
<td>to include Rayleigh damping from the element (optional, default = no Rayleigh damping contribution)</td>
</tr>
<tr>
<td>m (float)</td>
<td>element mass (optional, default = 0.0)</td>
</tr>
</tbody>
</table>

See also:

Notes

Bearing Elements

1. Elastomeric Bearing (Plasticity) Element
2. Elastomeric Bearing (Bouc-Wen) Element
3. Flat Slider Bearing Element
4. Single Friction Pendulum Bearing Element
5. Triple Friction Pendulum Bearing Element
6. Triple Friction Pendulum Element
7. MultipleShearSpring Element
8. KikuchiBearing Element
9. YamamotoBiaxialHDR Element
10. ElastomericX
11. LeadRubberX
12. HDR
13. RJ-Watson EQS Bearing Element
14. FPBearingPTV

Elastomeric Bearing (Plasticity) Element

This command is used to construct an elastomericBearing element object, which is defined by two nodes. The element can have zero length or the appropriate bearing height. The bearing has unidirectional (2D) or coupled (3D) plasticity properties for the shear deformations, and force-deformation behaviors defined by UniaxialMaterials in the remaining two (2D) or four (3D) directions. By default (sDratio = 0.5) P-Delta moments are equally distributed to the two end-nodes. To avoid the introduction of artificial viscous damping in the isolation system (sometimes referred to as “damping leakage in the isolation system”), the bearing element does not contribute to the Rayleigh damping by default. If the element has non-zero length, the local x-axis is determined from the nodal geometry unless the optional x-axis vector is specified in which case the nodal geometry is ignored and the user-defined orientation is utilized.

```python
for element in elements:
    element('elastomericBearingPlasticity', eleTag, *eleNodes, kInit, qd, alpha1, alpha2, mu, '-P', matTag, '-Mz', matTag, '-orient', x1, x2, x3, y1, y2, y3, '-shearDist', sDratio, '-doRayleigh', '-mass', m)
```

For a two-dimensional problem

```python
for element in elements:
    element('elastomericBearingPlasticity', eleTag, *eleNodes, kInit, qd, alpha1, alpha2, mu, '-P', matTag, '-T', matTag, '-My', matTag, '-Mz', matTag, '-orient', x1, x2, x3, y1, y2, y3, '-shearDist', sDratio, '-doRayleigh', '-mass', m)
```

For a three-dimensional problem
eleTag (int) | unique element object tag
---|---
eleNodes (list (int)) | a list of two element nodes
kInit (float) | initial elastic stiffness in local shear direction
qd (float) | characteristic strength
alpha1 (float) | post yield stiffness ratio of linear hardening component
alpha2 (float) | post yield stiffness ratio of non-linear hardening component
mu (float) | exponent of non-linear hardening component
'-P' matTag (int) | tag associated with previously-defined UniaxialMaterial in axial direction
'-T' matTag (int) | tag associated with previously-defined UniaxialMaterial in torsional direction
'-My' matTag (int) | tag associated with previously-defined UniaxialMaterial in moment direction around local y-axis
'-Mz' matTag (int) | tag associated with previously-defined UniaxialMaterial in moment direction around local z-axis
x1 x2 x3 (float) | vector components in global coordinates defining local x-axis (optional)
y1 y2 y3 (float) | vector components in global coordinates defining local y-axis (optional)
sDratio (float) | shear distance from iNode as a fraction of the element length (optional, default = 0.5)
'-doRayleigh' (str) | to include Rayleigh damping from the bearing (optional, default = no Rayleigh damping contribution)
m (float) | element mass (optional, default = 0.0)

See also:

Notes

Elastomeric Bearing (Bouc-Wen) Element

This command is used to construct an elastomericBearing element object, which is defined by two nodes. The element can have zero length or the appropriate bearing height. The bearing has unidirectional (2D) or coupled (3D) plasticity properties for the shear deformations, and force-deformation behaviors defined by UniaxialMaterials in the remaining two (2D) or four (3D) directions. By default (sDratio = 0.5) P-Delta moments are equally distributed to the two end-nodes. To avoid the introduction of artificial viscous damping in the isolation system (sometimes referred to as “damping leakage in the isolation system”), the bearing element does not contribute to the Rayleigh damping by default.

For a two-dimensional problem

```
element ('elastomericBearingBoucWen', eleTag, *eleNodes, kInit, qd, alpha1, alpha2, mu, eta, beta, gamma
            '-P', matTag '-Mz', matTag ['-orient', x1, x2, x3, y1, y2, y3], '-shearDist', sDratio]
            ['-doRayleigh'] ['-', 'mass', m ]
```

For a three-dimensional problem

```
element ('elastomericBearingBoucWen', eleTag, *eleNodes, kInit, qd, alpha1, alpha2, mu, eta, beat, gamma,
            '-P', matTag '-T', matTag '-My', matTag '-Mz', matTag ['-orient', x1, x2, x3], y1, y2, y3]
            ['-shearDist', sDratio], ['-doRayleigh'] ['-', 'mass', m ]
```

For a three-dimensional problem
<table>
<thead>
<tr>
<th>eleTag (int)</th>
<th>unique element object tag</th>
</tr>
</thead>
<tbody>
<tr>
<td>eleNodes (list of int)</td>
<td>a list of two element nodes</td>
</tr>
<tr>
<td>kInit (float)</td>
<td>initial elastic stiffness in local shear direction</td>
</tr>
<tr>
<td>qd (float)</td>
<td>characteristic strength</td>
</tr>
<tr>
<td>alpha1 (float)</td>
<td>post yield stiffness ratio of linear hardening component</td>
</tr>
<tr>
<td>alpha2 (float)</td>
<td>post yield stiffness ratio of non-linear hardening component</td>
</tr>
<tr>
<td>mu (float)</td>
<td>exponent of non-linear hardening component</td>
</tr>
<tr>
<td>eta (float)</td>
<td>yielding exponent (sharpness of hysteresis loop corners) (default = 1.0)</td>
</tr>
<tr>
<td>beta (float)</td>
<td>first hysteretic shape parameter (default = 0.5)</td>
</tr>
<tr>
<td>gamma (float)</td>
<td>second hysteretic shape parameter (default = 0.5)</td>
</tr>
<tr>
<td>'-P' matTag (int)</td>
<td>tag associated with previously-defined UniaxialMaterial in axial direction</td>
</tr>
<tr>
<td>'-T' matTag (int)</td>
<td>tag associated with previously-defined UniaxialMaterial in torsional direction</td>
</tr>
<tr>
<td>'-My' matTag (int)</td>
<td>tag associated with previously-defined UniaxialMaterial in moment direction around local y-axis</td>
</tr>
<tr>
<td>'-Mz' matTag (int)</td>
<td>tag associated with previously-defined UniaxialMaterial in moment direction around local z-axis</td>
</tr>
<tr>
<td>x1 x2 x3 (float)</td>
<td>vector components in global coordinates defining local x-axis (optional)</td>
</tr>
<tr>
<td>y1 y2 y3 (float)</td>
<td>vector components in global coordinates defining local y-axis (optional)</td>
</tr>
<tr>
<td>sDratio (float)</td>
<td>shear distance from iNode as a fraction of the element length (optional, default = 0.5)</td>
</tr>
<tr>
<td>'-doRayleigh' (str)</td>
<td>to include Rayleigh damping from the bearing (optional, default = no Rayleigh damping contribution)</td>
</tr>
<tr>
<td>m (float)</td>
<td>element mass (optional, default = 0.0)</td>
</tr>
</tbody>
</table>

See also:

Notes

Flat Slider Bearing Element

This command is used to construct a flatSliderBearing element object, which is defined by two nodes. The iNode represents the flat sliding surface and the jNode represents the slider. The element can have zero length or the appropriate bearing height. The bearing has unidirectional (2D) or coupled (3D) friction properties for the shear deformations, and force-deformation behaviors defined by UniaxialMaterials in the remaining two (2D) or four (3D) directions. To capture the uplift behavior of the bearing, the user-specified UniaxialMaterial in the axial direction is modified for no-tension behavior. By default (sDratio = 0.0) P-Delta moments are entirely transferred to the flat sliding surface (iNode). It is important to note that rotations of the flat sliding surface (rotations at the iNode) affect the shear behavior of the bearing. To avoid the introduction of artificial viscous damping in the isolation system (sometimes referred to as “damping leakage in the isolation system”), the bearing element does not contribute to the Rayleigh damping by default. If the element has non-zero length, the local x-axis is determined from the nodal geometry unless the optional x-axis vector is specified in which case the nodal geometry is ignored and the user-defined orientation is utilized.

```
element ('flatSliderBearing', eleTag, *eleNodes, frnMdlTag, kInit, '-P', matTag, '-Mz', matTag, '-orient', x1, x2, x3, y1, y2, y3, '-shearDist', sDratio, '-doRayleigh', '-mass', m, '-iter', maxIter, tol)
```

For a two-dimensional problem

```
element ('flatSliderBearing', eleTag, *eleNodes, frnMdlTag, kInit, '-P', matTag, '-T', matTag, '-My', matTag, '-Mz', matTag, '-orient', x1, x2, x3, y1, y2, y3, '-shearDist', sDratio, '-doRayleigh', '-mass', m, '-iter', maxIter, tol)
```

For a three-dimensional problem

1.4. Model Commands
eleTag (int) | unique element object tag
---|---
eleNodes (list (int)) | a list of two element nodes
frnMdlTag (float) | tag associated with previously-defined FrictionModel
kInit (float) | initial elastic stiffness in local shear direction
'-P' matTag (int) | tag associated with previously-defined UniaxialMaterial in axial direction
'-T' matTag (int) | tag associated with previously-defined UniaxialMaterial in torsional direction
'-My' matTag (int) | tag associated with previously-defined UniaxialMaterial in moment direction around local y-axis
'-Mz' matTag (int) | tag associated with previously-defined UniaxialMaterial in moment direction around local z-axis
x1 x2 x3 (float) | vector components in global coordinates defining local x-axis (optional)
y1 y2 y3 (float) | vector components in global coordinates defining local y-axis (optional)
sDratio (float) | shear distance from iNode as a fraction of the element length (optional, default = 0.0)
'-doRayleigh' (str) | to include Rayleigh damping from the bearing (optional, default = no Rayleigh damping contribution)
m (float) | element mass (optional, default = 0.0)
maxIter (int) | maximum number of iterations to undertake to satisfy element equilibrium (optional, default = 20)
tol (float) | convergence tolerance to satisfy element equilibrium (optional, default = 1E-8)

See also:

Notes

**Single Friction Pendulum Bearing Element**

This command is used to construct a singleFPBearing element object, which is defined by two nodes. The iNode represents the concave sliding surface and the jNode represents the articulated slider. The element can have zero length or the appropriate bearing height. The bearing has unidirectional (2D) or coupled (3D) friction properties (with post-yield stiffening due to the concave sliding surface) for the shear deformations, and force-deformation behaviors defined by UniaxialMaterials in the remaining two (2D) or four (3D) directions. To capture the uplift behavior of the bearing, the user-specified UniaxialMaterial in the axial direction is modified for no-tension behavior. By default (sDratio = 0.0) P-Delta moments are entirely transferred to the concave sliding surface (iNode). It is important to note that rotations of the concave sliding surface (rotations at the iNode) affect the shear behavior of the bearing. To avoid the introduction of artificial viscous damping in the isolation system (sometimes referred to as “damping leakage in the isolation system”), the bearing element does not contribute to the Rayleigh damping by default. If the element has non-zero length, the local x-axis is determined from the nodal geometry unless the optional x-axis vector is specified in which case the nodal geometry is ignored and the user-defined orientation is utilized.

```python
element ('singleFPBearing', eleTag, *eleNodes, frnMdlTag, Reff, kInit, '-P', matTag, '-My', matTag, '-Mz', matTag, '-orient', x1, x2, x3, y1, y2, y3, '-shearDist', sDratio, '-doRayleigh', 'mass', m, '-iter', 'maxIter', 'tol')
```

For a two-dimensional problem

```python
element ('singleFPBearing', eleTag, *eleNodes, frnMdlTag, Reff, kInit, '-P', matTag, '-T', matTag, '-My', matTag, '-Mz', matTag, '-orient', x1, x2, x3, y1, y2, y3, '-shearDist', sDratio, '-doRayleigh', 'mass', m, '-iter', 'maxIter', 'tol')
```

For a three-dimensional problem
This command is used to construct a Triple Friction Pendulum Bearing element object, which is defined by two nodes. The element can have zero length or the appropriate bearing height. The bearing has unidirectional (2D) or coupled (3D) friction properties (with post-yield stiffening due to the concave sliding surface) for the shear deformations, and force-deformation behaviors defined by UniaxialMaterials in the remaining two (2D) or four (3D) directions. To capture the uplift behavior of the bearing, the user-specified UniaxialMaterial in the axial direction is modified for no-tension behavior. P-Delta moments are entirely transferred to the concave sliding surface (iNode). It is important to note that rotations of the concave sliding surface (rotations at the iNode) affect the shear behavior of the bearing. If the element has non-zero length, the local x-axis is determined from the nodal geometry unless the optional x-axis vector is specified in which case the nodal geometry is ignored and the user-defined orientation is utilized.

```
element ("TFP", eleTag, *eleNodes, R1, R2, R3, R4, D1, D2, D3, D4, d1, d2, d3, d4, mu1, mu2, mu3, mu4,
        h1, h2, h3, h4, H0, colLoad[, K])
```

See also:

Notes
eleTag (int)  | unique element object tag  
-------------|---------------------------
eleNodes (list (int))  | a list of two element nodes  
R1 (float)  | Radius of inner bottom sliding surface  
R2 (float)  | Radius of inner top sliding surface  
R3 (float)  | Radius of outer bottom sliding surface  
R4 (float)  | Radius of outer top sliding surface  
D1 (float)  | Diameter of inner bottom sliding surface  
D2 (float)  | Diameter of inner top sliding surface  
D3 (float)  | Diameter of outer bottom sliding surface  
D4 (float)  | Diameter of outer top sliding surface  
d1 (float)  | diameter of inner slider  
d2 (float)  | diameter of inner slider  
d3 (float)  | diameter of outer bottom slider  
d4 (float)  | diameter of outer top slider  
mu1 (float)  | friction coefficient of inner bottom sliding surface  
mu2 (float)  | friction coefficient of inner top sliding surface  
mu3 (float)  | friction coefficient of outer bottom sliding surface  
mu4 (float)  | friction coefficient of outer top sliding surface  
h1 (float)  | height from inner bottom sliding surface to center of bearing  
h2 (float)  | height from inner top sliding surface to center of bearing  
h3 (float)  | height from outer bottom sliding surface to center of bearing  
h4 (float)  | height from inner top sliding surface to center of bearing  
H0 (float)  | total height of bearing  
colLoad (float)  | initial axial load on bearing (only used for first time step then load come from model)  
K (float)  | optional, stiffness of spring in vertical dirn (dof 2 if ndm= 2, dof 3 if ndm = 3) (default=1.0e15)  

See also:

Notes

**Triple Friction Pendulum Element**

element ("TripleFrictionPendulum", eleTag, *eleNodes, frnTag1, frnTag2, frnTag3, vertMatTag, rotZMatTag, rotXMatTag, rotYMatTag, L1, L2, L3, d1, d2, d3, W, uy, kvt, minFv, tol)
<table>
<thead>
<tr>
<th>eleTag</th>
<th>unique element object tag (int)</th>
</tr>
</thead>
<tbody>
<tr>
<td>eleNodes</td>
<td>a list of two element nodes (list (int))</td>
</tr>
<tr>
<td>frnTag1, frnTag2, frnTag3</td>
<td>tags associated with previously-defined FrictionModels at the three sliding interfaces (int)</td>
</tr>
<tr>
<td>vertMatTag</td>
<td>Pre-defined material tag for COMPRESSION behavior of the bearing (int)</td>
</tr>
<tr>
<td>rotZMatTag, rotXMatTag, rotYMatTag</td>
<td>Pre-defined material tags for rotational behavior about 3-axis, 1-axis and 2-axis, respectively. (int)</td>
</tr>
<tr>
<td>L1, L2, L3</td>
<td>effective radii. Li = R_i - h_i (see Figure 1) (float)</td>
</tr>
<tr>
<td>d1, d2, d3</td>
<td>displacement limits of pendulums (Figure 1). Displacement limit of the bearing is 2 d1 + d2 + d3 + L1. d3/L3 - L1. d2/L2 (float)</td>
</tr>
<tr>
<td>W</td>
<td>axial force used for the first trial of the first analysis step. (float)</td>
</tr>
<tr>
<td>uy</td>
<td>lateral displacement where sliding of the bearing starts. Recommended value = 0.25 to 1 mm. A smaller value may cause convergence problem. (float)</td>
</tr>
<tr>
<td>kvvt</td>
<td>Tension stiffness k_vt of the bearing. (float)</td>
</tr>
<tr>
<td>minFv</td>
<td>minimum vertical compression force in the bearing used for computing the horizontal tangent stiffness matrix from the normalized tangent stiffness matrix of the element. minFv is substituted for the actual compressive force when it is less than minFv, and prevents the element from using a negative stiffness matrix in the horizontal direction when uplift occurs. The vertical nodal force returned to nodes is always computed from kvc (or kvt) and vertical deformation, and thus is not affected by minFv. (float)</td>
</tr>
<tr>
<td>tol</td>
<td>relative tolerance for checking the convergence of the element. Recommended value = 1.e-10 to 1.e-3. (float)</td>
</tr>
</tbody>
</table>

See also:

Notes

**MultipleShearSpring Element**

This command is used to construct a multipleShearSpring (MSS) element object, which is defined by two nodes. This element consists of a series of identical shear springs arranged radially to represent the isotropic behavior in the local y-z plane.

```python
element ('multipleShearSpring', eleTag, *eleNodes, nSpring, '-mat', matTag[, '-lim', dsp][, '-orient'[, x1, x2, x3 ], yp1, yp2, yp3 ][, '-mass', m ])
```
eleTag (int): unique element object tag

eleNodes (list of int): a list of two element nodes

nSpring (int): number of springs

matTag (int): tag associated with previously-defined UniaxialMaterial object

dsp (float): minimum deformation to calculate equivalent coefficient (see note 1)

x1 x2 x3 (float): vector components in global coordinates defining local x-axis

yp1 yp2 yp3 (float): vector components in global coordinates defining vector yp which lies in the local x-y plane for the element

m (float): element mass

Note: If dsp is positive and the shear deformation of MSS exceeds dsp, this element calculates equivalent coefficient to adjust force and stiffness of MSS. The adjusted MSS force and stiffness reproduce the behavior of the previously defined uniaxial material under monotonic loading in every direction. If dsp is zero, the element does not calculate the equivalent coefficient.

See also:

Notes

KikuchiBearing Element

This command is used to construct a KikuchiBearing element object, which is defined by two nodes. This element consists of multiple shear spring model (MSS) and multiple normal spring model (MNS).

```python
element ('KikuchiBearing', eleTag, 'eleNodes', '-shape', shape, '-size', size, totalRubber, '-totalHeight', totalHeight, '-nMSS', nMSS, '-matMSS', matMSSTag, '-limDisp', limDisp, '-nMNS', nMNS, '-matMNS', matMNSTag, '-lambda', lambda, '-orient', x1, x2, x3, yp1, yp2, yp3, '-mass', m, '-noPDInput', '-noTilt', '-adjustPDOutput', ci, cj, '-doBalance', limFo, limFi, nIter)
```
<table>
<thead>
<tr>
<th>eleTag (int)</th>
<th>unique element object tag</th>
</tr>
</thead>
<tbody>
<tr>
<td>eleNodes (list (int))</td>
<td>a list of two element nodes</td>
</tr>
<tr>
<td>shape (float)</td>
<td>following shapes are available: round, square</td>
</tr>
<tr>
<td>size (float)</td>
<td>diameter (round shape), length of edge (square shape)</td>
</tr>
<tr>
<td>totalRubber (float)</td>
<td>total rubber thickness</td>
</tr>
<tr>
<td>totalHeight (float)</td>
<td>total height of the bearing (default: distance between iNode and jNode)</td>
</tr>
<tr>
<td>nMSS (int)</td>
<td>number of springs in MSS = nMSS</td>
</tr>
<tr>
<td>matMSSTag (int)</td>
<td>matTag for MSS</td>
</tr>
<tr>
<td>limDisp (float)</td>
<td>minimum deformation to calculate equivalent coefficient of MSS (see note 1)</td>
</tr>
<tr>
<td>nMNS (int)</td>
<td>number of springs in MNS = nMNS*nMNS (for round and square shape)</td>
</tr>
<tr>
<td>matMNSTag (int)</td>
<td>matTag for MNS</td>
</tr>
<tr>
<td>lambda (float)</td>
<td>parameter to calculate compression modulus distribution on MNS (see note 2)</td>
</tr>
<tr>
<td>x1 x2 x3 (float)</td>
<td>vector components in global coordinates defining local x-axis</td>
</tr>
<tr>
<td>yp1 yp2 yp3 (float)</td>
<td>vector components in global coordinates defining vector yp which lies in the local x-y plane for the element</td>
</tr>
<tr>
<td>m (float)</td>
<td>element mass</td>
</tr>
<tr>
<td>'-noPDInput' (str)</td>
<td>not consider P-Delta moment</td>
</tr>
<tr>
<td>'-noTilt' (str)</td>
<td>not consider tilt of rigid link</td>
</tr>
<tr>
<td>ci cj (float)</td>
<td>P-Delta moment adjustment for reaction force (default: c1 =0.5, c2 =0.5)</td>
</tr>
<tr>
<td>limFo limFi nIter (float)</td>
<td>tolerance of external unbalanced force (limFo), tolerance of internal unbalanced force (limFi), number of iterations to get rid of internal unbalanced force (nIter)</td>
</tr>
</tbody>
</table>

**See also:**

**Notes**

**YamamotoBiaxialHDR Element**

This command is used to construct a YamamotoBiaxialHDR element object, which is defined by two nodes. This element can be used to represent the isotropic behavior of high-damping rubber bearing in the local y-z plane.

```python
element('YamamotoBiaxialHDR', eleTag, *eleNodes, Tp, DDo, DDi, Hr[, '-coRS', cr, cs ][, '-orient'[, x1, x2, x3 ], y1, y2, y3 ][, '-mass', m ])
```
eleTag (int) | unique element object tag
-----|---------------------
eleNodes (list (int)) | a list of two element nodes
Tp (int) | compound type = 1 : X0.6R manufactured by Bridgestone corporation.
DDo (float) | outer diameter [m]
DDi (float) | bore diameter [m]
Hr (float) | total thickness of rubber layer [m] Optional Data
crs (float) | coefficients for shear stress components of $\tau_r$ and $\tau_s$
x1 x2 x3 (float) | vector components in global coordinates defining local x-axis
yp1 yp2 yp3 (float) | vector components in global coordinates defining vector yp which lies in the local x-y plane for the element
m (float) | element mass [kg]

See also:

Notes

ElastomericX

This command is used to construct an ElastomericX bearing element object in three-dimension. The 3D continuum geometry of an elastomeric bearing is modeled as a 2-node, 12 DOF discrete element. This elements extends the formulation of Elastomeric_Bearing_(Bouc-Wen)_Element element. However, instead of the user providing material models as input arguments, it only requires geometric and material properties of an elastomeric bearing as arguments. The material models in six direction are formulated within the element from input arguments. The time-dependent values of mechanical properties (e.g., shear stiffness, buckling load capacity) can also be recorded using the “parameters” recorder.

`element('ElastomericX', eleTag, *eleNodes, Fy, alpha, Gr, Kbulk, D1, D2, ts, tr, n[[x1, x2, x3]], y1, y2, y3, kc, PhiM, sDratio, m, cd, tc, tag1, tag2, tag3, tag4)`

For 3D problem
eleTag (int)  | unique element object tag  
---|---
eleNodes  (list (int)) | a list of two element nodes  
Fy (float) | yield strength  
alpha (float) | post-yield stiffness ratio  
Gr (float) | shear modulus of elastomeric bearing  
Kbulk (float) | bulk modulus of rubber  
D1 (float) | internal diameter  
D2 (float) | outer diameter (excluding cover thickness)  
ts (float) | single steel shim layer thickness  
tr (float) | single rubber layer thickness  
n (int) | number of rubber layers  
x1 x2 x3 (float) | vector components in global coordinates defining local x-axis (optional)  
y1 y2 y3 (float) | vector components in global coordinates defining local y-axis (optional)  
kc (float) | cavitation parameter (optional, default = 10.0)  
PhiM (float) | damage parameter (optional, default = 0.5)  
ac (float) | strength reduction parameter (optional, default = 1.0)  
sDratio (float) | shear distance from iNode as a fraction of the element length (optional, default = 0.5)  
m (float) | element mass (optional, default = 0.0)  
cd (float) | viscous damping parameter (optional, default = 0.0)  
tc (float) | cover thickness (optional, default = 0.0)  
tag1 (float) | Tag to include cavitation and post-cavitation (optional, default = 0)  
tag2 (float) | Tag to include buckling load variation (optional, default = 0)  
tag3 (float) | Tag to include horizontal stiffness variation (optional, default = 0)  
tag4 (float) | Tag to include vertical stiffness variation (optional, default = 0)

**Note:** Because default values of heating parameters are in SI units, user must override the default heating parameters values if using Imperial units.

User should distinguish between yield strength of elastomeric bearing \( F_y \) and characteristic strength \( Q_d \): 

\[
Q_d = F_y \times (1 - \alpha)
\]

**See also:**

**Notes**

**LeadRubberX**

This command is used to construct a LeadRubberX bearing element object in three-dimension. The 3D continuum geometry of a lead rubber bearing is modeled as a 2-node, 12 DOF discrete element. It extends the formulation of ElastomericX by including strength degradation in lead rubber bearing due to heating of the lead-core. The LeadRubberX element requires only the geometric and material properties of an elastomeric bearing as arguments. The material models in six direction are formulated within the element from input arguments. The time-dependent values of mechanical properties (e.g., shear stiffness, buckling load capacity, temperature in the lead-core, yield strength) can also be recorded using the “parameters” recorder.

```python
element ('LeadRubberX', eleTag, *eleNodes, Fy, alpha, Gr, Kbulk, D1, D2, ts, tr, n[[], x1, x2, x3, y1, y2, y3, kc[[], PhiM[[], ac[[], sDratio[[], m[[], cd[[], tc[[], qL[[], cL[[], kS[[], aS[[], tag1[[], tag2[[], tag3[[], tag4[[], tag5[[])]
```
<table>
<thead>
<tr>
<th>Parameter</th>
<th>Description</th>
</tr>
</thead>
<tbody>
<tr>
<td><code>eleTag</code> (int)</td>
<td>unique element object tag</td>
</tr>
<tr>
<td><code>eleNodes</code> (list of int)</td>
<td>a list of two element nodes</td>
</tr>
<tr>
<td><code>Fy</code> (float)</td>
<td>yield strength</td>
</tr>
<tr>
<td><code>alpha</code> (float)</td>
<td>post-yield stiffness ratio</td>
</tr>
<tr>
<td><code>Gr</code> (float)</td>
<td>shear modulus of elastomeric bearing</td>
</tr>
<tr>
<td><code>Kbulk</code> (float)</td>
<td>bulk modulus of rubber</td>
</tr>
<tr>
<td><code>D1</code> (float)</td>
<td>internal diameter</td>
</tr>
<tr>
<td><code>D2</code> (float)</td>
<td>outer diameter (excluding cover thickness)</td>
</tr>
<tr>
<td><code>ts</code> (float)</td>
<td>single steel shim layer thickness</td>
</tr>
<tr>
<td><code>tr</code> (float)</td>
<td>single rubber layer thickness</td>
</tr>
<tr>
<td><code>n</code> (int)</td>
<td>number of rubber layers</td>
</tr>
<tr>
<td><code>x1</code> <code>x2</code> <code>x3</code> (float)</td>
<td>vector components in global coordinates defining local x-axis (optional)</td>
</tr>
<tr>
<td><code>y1</code> <code>y2</code> <code>y3</code> (float)</td>
<td>vector components in global coordinates defining local y-axis (optional)</td>
</tr>
<tr>
<td><code>kc</code> (float)</td>
<td>cavitation parameter (optional, default = 10.0)</td>
</tr>
<tr>
<td><code>PhiM</code> (float)</td>
<td>damage parameter (optional, default = 0.5)</td>
</tr>
<tr>
<td><code>ac</code> (float)</td>
<td>strength reduction parameter (optional, default = 1.0)</td>
</tr>
<tr>
<td><code>sDratio</code> (float)</td>
<td>shear distance from iNode as a fraction of the element length (optional, default = 0.5)</td>
</tr>
<tr>
<td><code>m</code> (float)</td>
<td>element mass (optional, default = 0.0)</td>
</tr>
<tr>
<td><code>cd</code> (float)</td>
<td>viscous damping parameter (optional, default = 0.0)</td>
</tr>
<tr>
<td><code>tc</code> (float)</td>
<td>cover thickness (optional, default = 0.0)</td>
</tr>
<tr>
<td><code>qL</code> (float)</td>
<td>density of lead (optional, default = 11200 kg/m³)</td>
</tr>
<tr>
<td><code>cL</code> (float)</td>
<td>specific heat of lead (optional, default = 130 N·m/kg oC)</td>
</tr>
<tr>
<td><code>kS</code> (float)</td>
<td>thermal conductivity of steel (optional, default = 50 W/m oC)</td>
</tr>
<tr>
<td><code>aS</code> (float)</td>
<td>thermal diffusivity of steel (optional, default = 1.41e-05 m²/s)</td>
</tr>
<tr>
<td><code>tag1</code> (int)</td>
<td>Tag to include cavitation and post-cavitation (optional, default = 0)</td>
</tr>
<tr>
<td><code>tag2</code> (int)</td>
<td>Tag to include buckling load variation (optional, default = 0)</td>
</tr>
<tr>
<td><code>tag3</code> (int)</td>
<td>Tag to include horizontal stiffness variation (optional, default = 0)</td>
</tr>
<tr>
<td><code>tag4</code> (int)</td>
<td>Tag to include vertical stiffness variation (optional, default = 0)</td>
</tr>
<tr>
<td><code>tag5</code> (int)</td>
<td>Tag to include strength degradation in shear due to heating of lead core (optional, default = 0)</td>
</tr>
</tbody>
</table>

Note: Because default values of heating parameters are in SI units, user must override the default heating parameters values if using Imperial units.

User should distinguish between yield strength of elastomeric bearing ($F_y$) and characteristic strength ($Q_d$): $Q_d = F_y \times (1 - \alpha \text{pha})$

See also:

Notes

HDR

This command is used to construct an HDR bearing element object in three-dimension. The 3D continuum geometry of an high damping rubber bearing is modeled as a 2-node, 12 DOF discrete element. This is the third element in the series of elements developed for analysis of base-isolated structures under extreme loading (others being ElastomericX and LeadRubberX). The major difference between HDR element with ElastomericX is the hysteresis model in shear. The HDR element uses a model proposed by Grant et al. (2004) to capture the shear behavior of a high damping rubber bearing. The time-dependent values of mechanical properties (e.g., vertical stiffness, buckling load capacity) can also be recorded using the “parameters” recorder.
element ('HDR', eleTag, *eleNodes, Gr, Kbulk, D1, D2, ts, tr, n, a1, a2, a3, b1, b2, b3, c1, c2, c3, c4[[[ x1, x2, x3], y1, y2, y3]], kc, PhiM, ac, sDratio, [ m ], [ tc ])

For 3D problem

<table>
<thead>
<tr>
<th>Parameters</th>
<th>Description</th>
</tr>
</thead>
<tbody>
<tr>
<td>eleTag (int)</td>
<td>unique element object tag</td>
</tr>
<tr>
<td>eleNodes (list (int))</td>
<td>a list of two element nodes</td>
</tr>
<tr>
<td>Gr (float)</td>
<td>shear modulus of elastomeric bearing</td>
</tr>
<tr>
<td>Kbulk (float)</td>
<td>bulk modulus of rubber</td>
</tr>
<tr>
<td>D1 (float)</td>
<td>internal diameter</td>
</tr>
<tr>
<td>D2 (float)</td>
<td>outer diameter (excluding cover thickness)</td>
</tr>
<tr>
<td>ts (float)</td>
<td>single steel shim layer thickness</td>
</tr>
<tr>
<td>tr (float)</td>
<td>single rubber layer thickness</td>
</tr>
<tr>
<td>n (int)</td>
<td>number of rubber layers</td>
</tr>
<tr>
<td>a1 a2 a3 b1 b2 b3 c1 c2 c3 c4 (float)</td>
<td>parameters of the Grant model</td>
</tr>
<tr>
<td>x1 x2 x3 (float)</td>
<td>vector components in global coordinates defining local x-axis (optional)</td>
</tr>
<tr>
<td>y1 y2 y3 (float)</td>
<td>vector components in global coordinates defining local y-axis (optional)</td>
</tr>
<tr>
<td>kc (float)</td>
<td>cavitation parameter (optional, default = 10.0)</td>
</tr>
<tr>
<td>PhiM (float)</td>
<td>damage parameter (optional, default = 0.5)</td>
</tr>
<tr>
<td>ac (float)</td>
<td>strength reduction parameter (optional, default = 1.0)</td>
</tr>
<tr>
<td>sDratio (float)</td>
<td>shear distance from iNode as a fraction of the element length (optional, default = 0.5)</td>
</tr>
<tr>
<td>m (float)</td>
<td>element mass (optional, default = 0.0)</td>
</tr>
<tr>
<td>tc (float)</td>
<td>cover thickness (optional, default = 0.0)</td>
</tr>
</tbody>
</table>

See also:

Notes

**RJ-Watson EQS Bearing Element**

This command is used to construct a RJWatsonEqsBearing element object, which is defined by two nodes. The iNode represents the masonry plate and the jNode represents the sliding surface plate. The element can have zero length or the appropriate bearing height. The bearing has unidirectional (2D) or coupled (3D) friction properties (with post-yield stiffening due to the mass-energy-regulator (MER) springs) for the shear deformations, and force-deformation behaviors defined by UniaxialMaterials in the remaining two (2D) or four (3D) directions. To capture the uplift behavior of the bearing, the user-specified UniaxialMaterial in the axial direction is modified for no-tension behavior. By default (sDratio = 1.0) P-Delta moments are entirely transferred to the sliding surface (jNode). It is important to note that rotations of the sliding surface (rotations at the jNode) affect the shear behavior of the bearing. To avoid the introduction of artificial viscous damping in the isolation system (sometimes referred to as “damping leakage in the isolation system”), the bearing element does not contribute to the Rayleigh damping by default. If the element has non-zero length, the local x-axis is determined from the nodal geometry unless the optional x-axis vector is specified in which case the nodal geometry is ignored and the user-defined orientation is utilized.

element ('RJWatsonEqsBearing', eleTag, *eleNodes, frnMdlTag, kInit, 'P', matTag, 'Vy', matTag, 'Mz', matTag, '-orient', x1, x2, x3, y1, y2, y3, 'shearDist', sDratio, '-doRayleigh', '-mass', m

For a two-dimensional problem

**element** ('RJWatsonEqsBearing', eleTag, *eleNodes, frnMdlTag, kInit, 'P', matTag, 'Vy', matTag, 'Vz', matTag, 'T', matTag, 'My', matTag, 'Mz', matTag, '-orient', x1, x2, x3, y1, y2, y3, 'shearDist', sDratio, '-doRayleigh', '-mass', m

1.4. Model Commands
For a three-dimensional problem

<table>
<thead>
<tr>
<th>Parameter</th>
<th>Description</th>
</tr>
</thead>
<tbody>
<tr>
<td>eleTag (int)</td>
<td>unique element object tag</td>
</tr>
<tr>
<td>eleNodes (list (int))</td>
<td>a list of two element nodes</td>
</tr>
<tr>
<td>frnMdlTag (float)</td>
<td>tag associated with previously-defined FrictionModel</td>
</tr>
<tr>
<td>kInit (float)</td>
<td>initial stiffness of sliding friction component in local shear direction</td>
</tr>
<tr>
<td>'-P' matTag (int)</td>
<td>tag associated with previously-defined UniaxialMaterial in axial direction</td>
</tr>
<tr>
<td>'-Vy' matTag (int)</td>
<td>tag associated with previously-defined UniaxialMaterial in shear direction along local y-axis (MER spring behavior not including friction)</td>
</tr>
<tr>
<td>'-Vz' matTag (int)</td>
<td>tag associated with previously-defined UniaxialMaterial in shear direction along local z-axis (MER spring behavior not including friction)</td>
</tr>
<tr>
<td>'-T' matTag (int)</td>
<td>tag associated with previously-defined UniaxialMaterial in torsional direction</td>
</tr>
<tr>
<td>'-My' matTag (int)</td>
<td>tag associated with previously-defined UniaxialMaterial in moment direction around local y-axis</td>
</tr>
<tr>
<td>'-Mz' matTag (int)</td>
<td>tag associated with previously-defined UniaxialMaterial in moment direction around local z-axis</td>
</tr>
<tr>
<td>x1 x2 x3 (float)</td>
<td>vector components in global coordinates defining local x-axis (optional)</td>
</tr>
<tr>
<td>y1 y2 y3 (float)</td>
<td>vector components in global coordinates defining local y-axis (optional)</td>
</tr>
<tr>
<td>sDratio (float)</td>
<td>shear distance from iNode as a fraction of the element length (optional, default = 0.0)</td>
</tr>
<tr>
<td>'-doRayleigh' (str)</td>
<td>to include Rayleigh damping from the bearing (optional, default = no Rayleigh damping contribution)</td>
</tr>
<tr>
<td>m (float)</td>
<td>element mass (optional, default = 0.0)</td>
</tr>
<tr>
<td>maxIter (int)</td>
<td>maximum number of iterations to undertake to satisfy element equilibrium (optional, default = 20)</td>
</tr>
<tr>
<td>tol (float)</td>
<td>convergence tolerance to satisfy element equilibrium (optional, default = 1E-8)</td>
</tr>
</tbody>
</table>

See also:

Notes

**FPBearingPTV**

The FPBearingPTV command creates a single Friction Pendulum bearing element, which is capable of accounting for the changes in the coefficient of friction at the sliding surface with instantaneous values of the sliding velocity, axial pressure and temperature at the sliding surface. The constitutive modelling is similar to the existing singleFPBearing element, otherwise. The FPBearingPTV element has been verified and validated in accordance with the ASME guidelines, details of which are presented in Chapter 4 of Kumar et al. (2015a).

```python
element ('FPBearingPTV', eleTag, *eleNodes, MuRef, IsPressureDependent, pRef, IsTemperatureDependent, Diffusivity, Conductivity, IsVelocityDependent, rateParameter, ReflectiveFP, Radius_Contact, kInitial, theMaterialA, theMaterialB, theMaterialC, theMaterialD, x1, x2, x3, y1, y2, y3, shearDist, doRayleigh, mass, iter, tol, unit)
```
<table>
<thead>
<tr>
<th>Parameter</th>
<th>Description</th>
</tr>
</thead>
<tbody>
<tr>
<td><code>eleTag</code> (int)</td>
<td>unique element object tag</td>
</tr>
<tr>
<td><code>eleNodes</code> (list (int))</td>
<td>a list of two element nodes</td>
</tr>
<tr>
<td><code>MuRef</code> (float)</td>
<td>Reference coefficient of friction</td>
</tr>
<tr>
<td><code>IsPressureDependent</code> (int)</td>
<td>1 if the coefficient of friction is a function of instantaneous axial pressure</td>
</tr>
<tr>
<td><code>pRef</code> (float)</td>
<td>Reference axial pressure (the bearing pressure under static loads)</td>
</tr>
<tr>
<td><code>IsTemperatureDependent</code> (int)</td>
<td>1 if the coefficient of friction is a function of instantaneous temperature at the sliding surface</td>
</tr>
<tr>
<td><code>Diffusivity</code> (float)</td>
<td>Thermal diffusivity of steel</td>
</tr>
<tr>
<td><code>Conductivity</code> (float)</td>
<td>Thermal conductivity of steel</td>
</tr>
<tr>
<td><code>IsVelocityDependent</code> (int)</td>
<td>1 if the coefficient of friction is a function of instantaneous velocity at the sliding surface</td>
</tr>
<tr>
<td><code>rateParameter</code> (float)</td>
<td>The exponent that determines the shape of the coefficient of friction vs. sliding velocity curve</td>
</tr>
<tr>
<td><code>ReffectiveFP</code> (float)</td>
<td>Effective radius of curvature of the sliding surface of the FP bearing</td>
</tr>
<tr>
<td><code>Radius_Contract</code> (float)</td>
<td>Radius of contact area at the sliding surface</td>
</tr>
<tr>
<td><code>kInitial</code> (float)</td>
<td>Lateral stiffness of the sliding bearing before sliding begins</td>
</tr>
<tr>
<td><code>theMaterialA</code> (int)</td>
<td>Tag for the uniaxial material in the axial direction</td>
</tr>
<tr>
<td><code>theMaterialB</code> (int)</td>
<td>Tag for the uniaxial material in the torsional direction</td>
</tr>
<tr>
<td><code>theMaterialC</code> (int)</td>
<td>Tag for the uniaxial material for rocking about local Y axis</td>
</tr>
<tr>
<td><code>theMaterialD</code> (int)</td>
<td>Tag for the uniaxial material for rocking about local Z axis</td>
</tr>
<tr>
<td><code>x1 x2 x3</code> (float)</td>
<td>Vector components to define local X axis</td>
</tr>
<tr>
<td><code>y1 y2 y3</code> (float)</td>
<td>Vector components to define local Y axis</td>
</tr>
<tr>
<td><code>shearDist</code> (float)</td>
<td>Shear distance from iNode as a fraction of the length of the element</td>
</tr>
<tr>
<td><code>doRayleigh</code> (int)</td>
<td>To include Rayleigh damping from the bearing</td>
</tr>
<tr>
<td><code>mass</code> (float)</td>
<td>Element mass</td>
</tr>
<tr>
<td><code>iter</code> (int)</td>
<td>Maximum number of iterations to satisfy the equilibrium of element</td>
</tr>
<tr>
<td><code>tol</code> (float)</td>
<td>Convergence tolerance to satisfy the equilibrium of the element</td>
</tr>
<tr>
<td><code>unit</code> (int)</td>
<td>Tag to identify the unit from the list below.</td>
</tr>
<tr>
<td></td>
<td>• 1: N, m, s, C</td>
</tr>
<tr>
<td></td>
<td>• 2: kN, m, s, C</td>
</tr>
<tr>
<td></td>
<td>• 3: N, mm, s, C</td>
</tr>
<tr>
<td></td>
<td>• 4: kN, mm, s, C</td>
</tr>
<tr>
<td></td>
<td>• 5: lb, in, s, C</td>
</tr>
<tr>
<td></td>
<td>• 6: kip, in, s, C</td>
</tr>
<tr>
<td></td>
<td>• 7: lb, ft, s, C</td>
</tr>
<tr>
<td></td>
<td>• 8: kip, ft, s, C</td>
</tr>
</tbody>
</table>

**See also:**

**Notes**
Quad Element

This command is used to construct a FourNodeQuad element object which uses a bilinear isoparametric formulation. 

```python
{element}( \text{'quad'}, \text{eleTag}, \ast\text{eleNodes}, \text{thick}, \text{type}, \text{matTag}[, \text{pressure}, \text{rho}, b1, b2 ])
```

<table>
<thead>
<tr>
<th>eleTag (int)</th>
<th>unique element object tag</th>
</tr>
</thead>
<tbody>
<tr>
<td>eleNodes (list (int))</td>
<td>a list of four element nodes in counter-clockwise order</td>
</tr>
<tr>
<td>thick (float)</td>
<td>element thickness</td>
</tr>
<tr>
<td>type (str)</td>
<td>string representing material behavior. The type parameter can be either 'PlaneStrain' or 'PlaneStress'</td>
</tr>
<tr>
<td>matTag (int)</td>
<td>tag of nDMaterial</td>
</tr>
<tr>
<td>pressure (float)</td>
<td>surface pressure (optional, default = 0.0)</td>
</tr>
<tr>
<td>rho (float)</td>
<td>element mass density (per unit volume) from which a lumped element mass matrix is computed (optional, default=0.0)</td>
</tr>
<tr>
<td>b1, b2 (float)</td>
<td>constant body forces defined in the isoparametric domain (optional, default=0.0)</td>
</tr>
</tbody>
</table>

Note:

1. Consistent nodal loads are computed from the pressure and body forces.
2. The valid queries to a Quad element when creating an ElementRecorder object are ‘forces’, ‘stresses,’ and ‘material $matNum matArg1 matArg2 …’ Where $matNum refers to the material object at the integration point corresponding to the node numbers in the isoparametric domain.

See also:

Notes
Shell Element

This command is used to construct a ShellMITC4 element object, which uses a bilinear isoparametric formulation in combination with a modified shear interpolation to improve thin-plate bending performance.

```python
element('ShellMITC4', eleTag, *eleNodes, secTag)
```

<table>
<thead>
<tr>
<th>eleTag</th>
<th>unique element object tag</th>
</tr>
</thead>
<tbody>
<tr>
<td>(int)</td>
<td></td>
</tr>
</tbody>
</table>

<table>
<thead>
<tr>
<th>eleNodes</th>
<th>a list of four element nodes in counter-clockwise order</th>
</tr>
</thead>
<tbody>
<tr>
<td>(list(int))</td>
<td></td>
</tr>
</tbody>
</table>

<table>
<thead>
<tr>
<th>secTag</th>
<th>tag associated with previously-defined SectionForceDeformation object. Currently must be either a 'PlateFiberSection', or 'ElasticMembranePlateSection'</th>
</tr>
</thead>
<tbody>
<tr>
<td>(int)</td>
<td></td>
</tr>
</tbody>
</table>

Note:

1. The valid queries to a Quad element when creating an ElementRecorder object are ‘forces’, ‘stresses,’ and ‘material $matNum matArg1 matArg2 … ’ Where $matNum refers to the material object at the integration point corresponding to the node numbers in the isoparametric domain.

2. It is a 3D element with 6 dofs and CAN NOT be used in 2D domain.

See also:

Notes

ShellDKGQ

This command is used to construct a ShellDKGQ element object, which is a quadrilateral shell element based on the theory of generalized conforming element.

```python
element('ShellDKGQ', eleTag, *eleNodes, secTag)
```

<table>
<thead>
<tr>
<th>eleTag</th>
<th>unique element object tag</th>
</tr>
</thead>
<tbody>
<tr>
<td>(int)</td>
<td></td>
</tr>
</tbody>
</table>

<table>
<thead>
<tr>
<th>eleNodes</th>
<th>a list of four element nodes in counter-clockwise order</th>
</tr>
</thead>
<tbody>
<tr>
<td>(list(int))</td>
<td></td>
</tr>
</tbody>
</table>

<table>
<thead>
<tr>
<th>secTag</th>
<th>tag associated with previously-defined SectionForceDeformation object. Currently can be a 'PlateFiberSection', a 'ElasticMembranePlateSection' and a 'LayeredShell' section</th>
</tr>
</thead>
<tbody>
<tr>
<td>(int)</td>
<td></td>
</tr>
</tbody>
</table>

See also:

Notes

ShellDKGT

This command is used to construct a ShellDKGT element object, which is a triangular shell element based on the theory of generalized conforming element.
**element** ('ShellDKGT', eleTag, *eleNodes, secTag)

<table>
<thead>
<tr>
<th>eleTag</th>
<th>unique element object tag</th>
</tr>
</thead>
<tbody>
<tr>
<td>(int)</td>
<td></td>
</tr>
<tr>
<td>eleNodes</td>
<td>a list of three element nodes in clockwise or counter-clockwise order</td>
</tr>
<tr>
<td>(list (int))</td>
<td></td>
</tr>
<tr>
<td>secTag</td>
<td>tag associated with previously-defined SectionForceDeformation object. currently can be a 'PlateFiberSection', a 'ElasticMembranePlateSection' and a 'LayeredShell' section</td>
</tr>
<tr>
<td>(int)</td>
<td></td>
</tr>
</tbody>
</table>

**See also:**
Notes

**ShellNLDKGQ**

This command is used to construct a ShellNLDKGQ element object accounting for the geometric nonlinearity of large deformation using the updated Lagrangian formula, which is developed based on the ShellDKGT element.

**element** ('ShellNLDKGQ', eleTag, *eleNodes, secTag)

<table>
<thead>
<tr>
<th>eleTag</th>
<th>unique element object tag</th>
</tr>
</thead>
<tbody>
<tr>
<td>(int)</td>
<td></td>
</tr>
<tr>
<td>eleNodes</td>
<td>a list of four element nodes in counter-clockwise order</td>
</tr>
<tr>
<td>(list (int))</td>
<td></td>
</tr>
<tr>
<td>secTag</td>
<td>tag associated with previously-defined SectionForceDeformation object. currently can be a 'PlateFiberSection', a 'ElasticMembranePlateSection' and a 'LayeredShell' section</td>
</tr>
<tr>
<td>(int)</td>
<td></td>
</tr>
</tbody>
</table>

**See also:**
Notes

**ShellNLDKGT**

This command is used to construct a ShellNLDKGT element object accounting for the geometric nonlinearity of large deformation using the updated Lagrangian formula, which is developed based on the ShellDKGT element.

**element** ('ShellNLDKGT', eleTag, *eleNodes, secTag)

<table>
<thead>
<tr>
<th>eleTag</th>
<th>unique element object tag</th>
</tr>
</thead>
<tbody>
<tr>
<td>(int)</td>
<td></td>
</tr>
<tr>
<td>eleNodes</td>
<td>a list of three element nodes in clockwise or counter-clockwise order around the element</td>
</tr>
<tr>
<td>(list (int))</td>
<td></td>
</tr>
<tr>
<td>secTag</td>
<td>tag associated with previously-defined SectionForceDeformation object. currently can be a 'PlateFiberSection', a 'ElasticMembranePlateSection' and a 'LayeredShell' section</td>
</tr>
<tr>
<td>(int)</td>
<td></td>
</tr>
</tbody>
</table>

**See also:**
Notes

**ShellNL**

`element` (<ShellNL>, eleTag, *eleNodes, secTag)

<table>
<thead>
<tr>
<th>Name</th>
<th>Description</th>
</tr>
</thead>
<tbody>
<tr>
<td>eleTag</td>
<td>unique element object tag</td>
</tr>
<tr>
<td>eleNodes</td>
<td>a list of nine element nodes, input is the typical, firstly four corner nodes counter-clockwise, then mid-side nodes counter-clockwise and finally the central node</td>
</tr>
<tr>
<td>secTag</td>
<td>tag associated with previously-defined SectionForceDeformation object. currently can be a 'PlateFiberSection', a 'ElasticMembranePlateSection' and a 'LayeredShell' section</td>
</tr>
</tbody>
</table>

See also:

Notes

**Bbar Plane Strain Quadrilateral Element**

This command is used to construct a four-node quadrilateral element object, which uses a bilinear isoparametric formulation along with a mixed volume/pressure B-bar assumption. This element is for plane strain problems only.

`element` (<bbarQuad>, eleTag, *eleNodes, thick, matTag)

<table>
<thead>
<tr>
<th>Name</th>
<th>Description</th>
</tr>
</thead>
<tbody>
<tr>
<td>eleTag</td>
<td>unique element object tag</td>
</tr>
<tr>
<td>eleNodes</td>
<td>a list of four element nodes in counter-clockwise order</td>
</tr>
<tr>
<td>thick</td>
<td>element thickness</td>
</tr>
<tr>
<td>matTag</td>
<td>tag of nDMaterial</td>
</tr>
</tbody>
</table>

**Note:**

1. PlainStrain only.

2. The valid queries to a Quad element when creating an ElementRecorder object are ‘forces’, ‘stresses,’ and ‘material $matNum matArg1 matArg2 ...’ Where $matNum refers to the material object at the integration point corresponding to the node numbers in the isoparametric domain.

See also:

Notes

**Enhanced Strain Quadrilateral Element**

This command is used to construct a four-node quadrilateral element, which uses a bilinear isoparametric formulation with enhanced strain modes.
element ('enhancedQuad', eleTag, *eleNodes, thick, type, matTag)

<table>
<thead>
<tr>
<th>eleTag (int)</th>
<th>unique element object tag</th>
</tr>
</thead>
<tbody>
<tr>
<td>eleNodes (list (int))</td>
<td>a list of four element nodes in counter-clockwise order</td>
</tr>
<tr>
<td>thick (float)</td>
<td>element thickness</td>
</tr>
<tr>
<td>type (str)</td>
<td>string representing material behavior. Valid options depend on the NDMaterial object and its available material formulations. The type parameter can be either 'PlaneStrain' or 'PlaneStress'</td>
</tr>
<tr>
<td>matTag (int)</td>
<td>tag of nDMaterial</td>
</tr>
</tbody>
</table>

See also:

Notes

SSPquad Element

This command is used to construct a SSPquad element object.

element ('SSPquad', eleTag, *eleNodes, matTag, type, thick[!, b1, b2])

<table>
<thead>
<tr>
<th>eleTag (int)</th>
<th>unique element object tag</th>
</tr>
</thead>
<tbody>
<tr>
<td>eleNodes (list (int))</td>
<td>a list of four element nodes in counter-clockwise order</td>
</tr>
<tr>
<td>thick (float)</td>
<td>thickness of the element in out-of-plane direction</td>
</tr>
<tr>
<td>type (str)</td>
<td>string to relay material behavior to the element, can be either 'PlaneStrain' or 'PlaneStress'</td>
</tr>
<tr>
<td>matTag (int)</td>
<td>unique integer tag associated with previously-defined nDMaterial object</td>
</tr>
<tr>
<td>b1 b2 (float)</td>
<td>constant body forces in global x- and y-directions, respectively (optional, default = 0.0)</td>
</tr>
</tbody>
</table>

The SSPquad element is a four-node quadrilateral element using physically stabilized single-point integration (SSP -> Stabilized Single Point). The stabilization incorporates an assumed strain field in which the volumetric dilation and the shear strain associated with the the hourglass modes are zero, resulting in an element which is free from volumetric and shear locking. The elimination of shear locking results in greater coarse mesh accuracy in bending dominated problems, and the elimination of volumetric locking improves accuracy in nearly-incompressible problems. Analysis times are generally faster than corresponding full integration elements. The formulation for this element is identical to the solid phase portion of the SSPquadUP element as described by McGann et al. (2012).

Note:

1. Valid queries to the SSPquad element when creating an ElementalRecorder object correspond to those for the nDMaterial object assigned to the element (e.g., ‘stress’, ‘strain’). Material response is recorded at the single integration point located in the center of the element.

2. The SSPquad element was designed with intentions of duplicating the functionality of the Quad Element. If an example is found where the SSPquad element cannot do something that works for the Quad Element, e.g., material updating, please contact the developers listed below so the bug can be fixed.
**Triangular Elements**

1. **Tri31 Element**

This command is used to construct a constant strain triangular element (Tri31) which uses three nodes and one integration points.

\[ \text{element} ('\text{Tri31}', \text{eleTag}, \text{*eleNodes}, \text{thick}, \text{type}, \text{matTag}, \text{pressure}, \text{rho}, \text{b1}, \text{b2}) \]

<table>
<thead>
<tr>
<th>parameter</th>
<th>description</th>
</tr>
</thead>
<tbody>
<tr>
<td>eleTag</td>
<td>unique element object tag</td>
</tr>
<tr>
<td>eleNodes</td>
<td>a list of three element nodes in counter-clockwise order</td>
</tr>
<tr>
<td>thick</td>
<td>element thickness</td>
</tr>
<tr>
<td>type</td>
<td>string representing material behavior. The type parameter can be either 'PlaneStrain' or 'PlaneStress'</td>
</tr>
<tr>
<td>matTag</td>
<td>tag of nDMaterial</td>
</tr>
<tr>
<td>pressure</td>
<td>surface pressure (optional, default = 0.0)</td>
</tr>
<tr>
<td>rho</td>
<td>element mass density (per unit volume) from which a lumped element mass matrix is computed (optional, default=0.0)</td>
</tr>
<tr>
<td>b1, b2</td>
<td>constant body forces defined in the domain (optional, default=0.0)</td>
</tr>
</tbody>
</table>

**Note:**

1. Consistent nodal loads are computed from the pressure and body forces.
2. The valid queries to a Tri31 element when creating an ElementRecorder object are ‘forces’, ‘stresses,’ and ‘material $matNum matArg1 matArg2 ...’ Where $matNum refers to the material object at the integration point corresponding to the node numbers in the domain.

**See also:**

Notes

**Brick Elements**

1. **Standard Brick Element**
2. **Bbar Brick Element**
3. **Twenty Node Brick Element**
4. **SSPbrick Element**
Standard Brick Element

This element is used to construct an eight-node brick element object, which uses a trilinear isoparametric formulation.

\texttt{element(\textquotesingle stdBrick\textquotesingle, eleTag, *eleNodes, matTag[, b1, b2, b3])}

<table>
<thead>
<tr>
<th>eleTag (int)</th>
<th>unique element object tag</th>
</tr>
</thead>
<tbody>
<tr>
<td>eleNodes (list(int))</td>
<td>a list of eight element nodes in bottom and top faces and in counter-clockwise order</td>
</tr>
<tr>
<td>matTag (int)</td>
<td>tag of nDMaterial</td>
</tr>
<tr>
<td>b1 b2 b3 (float)</td>
<td>body forces in global x,y,z directions</td>
</tr>
</tbody>
</table>

Note:

1. The valid queries to a Brick element when creating an ElementRecorder object are ‘forces’, ‘stresses,’ (‘strains’ version > 2.2.0) and ‘material $matNum matArg1 matArg2 . . .’ Where $matNum refers to the material object at the integration point corresponding to the node numbers in the isoparametric domain.

2. This element can only be defined in -ndm 3 -ndf 3

See also:

Notes

Bbar Brick Element

This command is used to construct an eight-node mixed volume/pressure brick element object, which uses a trilinear isoparametric formulation.

\texttt{element(\textquotesingle bbarBrick\textquotesingle, eleTag, *eleNodes, matTag[, b1, b2, b3])}

<table>
<thead>
<tr>
<th>eleTag (int)</th>
<th>unique element object tag</th>
</tr>
</thead>
<tbody>
<tr>
<td>eleNodes (list(int))</td>
<td>a list of eight element nodes in bottom and top faces and in counter-clockwise order</td>
</tr>
<tr>
<td>matTag (int)</td>
<td>tag of nDMaterial</td>
</tr>
<tr>
<td>b1 b2 b3 (float)</td>
<td>body forces in global x,y,z directions</td>
</tr>
</tbody>
</table>

Note:

1. Node numbering for this element is different from that for the eight-node brick (Brick8N) element.

2. The valid queries to a Quad element when creating an ElementRecorder object are ‘forces’, ‘stresses’, ‘strains’, and ‘material $matNum matArg1 matArg2 . . .’ Where $matNum refers to the material object at the integration point corresponding to the node numbers in the isoparametric domain.

See also:

Notes
Twenty Node Brick Element

The element is used to construct a twenty-node three dimensional element object

```
 element ('Brick20N', eleTag, *eleNodes, matTag, bf1, bf2, bf3, massDen)
```

<table>
<thead>
<tr>
<th>Parameter</th>
<th>Description</th>
</tr>
</thead>
<tbody>
<tr>
<td>eleTag (int)</td>
<td>unique element object tag</td>
</tr>
<tr>
<td>eleNodes (list (int))</td>
<td>a list of twenty element nodes, input order is shown in notes below</td>
</tr>
<tr>
<td>matTag (int)</td>
<td>material tag associated with previously-defined NDMaterial object</td>
</tr>
<tr>
<td>bf1 bf2 bf3 (float)</td>
<td>body force in the direction of global coordinates x, y and z</td>
</tr>
<tr>
<td>massDen (float)</td>
<td>mass density (mass/volume)</td>
</tr>
</tbody>
</table>

**Note:** The valid queries to a Brick20N element when creating an ElementRecorder object are ‘force,’ ‘stiffness,’ ‘stress’, ‘gausspoint’ or ‘plastic’. The output is given as follows:

1. ‘stress’
   - the six stress components from each Gauss points are output by the order: sigma_xx, sigma_yy, sigma_zz, sigma_xy, sigma_xz, sigma_yz
2. ‘gausspoint’
   - the coordinates of all Gauss points are printed out
3. ‘plastic’
   - the equivalent deviatoric plastic strain from each Gauss point is output in the same order as the coordinates are printed

See also:

Notes

SSPbrick Element

This command is used to construct a SSPbrick element object.

```
 element ('SSPbrick', eleTag, *eleNodes, matTag[, b1, b2, b3])
```

<table>
<thead>
<tr>
<th>Parameter</th>
<th>Description</th>
</tr>
</thead>
<tbody>
<tr>
<td>eleTag (int)</td>
<td>unique element object tag</td>
</tr>
<tr>
<td>eleNodes (list (int))</td>
<td>a list of eight element nodes in bottom and top faces and in counter-clockwise order</td>
</tr>
<tr>
<td>matTag (int)</td>
<td>unique integer tag associated with previously-defined nDMaterial object</td>
</tr>
<tr>
<td>b1 b2 b3 (float)</td>
<td>constant body forces in global x-, y-, and z-directions, respectively (optional, default = 0.0)</td>
</tr>
</tbody>
</table>

The SSPbrick element is an eight-node hexahedral element using physically stabilized single-point integration (SSP -> Stabilized Single Point). The stabilization incorporates an enhanced assumed strain field, resulting in an element which is free from volumetric and shear locking. The elimination of shear locking results in greater coarse mesh accuracy in bending dominated problems, and the elimination of volumetric locking improves accuracy in nearly-incompressible problems. Analysis times are generally faster than corresponding full integration elements.
Note:

1. Valid queries to the SSPbrick element when creating an ElementalRecorder object correspond to those for the nDMaterial object assigned to the element (e.g., ‘stress’, ‘strain’). Material response is recorded at the single integration point located in the center of the element.

2. The SSPbrick element was designed with intentions of duplicating the functionality of the stdBrick Element. If an example is found where the SSPbrick element cannot do something that works for the stdBrick Element, e.g., material updating, please contact the developers listed below so the bug can be fixed.

See also:

Notes

Tetrahedron Elements

1. **FourNodeTetrahedron**

**FourNodeTetrahedron**

This command is used to construct a standard four-node tetrahedron element object with one-point Gauss integration.

```python
element('FourNodeTetrahedron', eleTag, *eleNodes, matTag, [b1, b2, b3])
```

- **eleTag**: int, unique element object tag
- **eleNodes**: list (int), a list of four element nodes
- **matTag**: int, tag of nDMaterial
- **b1 b2 b3** (float), body forces in global x,y,z directions

See also:

Notes

**UC San Diego u-p element (saturated soil)**

1. **Four Node Quad u-p Element**
2. **Brick u-p Element**
3. **BbarQuad u-p Element**
4. **BbarBrick u-p Element**
5. **Nine Four Node Quad u-p Element**
6. **Twenty Eight Node Brick u-p Element**

**Four Node Quad u-p Element**

FourNodeQuadUP is a four-node plane-strain element using bilinear isoparametric formulation. This element is implemented for simulating dynamic response of solid-fluid fully coupled material, based on Biot’s theory of porous medium. Each element node has 3 degrees-of-freedom (DOF): DOF 1 and 2 for solid displacement (u) and DOF 3 for fluid pressure (p).
\textbf{element} ("", eleTag, *eleNodes, thick, matTag, bulk, fmass, hPerm, vPerm[, b1=0, b2=0, t=0])

<table>
<thead>
<tr>
<th>eleTag</th>
<th>unique element object tag</th>
</tr>
</thead>
<tbody>
<tr>
<td>eleNodes</td>
<td>a list of four element nodes in counter-clockwise order</td>
</tr>
<tr>
<td>thick</td>
<td>Element thickness</td>
</tr>
<tr>
<td>matTag</td>
<td>Tag of an NDMaterial object (previously defined) of which the element is composed</td>
</tr>
<tr>
<td>bulk</td>
<td>Combined undrained bulk modulus $Bc$ relating changes in pore pressure and volumetric strain, may be approximated by: $Bc = Bf/n$ where $Bf$ is the bulk modulus of fluid phase ($2.2 \times 10^6$ kPa (or $3.191 \times 10^5$ psi) for water), and $n$ the initial porosity.</td>
</tr>
<tr>
<td>fmass</td>
<td>Fluid mass density</td>
</tr>
<tr>
<td>hPerm, vPerm</td>
<td>Permeability coefficient in horizontal and vertical directions respectively.</td>
</tr>
<tr>
<td>b1, b2</td>
<td>Optional gravity acceleration components in horizontal and vertical directions respectively (defaults are 0.0)</td>
</tr>
<tr>
<td>t</td>
<td>Optional uniform element normal traction, positive in tension (default is 0.0)</td>
</tr>
</tbody>
</table>

\textbf{See also:}

\textbf{Notes}

\textbf{Brick u-p Element}

BrickUP is an 8-node hexahedral linear isoparametric element. Each node has 4 degrees-of-freedom (DOF): DOFs 1 to 3 for solid displacement (u) and DOF 4 for fluid pressure (p). This element is implemented for simulating dynamic response of solid-fluid fully coupled material, based on Biot’s theory of porous medium.

\textbf{element} ('brickUP', eleTag, *eleNodes, matTag, bulk, fmass, PermX, PermY, PermZ[, bX=0, bY=0, bZ=0])
<table>
<thead>
<tr>
<th></th>
<th>Description</th>
</tr>
</thead>
<tbody>
<tr>
<td><code>eleTag</code></td>
<td>unique element object tag</td>
</tr>
<tr>
<td><code>int</code></td>
<td></td>
</tr>
<tr>
<td><code>eleNodes</code></td>
<td>a list of eight element nodes</td>
</tr>
<tr>
<td><code>list (int)</code></td>
<td></td>
</tr>
<tr>
<td><code>matTag</code></td>
<td>Tag of an NDMaterial object (previously defined) of which the element is composed</td>
</tr>
<tr>
<td><code>int</code></td>
<td></td>
</tr>
<tr>
<td><code>bulk</code></td>
<td>Combined undrained bulk modulus $B_c$ relating changes in pore pressure and volumetric strain, may be approximated by: $B_c \approx B_f / n$ where $B_f$ is the bulk modulus of fluid phase ($2.2 \times 10^6$ kPa (or $3.191 \times 10^5$ psi) for water), and $n$ the initial porosity.</td>
</tr>
<tr>
<td><code>float</code></td>
<td></td>
</tr>
<tr>
<td><code>fmass</code></td>
<td>Fluid mass density</td>
</tr>
<tr>
<td><code>float</code></td>
<td></td>
</tr>
<tr>
<td><code>permX, permY, permZ</code></td>
<td>Permeability coefficients in x, y, and z directions respectively.</td>
</tr>
<tr>
<td><code>float</code></td>
<td></td>
</tr>
<tr>
<td><code>bX, bY, bZ</code></td>
<td>Optional gravity acceleration components in x, y, and z directions respectively (defaults are 0.0)</td>
</tr>
<tr>
<td><code>float</code></td>
<td></td>
</tr>
</tbody>
</table>

**See also:**

Notes

**BbarQuad u-p Element**

bbarQuadUP is a four-node plane-strain mixed volume/pressure element, which uses a tri-linear isoparametric formulation. This element is implemented for simulating dynamic response of solid-fluid fully coupled material, based on Biot’s theory of porous medium. Each element node has 3 degrees-of-freedom (DOF): DOF 1 and 2 for solid displacement (u) and DOF 3 for fluid pressure (p).

```python
element ("bbarQuadUP", eleTag, *eleNodes, thick, matTag, bulk, fmass, hPerm, vPerm[, b1=0, b2=0, t=0])
```
<table>
<thead>
<tr>
<th>eleTag</th>
<th>unique element object tag</th>
</tr>
</thead>
<tbody>
<tr>
<td>eleNodes</td>
<td>a list of four element nodes in counter-clockwise order</td>
</tr>
<tr>
<td>thick</td>
<td>Element thickness</td>
</tr>
<tr>
<td>matTag</td>
<td>Tag of an NDMaterial object (previously defined) of which the element is composed</td>
</tr>
<tr>
<td>bulk</td>
<td>Combined undrained bulk modulus $B_c$ relating changes in pore pressure and volumetric strain, may be approximated by: $B_c = B_f/n$ where $B_f$ is the bulk modulus of fluid phase ($2.2 \times 10^6$ kPa (or $3.191 \times 10^5$ psi) for water), and $n$ the initial porosity.</td>
</tr>
<tr>
<td>fmass</td>
<td>Fluid mass density</td>
</tr>
<tr>
<td>hPerm, vPerm</td>
<td>Permeability coefficient in horizontal and vertical directions respectively.</td>
</tr>
<tr>
<td>bl, b2</td>
<td>Optional gravity acceleration components in horizontal and vertical directions respectively (defaults are 0.0)</td>
</tr>
<tr>
<td>t</td>
<td>Optional uniform element normal traction, positive in tension (default is 0.0)</td>
</tr>
</tbody>
</table>

**See also:**

**Notes**

**BbarBrick u-p Element**

bbarBrickUP is a 8-node mixed volume/pressure element, which uses a tri-linear isoparametric formulation.

Each node has 4 degrees-of-freedom (DOF): DOFs 1 to 3 for solid displacement ($u$) and DOF 4 for fluid pressure ($p$).

This element is implemented for simulating dynamic response of solid-fluid fully coupled material, based on Biot's theory of porous medium.

```
element('bbarBrickUP', eleTag, *eleNodes, matTag, bulk, fmass, PermX, PermY, PermZ[, bX=0, bY=0, bZ=0])
```
eleTag (int)  | unique element object tag
---|---
eleNodes (list (int)) | a list of eight element nodes
matTag (int) | Tag of an NDMaterial object (previously defined) of which the element is composed
bulk (float) | Combined undrained bulk modulus \(B_c\) relating changes in pore pressure and volumetric strain, may be approximated by: \(B_c \approx B_f/n\) where \(B_f\) is the bulk modulus of fluid phase (2.2 \(\times\) 10\(^6\) kPa (or 3.191 \(\times\) 10\(^5\) psi) for water), and \(n\) the initial porosity.
fmass (float) | Fluid mass density
permX, permY, permZ (float) | Permeability coefficients in x, y, and z directions respectively.
bX, bY, bZ (float) | Optional gravity acceleration components in x, y, and z directions respectively (defaults are 0.0)

**See also:**

**Notes**

**Nine Four Node Quad u-p Element**

Nine_Four_Node_QuadUP is a 9-node quadrilateral plane-strain element. The four corner nodes have 3 degrees-of-freedom (DOF) each: DOF 1 and 2 for solid displacement (u) and DOF 3 for fluid pressure (p). The other five nodes have 2 DOFs each for solid displacement. This element is implemented for simulating dynamic response of solid-fluid fully coupled material, based on Biot’s theory of porous medium.

**element** ('9_4_QuadUP', eleTag, *eleNodes, thick, matTag, bulk, fmass, hPerm, vPerm[, b1=0, b2=0 ])

<table>
<thead>
<tr>
<th>eleTag (int)</th>
<th>unique element object tag</th>
</tr>
</thead>
<tbody>
<tr>
<td>eleNodes (list (int))</td>
<td>a list of nine element nodes</td>
</tr>
<tr>
<td>thick (float)</td>
<td>Element thickness</td>
</tr>
<tr>
<td>matTag (int)</td>
<td>Tag of an NDMaterial object (previously defined) of which the element is composed</td>
</tr>
<tr>
<td>bulk (float)</td>
<td>Combined undrained bulk modulus (B_c) relating changes in pore pressure and volumetric strain, may be approximated by: (B_c \approx B_f/n) where (B_f) is the bulk modulus of fluid phase (2.2 (\times) 10(^6) kPa (or 3.191 (\times) 10(^5) psi) for water), and (n) the initial porosity.</td>
</tr>
<tr>
<td>fmass (float)</td>
<td>Fluid mass density</td>
</tr>
<tr>
<td>hPerm, vPerm (float)</td>
<td>Permeability coefficient in horizontal and vertical directions respectively.</td>
</tr>
<tr>
<td>b1, b2 (float)</td>
<td>Optional gravity acceleration components in horizontal and vertical directions respectively (defaults are 0.0)</td>
</tr>
</tbody>
</table>
Twenty Eight Node Brick u-p Element

The eight corner nodes have 4 degrees-of-freedom (DOF) each: DOFs 1 to 3 for solid displacement (u) and DOF 4 for fluid pressure (p). The other nodes have 3 DOFs each for solid displacement. This element is implemented for simulating dynamic response of solid-fluid fully coupled material, based on Biot’s theory of porous medium.

\[
\text{element} (\text{'bbarBrickUP'}, \text{eleTag}, \text{eleNodes}, \text{matTag}, \text{bulk}, \text{fmass}, \text{PermX}, \text{PermY}, \text{PermZ}[, \text{bX}=0, \text{bY}=0, \text{bZ}=0])
\]

\begin{tabular}{|l|l|}
\hline
\text{eleTag} (int) & unique element object tag \\
\hline
\text{eleNodes} (list (int)) & a list of twenty element nodes \\
\hline
\text{matTag} (int) & Tag of an NDMaterial object (previously defined) of which the element is composed \\
\hline
\text{bulk} (float) & Combined undrained bulk modulus \( B_c \) relating changes in pore pressure and volumetric strain, may be approximated by: 
\( B_c \approx B_f/n \) 
where \( B_f \) is the bulk modulus of fluid phase (2.2 \times 10^6 \text{ kPa} \) (or 3.191 \times 10^5 \text{ psi}) for water), and \( n \) the initial porosity. \\
\hline
\text{fmass} (float) & Fluid mass density \\
\hline
\text{permX}, \text{permY}, \text{permZ} (float) & Permeability coefficients in x, y, and z directions respectively. \\
\hline
\text{bX, bY, bZ} (float) & Optional gravity acceleration components in x, y, and z directions directions respectively (defaults are 0,0) \\
\hline
\end{tabular}

Other u-p elements

1. \textit{SSPquadUP Element}
2. \textit{SSPbrickUP Element}

SSPquadUP Element

This command is used to construct a SSPquadUP element object.

\[
\text{element} (\text{'SSPquadUP'}, \text{eleTag}, \text{eleNodes}, \text{matTag}, \text{thick}, \text{fBulk}, \text{fDen}, \text{k1}, \text{k2}, \text{void}, \text{alpha}[, \text{b1}, \text{b2}])
\]
The SSPquadUP element is an extension of the SSPquad Element for use in dynamic plane strain analysis of fluid saturated porous media. A mixed displacement-pressure (u-p) formulation is used, based upon the work of Biot as extended by Zienkiewicz and Shiomi (1984).

The physical stabilization necessary to allow for reduced integration incorporates an assumed strain field in which the volumetric dilation and the shear strain associated with the the hourglass modes are zero, resulting in an element which is free from volumetric and shear locking. The elimination of shear locking results in greater coarse mesh accuracy in bending dominated problems, and the elimination of volumetric locking improves accuracy in nearly-incompressible problems. Analysis times are generally faster than corresponding full integration elements.

Equal-order interpolation is used for the displacement and pressure fields, thus, the SSPquadUP element does not inherently pass the inf-sup condition, and is not fully acceptable in the incompressible-impermeable limit (the QuadUP Element has the same issue). A stabilizing parameter is employed to permit the use of equal-order interpolation for the SSPquadUP element. This parameter $\alpha$ can be computed as

$$\alpha = 0.25 \cdot \left( \frac{h^2}{c^2 \cdot den} \right)$$

where $h$ is the element size, $c$ is the speed of elastic wave propagation in the solid phase, and $den$ is the mass density of the solid phase. The $\alpha$ parameter should be a small number. With a properly defined $\alpha$ parameter, the SSPquadUP element can produce comparable results to a higher-order element such as the 9_4_QuadUP Element at a significantly lower computational cost and with a greater ease in mesh generation.

The full formulation for the SSPquadUP element can be found in McGann et al. (2012) along with several example applications.

**Note:**

1. The SSPquadUP element will only work in dynamic analysis.
2. For saturated soils, the mass density input into the associated nDMaterial object should be the saturated mass density.
3. When modeling soil, the body forces input into the SSPquadUP element should be the components of the gravitational vector, not the unit weight.
4. Fixing the pore pressure degree-of-freedom (dof 3) at a node is a drainage boundary condition at which zero pore pressure will be maintained throughout the analysis. Leaving the third dof free allows pore pressures to build at that node.
5. Valid queries to the SSPquadUP element when creating an ElementalRecorder object correspond to those for the nDMaterial object assigned to the element (e.g., ‘stress’, ‘strain’). Material response is recorded at the single integration point located in the center of the element.
6. The SSPquadUP element was designed with intentions of duplicating the functionality of the QuadUP Element. If an example is found where the SSPquadUP element cannot do something that works for the QuadUP Element, e.g., material updating, please contact the developers listed below so the bug can be fixed.

See also:

Notes

SSPbrickUP Element

This command is used to construct a SSPbrickUP element object.

\[ \text{element ('SSPbrickUP', eleTag, *eleNodes, matTag, fBulk, fDen, k1, k2, k3, void, alpha[, b1, b2, b3])} \]

<table>
<thead>
<tr>
<th>Args</th>
<th>Description</th>
</tr>
</thead>
<tbody>
<tr>
<td>eleTag (int)</td>
<td>unique element object tag</td>
</tr>
<tr>
<td>eleNodes (list(int))</td>
<td>a list of eight element nodes in counter-clockwise order</td>
</tr>
<tr>
<td>matTag (float)</td>
<td>unique integer tag associated with previously-defined nDMaterial object</td>
</tr>
<tr>
<td>fBulk (float)</td>
<td>bulk modulus of the pore fluid</td>
</tr>
<tr>
<td>fDen (float)</td>
<td>mass density of the pore fluid</td>
</tr>
<tr>
<td>k1 k2 k3 (float)</td>
<td>permeability coefficients in global x-, y-, and z-directions, respectively</td>
</tr>
<tr>
<td>void (float)</td>
<td>voids ratio</td>
</tr>
<tr>
<td>alpha (float)</td>
<td>spatial pressure field stabilization parameter (see discussion below for more information)</td>
</tr>
<tr>
<td>b1 b2 b3 (float)</td>
<td>constant body forces in global x-, y-, and z-directions, respectively (optional, default = 0.0) - See Note 3</td>
</tr>
</tbody>
</table>

The SSPbrickUP element is an extension of the SSPbrick Element for use in dynamic 3D analysis of fluid saturated porous media. A mixed displacement-pressure (u-p) formulation is used, based upon the work of Biot as extended by Zienkiewicz and Shiomi (1984).

The physical stabilization necessary to allow for reduced integration incorporates an enhanced assumed strain field, resulting in an element which is free from volumetric and shear locking. The elimination of shear locking results in greater coarse mesh accuracy in bending dominated problems, and the elimination of volumetric locking improves accuracy in nearly-incompressible problems. Analysis times are generally faster than corresponding full integration elements.

Equal-order interpolation is used for the displacement and pressure fields, thus, the SSPbrickUP element does not inherently pass the inf-sup condition, and is not fully acceptable in the incompressible-impermeable limit (the brickUP Element has the same issue). A stabilizing parameter is employed to permit the use of equal-order interpolation for the SSPbrickUP element. This parameter $\alpha$ can be computed as

\[ \alpha = h^2/(4 * (K_s + (4/3) * G_s)) \]

where $h$ is the element size, and $K_s$ and $G_s$ are the bulk and shear moduli for the solid phase. The $\alpha$ parameter should be a small number. With a properly defined $\alpha$ parameter, the SSPbrickUP element can produce comparable results to a higher-order element such as the 20_8_BrickUP Element at a significantly lower computational cost and with a greater ease in mesh generation.

Note:

1. The SSPbrickUP element will only work in dynamic analysis.
2. For saturated soils, the mass density input into the associated nDMaterial object should be the saturated mass density.

3. When modeling soil, the body forces input into the SSPbrickUP element should be the components of the gravitational vector, not the unit weight.

4. Fixing the pore pressure degree-of-freedom (dof 4) at a node is a drainage boundary condition at which zero pore pressure will be maintained throughout the analysis. Leaving the fourth dof free allows pore pressures to build at that node.

5. Valid queries to the SSPbrickUP element when creating an ElementalRecorder object correspond to those for the nDMaterial object assigned to the element (e.g., ‘stress’, ‘strain’). Material response is recorded at the single integration point located in the center of the element.

6. The SSPbrickUP element was designed with intentions of duplicating the functionality of the brickUP Element. If an example is found where the SSPbrickUP element cannot do something that works for the brickUP Element, e.g., material updating, please contact the developers listed below so the bug can be fixed.

---

**See also:**

**Notes**

**Contact Elements**

1. **SimpleContact2D**
2. **SimpleContact3D**
3. **BeamContact2D**
4. **BeamContact3D**
5. **BeamEndContact3D**

**SimpleContact2D**

This command is used to construct a SimpleContact2D element object.

`element('SimpleContact2D', eleTag, iNode, jNode, sNode, lNode, matTag, gTol, fTol)`

<table>
<thead>
<tr>
<th>Parameter</th>
<th>Description</th>
</tr>
</thead>
<tbody>
<tr>
<td>eleTag (int)</td>
<td>unique element object tag</td>
</tr>
<tr>
<td>iNode, jNode (int)</td>
<td>master nodes (-ndm 2 -ndf 2)</td>
</tr>
<tr>
<td>sNode (int)</td>
<td>slave node (-ndm 2 -ndf 2)</td>
</tr>
<tr>
<td>lNode (int)</td>
<td>Lagrange multiplier node (-ndm 2 -ndf 2)</td>
</tr>
<tr>
<td>matTag (int)</td>
<td>unique integer tag associated with previously-defined nDMaterial object</td>
</tr>
<tr>
<td>gTol (float)</td>
<td>gap tolerance</td>
</tr>
<tr>
<td>fTol (float)</td>
<td>force tolerance</td>
</tr>
</tbody>
</table>

The SimpleContact2D element is a two-dimensional node-to-segment contact element which defines a frictional contact interface between two separate bodies. The master nodes are the nodes which define the endpoints of a line segment on the first body, and the slave node is a node from the second body. The Lagrange multiplier node is required to enforce the contact condition. This node should not be shared with any other element in the domain. Information on the theory behind this element can be found in, e.g., Wriggers (2002).

---

**Note:**
1. The SimpleContact2D element has been written to work exclusively with the ContactMaterial2D nDMaterial object.

2. The valid recorder queries for this element are:
   1. force - returns the contact force acting on the slave node in vector form.
   2. frictionforce - returns the frictional force acting on the slave node in vector form.
   3. forceScalar - returns the scalar magnitudes of the normal and tangential contact forces.

4. The SimpleContact2D elements are set to consider frictional behavior as a default, but the frictional state of the SimpleContact2D element can be changed from the input file using the setParameter command.

   When updating, value of 0 corresponds to the frictionless condition, and a value of 1 signifies the inclusion of friction. An example command for this update procedure is provided below.

3. The SimpleContact2D element works well in static and pseudo-static analysis situations.

4. In transient analysis, the presence of the contact constraints can affect the stability of commonly-used time integration methods in the HHT or Newmark family (e.g., Laursen, 2002). For this reason, use of alternative time-integration methods which numerically damp spurious high frequency behavior may be required. The TRBDF2 integrator is an effective method for this purpose. The Newmark integrator can also be effective with proper selection of the gamma and beta coefficients. The trapezoidal rule, i.e., Newmark with gamma = 0.5 and beta = 0.25, is particularly prone to instability related to the contact constraints and is not recommended.

---

**SimpleContact3D**

This command is used to construct a SimpleContact3D element object.

```python
element('SimpleContact3D', eleTag, iNode, jNode, kNode, lNode, sNode, LNode, matTag, gTol, fTol)
```

<table>
<thead>
<tr>
<th>Parameter</th>
<th>Description</th>
</tr>
</thead>
<tbody>
<tr>
<td>eleTag</td>
<td>unique element object tag</td>
</tr>
<tr>
<td>iNode, jNode, kNode, lNode</td>
<td>master nodes (-ndm 3 -ndf 3)</td>
</tr>
<tr>
<td>sNode</td>
<td>slave node (-ndm 3 -ndf 3)</td>
</tr>
<tr>
<td>LNode</td>
<td>Lagrange multiplier node (-ndm 3 -ndf 3)</td>
</tr>
<tr>
<td>matTag</td>
<td>unique integer tag associated with previously-defined nDMaterial object</td>
</tr>
<tr>
<td>gTol</td>
<td>gap tolerance</td>
</tr>
<tr>
<td>fTol</td>
<td>force tolerance</td>
</tr>
</tbody>
</table>

The SimpleContact3D element is a three-dimensional node-to-surface contact element which defines a frictional contact interface between two separate bodies. The master nodes are the nodes which define a surface of a hexahedral element on the first body, and the slave node is a node from the second body. The Lagrange multiplier node is required to enforce the contact condition. This node should not be shared with any other element in the domain. Information on the theory behind this element can be found in, e.g. Wriggers (2002).

**Note:**

1. The SimpleContact3D element has been written to work exclusively with the ContactMaterial3D nDMaterial object.
2. The valid recorder queries for this element are:
   1. force - returns the contact force acting on the slave node in vector form.
   2. frictionforce - returns the frictional force acting on the slave node in vector form.
   3. forcescalar - returns the scalar magnitudes of the single normal and two tangential contact forces.
   4. The SimpleContact3D elements are set to consider frictional behavior as a default, but the frictional state of the SimpleContact3D element can be changed from the input file using the setParameter command. When updating, value of 0 corresponds to the frictionless condition, and a value of 1 signifies the inclusion of friction. An example command for this update procedure is provided below.

3. The SimpleContact3D element works well in static and pseudo-static analysis situations.

4. In transient analysis, the presence of the contact constraints can effect the stability of commonly-used time integration methods in the HHT or Newmark family (e.g., Laursen, 2002). For this reason, use of alternative time-integration methods which numerically damp spurious high frequency behavior may be required. The TRBDF2 integrator is an effective method for this purpose. The Newmark integrator can also be effective with proper selection of the gamma and beta coefficients. The trapezoidal rule, i.e., Newmark with gamma = 0.5 and beta = 0.25, is particularly prone to instability related to the contact constraints and is not recommended.

See also:

Notes

BeamContact2D

This command is used to construct a BeamContact2D element object.

```
  element ("BeamContact2D", eleTag, matTag, width, gTol, fTol[, cFlag])
```

<table>
<thead>
<tr>
<th>eleTag</th>
<th>unique element object tag</th>
</tr>
</thead>
<tbody>
<tr>
<td>iNode</td>
<td>master nodes (-ndm 2 -ndf 3)</td>
</tr>
<tr>
<td>jNode</td>
<td>master nodes (-ndm 2 -ndf 3)</td>
</tr>
<tr>
<td>sNode</td>
<td>slave node (-ndm 2 -ndf 2)</td>
</tr>
<tr>
<td>lNode</td>
<td>Lagrange multiplier node (-ndm 2 -ndf 2)</td>
</tr>
<tr>
<td>matTag</td>
<td>unique integer tag associated with previously-defined nDMaterial object</td>
</tr>
<tr>
<td>width</td>
<td>the width of the wall represented by the beam element in plane strain</td>
</tr>
<tr>
<td>gTol</td>
<td>gap tolerance</td>
</tr>
<tr>
<td>fTol</td>
<td>force tolerance</td>
</tr>
<tr>
<td>cFlag</td>
<td>optional initial contact flag</td>
</tr>
<tr>
<td></td>
<td>cFlag = 0 &gt;&gt; contact between bodies is initially assumed (DEFAULT)</td>
</tr>
<tr>
<td></td>
<td>cFlag = 1 &gt;&gt; no contact between bodies is initially assumed</td>
</tr>
</tbody>
</table>

The BeamContact2D element is a two-dimensional beam-to-node contact element which defines a frictional contact interface between a beam element and a separate body. The master nodes (3 DOF) are the endpoints of the beam
element, and the slave node (2 DOF) is a node from a second body. The Lagrange multiplier node (2 DOF) is required
to enforce the contact condition. Each contact element should have a unique Lagrange multiplier node. The Lagrange
multiplier node should not be fixed, otherwise the contact condition will not work.

Under plane strain conditions in 2D, a beam element represents a unit thickness of a wall. The width is the dimension
of this wall in the 2D plane. This width should be built-in to the model to ensure proper enforcement of the con-
tact condition. The Excavation Supported by Cantilevered Sheet Pile Wall practical example provides some further
examples and discussion on the usage of this element.

---

**Note:**

1. The BeamContact2D element has been written to work exclusively with the ContactMaterial2D nDMaterial
   object.
2. The valid recorder queries for this element are:
   1. force - returns the contact force acting on the slave node in vector form.
   2. frictionforce - returns the frictional force acting on the slave node in vector form.
   3. forcescalar - returns the scalar magnitudes of the normal and tangential contact forces.
   4. masterforce - returns the reactions (forces and moments) acting on the master nodes.
   5. The BeamContact2D elements are set to consider frictional behavior as a default, but the frictional state of
      the BeamContact2D element can be changed from the input file using the setParameter command. When
      updating, value of 0 corresponds to the frictionless condition, and a value of 1 signifies the inclusion of
      friction. An example command for this update procedure is provided below
3. The BeamContact2D element works well in static and pseudo-static analysis situations.
4. In transient analysis, the presence of the contact constraints can effect the stability of commonly-used time
   integration methods in the HHT or Newmark family (e.g., Laursen, 2002). For this reason, use of alternative
   time-integration methods which numerically damp spurious high frequency behavior may be required. The
   TRBDF2 integrator is an effective method for this purpose. The Newmark integrator can also be effective with
   proper selection of the gamma and beta coefficients. The trapezoidal rule, i.e., Newmark with gamma = 0.5 and
   beta = 0.25, is particularly prone to instability related to the contact constraints and is not recommended.

---

**See also:**

Notes

**BeamContact3D**

This command is used to construct a BeamContact3D element object.

\[
\text{element ('BeamContact3D', \text{eleTag}, \text{iNode}, \text{jNode}, \text{sNode}, \text{lNode}, \text{radius}, \text{crdTransf}, \text{matTag}, \text{gTol}, \text{fTol}[, \text{cFlag}\])}
\]
The BeamContact3D element is a three-dimensional beam-to-node contact element which defines a frictional contact interface between a beam element and a separate body. The master nodes (6 DOF) are the endpoints of the beam element, and the slave node (3 DOF) is a node from a second body. The Lagrange multiplier node (3 DOF) is required to enforce the contact condition. Each contact element should have a unique Lagrange multiplier node. The Lagrange multiplier node should not be fixed, otherwise the contact condition will not work.

Note:

1. The BeamContact3D element has been written to work exclusively with the ContactMaterial3D nDMaterial object.
2. The valid recorder queries for this element are:
   1. force - returns the contact force acting on the slave node in vector form.
   2. frictionforce - returns the frictional force acting on the slave node in vector form.
   3. forcescalar - returns the scalar magnitudes of the single normal and two tangential contact forces.
   4. masterforce - returns the reactions (forces only) acting on the master nodes.
   5. mastermoment - returns the reactions (moments only) acting on the master nodes.
   6. masterreaction - returns the full reactions (forces and moments) acting on the master nodes.
   7. The BeamContact3D elements are set to consider frictional behavior as a default, but the frictional state of the BeamContact3D element can be changed from the input file using the setParameter command. When updating, value of 0 corresponds to the frictionless condition, and a value of 1 signifies the inclusion of friction. An example command for this update procedure is provided below
3. The BeamContact3D element works well in static and pseudo-static analysis situations.
4. In transient analysis, the presence of the contact constraints can effect the stability of commonly-used time integration methods in the HHT or Newmark family (e.g., Laursen, 2002). For this reason, use of alternative time-integration methods which numerically damp spurious high frequency behavior may be required. The
TRBDF2 integrator is an effective method for this purpose. The Newmark integrator can also be effective with proper selection of the gamma and beta coefficients. The trapezoidal rule, i.e., Newmark with gamma = 0.5 and beta = 0.25, is particularly prone to instability related to the contact constraints and is not recommended.

See also:

Notes

**BeamEndContact3D**

This command is used to construct a BeamEndContact3D element object.

```python
element ('BeamEndContact3D', eleTag, iNode, jNode, sNode, lNode, radius, gTol, fTol[, cFlag])
```

<table>
<thead>
<tr>
<th>Parameter</th>
<th>Description</th>
</tr>
</thead>
<tbody>
<tr>
<td><code>eleTag</code></td>
<td>unique element object tag (int)</td>
</tr>
<tr>
<td><code>iNode</code></td>
<td>master node from the beam (-ndm 3 -ndf 6) (int)</td>
</tr>
<tr>
<td><code>jNode</code></td>
<td>the remaining node on the beam element with <code>iNode</code> (-ndm 3 -ndf 6) (int)</td>
</tr>
<tr>
<td><code>sNode</code></td>
<td>slave node (-ndm 3 -ndf 3) (int)</td>
</tr>
<tr>
<td><code>lNode</code></td>
<td>Lagrange multiplier node (-ndm 3 -ndf 3) (int)</td>
</tr>
<tr>
<td><code>radius</code></td>
<td>radius of circular beam associated with beam element (float)</td>
</tr>
<tr>
<td><code>gTol</code></td>
<td>gap tolerance (float)</td>
</tr>
<tr>
<td><code>fTol</code></td>
<td>force tolerance (float)</td>
</tr>
<tr>
<td><code>cFlag</code></td>
<td>optional initial contact flag (float)</td>
</tr>
</tbody>
</table>

The `BeamEndContact3D` element is a node-to-surface contact element which defines a normal contact interface between the end of a beam element and a separate body. The first master node ($iNode$) is the beam node which is at the end of the beam (i.e. only connected to a single beam element), the second node ($jNode$) is the remaining node on the beam element in question. The slave node is a node from a second body. The Lagrange multiplier node is required to enforce the contact condition. This node should not be shared with any other element in the domain, and should be created with the same number of DOF as the slave node.

The `BeamEndContact3D` element enforces a contact condition between a fictitious circular plane associated with a beam element and a node from a second body. The normal direction of the contact plane coincides with the endpoint tangent of the beam element at the master beam node ($iNode$). The extents of this circular plane are defined by the radius input parameter. The master beam node can only come into contact with a slave node which is within the extents of the contact plane. There is a lag step associated with changing between the ‘in contact’ and ‘not in contact’ conditions.

This element was developed for use in establishing a contact condition for the tip of a pile modeled as using beam elements and the underlying soil elements in three-dimensional analysis.

**Note:**
1. The BeamEndContact3D element does not use a material object.

2. The valid recorder queries for this element are:
   1. force - returns the contact force acting on the slave node in vector form.
   2. masterforce - returns the reactions (forces and moments) acting on the master node.
   3. The BeamEndContact3D element works well in static and pseudo-static analysis situations.

3. In transient analysis, the presence of the contact constraints can effect the stability of commonly-used time integration methods in the HHT or Newmark family (e.g., Laursen, 2002). For this reason, use of alternative time-integration methods which numerically damp spurious high frequency behavior may be required. The TRBDF2 integrator is an effective method for this purpose. The Newmark integrator can also be effective with proper selection of the gamma and beta coefficients. The trapezoidal rule, i.e., Newmark with gamma = 0.5 and beta = 0.25, is particularly prone to instability related to the contact constraints and is not recommended.

See also:

Notes

Cable Elements

1. CatenaryCableElement

CatenaryCableElement

This command is used to construct a catenary cable element object.

```python
element('CatenaryCable', eleTag, iNode, jNode, weight, E, A, L0, alpha, temperature_change, rho, errorTol, Nsubsteps, massType)
```

<table>
<thead>
<tr>
<th>Parameter</th>
<th>Description</th>
</tr>
</thead>
<tbody>
<tr>
<td>eleTag (int)</td>
<td>unique element object tag</td>
</tr>
<tr>
<td>iNode, jNode (int)</td>
<td>end nodes (3 dof per node)</td>
</tr>
<tr>
<td>E (float)</td>
<td>elastic modulus of the cable material</td>
</tr>
<tr>
<td>A (float)</td>
<td>cross-sectional area of element</td>
</tr>
<tr>
<td>L0 (float)</td>
<td>unstretched length of the cable</td>
</tr>
<tr>
<td>alpha (float)</td>
<td>coefficient of thermal expansion</td>
</tr>
<tr>
<td>temperature_change</td>
<td>temperature change for the element</td>
</tr>
<tr>
<td>rho (float)</td>
<td>mass per unit length</td>
</tr>
<tr>
<td>errorTol (float)</td>
<td>allowed tolerance for within-element equilibrium (Newton-Rhapson iterations)</td>
</tr>
<tr>
<td>Nsubsteps (int)</td>
<td>number of within-element substeps into which equilibrium iterations are subdivided (not number of steps to convergence)</td>
</tr>
<tr>
<td>massType (int)</td>
<td>Mass matrix model to use (massType = 0 lumped mass matrix, massType = 1 rigid-body mass matrix (in development))</td>
</tr>
</tbody>
</table>

This cable is a flexibility-based formulation of the catenary cable. An iterative scheme is used internally to compute equilibrium. At each iteration, node i is considered fixed while node j is free. End-forces are applied at node-j and its displacements computed. Corrections to these forces are applied iteratively using a Newton-Rhapson scheme (with optional sub-stepping via $Nsubsteps) until nodal displacements are within the provided tolerance ($errorTol). When convergence is reached, a stiffness matrix is computed by inversion of the flexibility matrix and rigid-body mode injection.
Note:

1. The stiffness of the cable comes from the large-deformation interaction between loading and cable shape. Therefore, all cables must have distributed forces applied to them. See example. Should not work for only nodal forces.

2. Valid queries to the CatenaryCable element when creating an ElementalRecorder object correspond to ‘forces’, which output the end-forces of the element in global coordinates (3 for each node).

3. Only the lumped-mass formulation is currently available.

4. The element does up 100 internal iterations. If convergence is not achieved, will result in error and some diagnostic information is printed out.

See also:

Notes

PFEM Elements

1. PFEMElementBubble

2. PFEMElementCompressible

PFEMElementBubble

`element('PFEMElementBubble', eleTag, nd1, nd2, nd3[, nd4] rho, mu, b1, b2[, b3], [thickness, kappa])`

Create a PFEM Bubble element, which is a fluid element for FSI analysis.

<table>
<thead>
<tr>
<th>eleTag (int)</th>
<th>tag of the element</th>
</tr>
</thead>
<tbody>
<tr>
<td>nd1 (int)</td>
<td>tag of node 1</td>
</tr>
<tr>
<td>nd2 (int)</td>
<td>tag of node 2</td>
</tr>
<tr>
<td>nd3 (int)</td>
<td>tag of node 3</td>
</tr>
<tr>
<td>nd4 (int)</td>
<td>tag of node 4 (required for 3D)</td>
</tr>
<tr>
<td>rho (float)</td>
<td>fluid density</td>
</tr>
<tr>
<td>mu (float)</td>
<td>fluid viscosity</td>
</tr>
<tr>
<td>b1 (float)</td>
<td>body body acceleration in x direction</td>
</tr>
<tr>
<td>b2 (float)</td>
<td>body body acceleration in y direction</td>
</tr>
<tr>
<td>b3 (float)</td>
<td>body body acceleration in z direction (required for 3D)</td>
</tr>
<tr>
<td>thickness (float)</td>
<td>element thickness (required for 2D)</td>
</tr>
<tr>
<td>kappa (float)</td>
<td>fluid bulk modulus (optional)</td>
</tr>
</tbody>
</table>

PFEMElementCompressible

`element('PFEMElementCompressible', eleTag, nd1, nd2, nd3, nd4, rho, mu, b1, b2[, thickness, kappa])`

Create a PFEM compressible element, which is a fluid element for FSI analysis.
<table>
<thead>
<tr>
<th><strong>eleTag</strong> (int)</th>
<th>tag of the element</th>
</tr>
</thead>
<tbody>
<tr>
<td><strong>nd1</strong> (int)</td>
<td>tag of node 1</td>
</tr>
<tr>
<td><strong>nd2</strong> (int)</td>
<td>tag of node 2</td>
</tr>
<tr>
<td><strong>nd3</strong> (int)</td>
<td>tag of node 3</td>
</tr>
<tr>
<td><strong>nd4</strong> (int)</td>
<td>tag of node 4 (middle node)</td>
</tr>
<tr>
<td><strong>rho</strong> (float)</td>
<td>fluid density</td>
</tr>
<tr>
<td><strong>mu</strong> (float)</td>
<td>fluid viscosity</td>
</tr>
<tr>
<td><strong>b1</strong> (float)</td>
<td>body body acceleration in x direction</td>
</tr>
<tr>
<td><strong>b2</strong> (float)</td>
<td>body body acceleration in y direction</td>
</tr>
<tr>
<td><strong>thickness</strong> (float)</td>
<td>element thickness (optional)</td>
</tr>
<tr>
<td><strong>kappa</strong> (float)</td>
<td>fluid bulk modulus (optional)</td>
</tr>
</tbody>
</table>

**Misc.**

1. *SurfaceLoad Element*
2. *VS3D4*
3. *AC3D8*
4. *ASI3D8*
5. *AV3D4*

**SurfaceLoad Element**

This command is used to construct a SurfaceLoad element object.

\[
\text{element} \left('\text{SurfaceLoad}', \text{eleTag}, \ast \text{eleNodes}, p\right)
\]

<table>
<thead>
<tr>
<th><strong>eleTag</strong> (int)</th>
<th>unique element object tag</th>
</tr>
</thead>
<tbody>
<tr>
<td><strong>eleNodes</strong> (list (int))</td>
<td>the four nodes defining the element, input in counterclockwise order (-ndm 3 -ndf 3)</td>
</tr>
<tr>
<td><strong>p</strong> (float)</td>
<td>applied pressure loading normal to the surface, outward is positive, inward is negative</td>
</tr>
</tbody>
</table>

The SurfaceLoad element is a four-node element which can be used to apply surface pressure loading to 3D brick elements. The SurfaceLoad element applies energetically-conjugate forces corresponding to the input scalar pressure to the nodes associated with the element. As these nodes are shared with a 3D brick element, the appropriate nodal loads are therefore applied to the brick.

**Note:**

1. There are no valid ElementalRecorder queries for the SurfaceLoad element. Its sole purpose is to apply nodal forces to the adjacent brick element.
2. The pressure loading from the SurfaceLoad element can be applied in a load pattern. See the analysis example below.

**See also:**

**Notes**
VS3D4

This command is used to construct a four-node 3D viscous-spring boundary quad element object based on a bilinear isoparametric formulation.

\[ \text{element ('VS3D4', eleTag, *eleNodes, E, G, rho, R, alphaN, alphaT)} \]

<table>
<thead>
<tr>
<th>eleTag (int)</th>
<th>unique element object tag</th>
</tr>
</thead>
<tbody>
<tr>
<td>eleNodes (list (int))</td>
<td>4 end nodes</td>
</tr>
<tr>
<td>E (float)</td>
<td>Young’s Modulus of element material</td>
</tr>
<tr>
<td>G (float)</td>
<td>Shear Modulus of element material</td>
</tr>
<tr>
<td>rho (float)</td>
<td>Mass Density of element material</td>
</tr>
<tr>
<td>R (float)</td>
<td>distance from the scattered wave source to the boundary</td>
</tr>
<tr>
<td>alphaN (float)</td>
<td>correction parameter in the normal direction</td>
</tr>
<tr>
<td>alphaT (float)</td>
<td>correction parameter in the tangential direction</td>
</tr>
</tbody>
</table>


See also:

Notes

AC3D8

This command is used to construct an eight-node 3D brick acoustic element object based on a trilinear isoparametric formulation.

\[ \text{element ('AC3D8', eleTag, *eleNodes, matTag)} \]

<table>
<thead>
<tr>
<th>eleTag (int)</th>
<th>unique element object tag</th>
</tr>
</thead>
<tbody>
<tr>
<td>eleNodes (list (int))</td>
<td>8 end nodes</td>
</tr>
<tr>
<td>matTag (int)</td>
<td>Material Tag of previously defined nD material</td>
</tr>
</tbody>
</table>

**Note:** Reference: ABAQUS theory manual. (2.9.1 Coupled acoustic-structural medium analysis)

See also:

Notes

ASI3D8

This command is used to construct an eight-node zero-thickness 3D brick acoustic-structure interface element object based on a bilinear isoparametric formulation. The nodes in the acoustic domain share the same coordinates with the nodes in the solid domain.
element ('ASI3D8', eleTag, *eleNodes1, *eleNodes2)

<table>
<thead>
<tr>
<th>eleTag (int)</th>
<th>unique element object tag</th>
</tr>
</thead>
<tbody>
<tr>
<td>*eleNodes (list (int))</td>
<td>four nodes defining structure domain of element boundaries</td>
</tr>
<tr>
<td>*eleNodes2 (list (int))</td>
<td>four nodes defining acoustic domain of element boundaries</td>
</tr>
</tbody>
</table>

**Note:** Reference: ABAQUS theory manual. (2.9.1 Coupled acoustic-structural medium analysis)

See also:

Notes

**AV3D4**

This command is used to construct a four-node 3D acoustic viscous boundary quad element object based on a bilinear isoparametric formulation.

**AV3D4**

<table>
<thead>
<tr>
<th>eleTag (int)</th>
<th>unique element object tag</th>
</tr>
</thead>
<tbody>
<tr>
<td>eleNodes (list (int))</td>
<td>4 end nodes</td>
</tr>
<tr>
<td>matTag (int)</td>
<td>Material Tag of previously defined nD material</td>
</tr>
</tbody>
</table>

See also:

Notes

**1.4.3 node command**

**node** (nodeTag, *crds, '-ndf', ndf, '-mass', *mass, '-disp', *disp, '-vel', *vel, '-accel', *accel)

Create a OpenSees node.

<table>
<thead>
<tr>
<th>nodeTag (int)</th>
<th>node tag.</th>
</tr>
</thead>
<tbody>
<tr>
<td>crds (list (float))</td>
<td>nodal coordinates.</td>
</tr>
<tr>
<td>ndf (float)</td>
<td>nodal ndf. (optional)</td>
</tr>
<tr>
<td>mass (list (float))</td>
<td>nodal mass. (optional)</td>
</tr>
<tr>
<td>vel (list (float))</td>
<td>nodal velocities. (optional)</td>
</tr>
<tr>
<td>accel (list (float))</td>
<td>nodal accelerations. (optional)</td>
</tr>
</tbody>
</table>

**1.4.4 sp constraint commands**

Create constraints for a single dof of a node.

1. **fix command**
2. **fixX command**
3. **fixY command**
4. **fixZ command**
fix command

```bash
fix (nodeTag, *constrValues)
Create a homogeneous SP constraint.
```

<table>
<thead>
<tr>
<th>nodeTag (int)</th>
<th>tag of node to be constrained</th>
</tr>
</thead>
</table>
| constrValues (list (int)) | a list of constraint values (0 or 1), must be preceded with *.
|                   | • 0 free                      |
|                   | • 1 fixed                     |

For example,

```bash
# fully fixed
vals = [1,1,1]
fix(nodeTag, *vals)
```

fixX command

```bash
fixX (x, *constrValues, '-tol', tol=1e-10)
Create homogeneous SP constraints.
```

<table>
<thead>
<tr>
<th>x (float)</th>
<th>x-coordinate of nodes to be constrained</th>
</tr>
</thead>
</table>
| constrValues (list (int)) | a list of constraint values (0 or 1), must be preceded with *.
|                   | • 0 free                                |
|                   | • 1 fixed                               |
| tol (float)     | user-defined tolerance (optional)       |

fixY command

```bash
fixY (y, *constrValues, '-tol', tol=1e-10)
Create homogeneous SP constraints.
```

<table>
<thead>
<tr>
<th>y (float)</th>
<th>y-coordinate of nodes to be constrained</th>
</tr>
</thead>
</table>
| constrValues (list (int)) | a list of constraint values (0 or 1), must be preceded with *.
|                   | • 0 free                                |
|                   | • 1 fixed                               |
| tol (float)     | user-defined tolerance (optional)       |

fixZ command

```bash
fixZ (z, *constrValues, '-tol', tol=1e-10)
Create homogeneous SP constraints.
```
### 1.4.5 mp constraint commands

Create constraints for multiple dofs of multiple nodes.

1. `equalDOF command`
2. `equalDOF_Mixed command`
3. `rigidDiaphragm command`
4. `rigidLink command`

**equalDOF command**

**equalDOF** (rNodeTag, cNodeTag, *dofs)

Create a multi-point constraint between nodes.

<table>
<thead>
<tr>
<th>Parameter</th>
<th>Description</th>
</tr>
</thead>
<tbody>
<tr>
<td>rNodeTag</td>
<td>integer tag identifying the retained, or master node.</td>
</tr>
<tr>
<td>cNodeTag</td>
<td>integer tag identifying the constrained, or slave node.</td>
</tr>
<tr>
<td>dofs</td>
<td>nodal degrees-of-freedom that are constrained at the cNode to be the same as those at the rNode. Valid range is from 1 through ndf, the number of nodal degrees-of-freedom.</td>
</tr>
</tbody>
</table>

**equalDOF_Mixed command**

**equalDOF_Mixed** (rNodeTag, cNodeTag, numDOF, *rcdofs)

Create a multi-point constraint between nodes.

<table>
<thead>
<tr>
<th>Parameter</th>
<th>Description</th>
</tr>
</thead>
<tbody>
<tr>
<td>rNodeTag</td>
<td>integer tag identifying the retained, or master node.</td>
</tr>
<tr>
<td>cNodeTag</td>
<td>integer tag identifying the constrained, or slave node.</td>
</tr>
<tr>
<td>numDOF</td>
<td>number of dofs to be constrained</td>
</tr>
<tr>
<td>rcdofs</td>
<td>nodal degrees-of-freedom that are constrained at the cNode to be the same as those at the rNode. Valid range is from 1 through ndf, the number of nodal degrees-of-freedom. rcdofs = [rdof1, cdof1, rdof2, cdof2, ...]</td>
</tr>
</tbody>
</table>
### rigidDiaphragm command

**rigidDiaphragm** (*perpDirn, rNodeTag, *cNodeTags*)

Create a multi-point constraint between nodes. These objects will constraint certain degrees-of-freedom at the listed slave nodes to move as if in a rigid plane with the master node. To enforce this constraint, **Transformation constraint** is recommended.

<table>
<thead>
<tr>
<th><strong>perpDirn (int)</strong></th>
<th>direction perpendicular to the rigid plane (i.e. direction 3 corresponds to the 1-2 plane)</th>
</tr>
</thead>
<tbody>
<tr>
<td><strong>rNodeTag (int)</strong></td>
<td>integer tag identifying the master node</td>
</tr>
<tr>
<td><strong>cNodeTags (list)</strong></td>
<td>integer tags identifying the slave nodes</td>
</tr>
</tbody>
</table>

### rigidLink command

**rigidLink** (*type, rNodeTag, cNodeTag*)

Create a multi-point constraint between nodes.

<table>
<thead>
<tr>
<th><strong>type (str)</strong></th>
<th>string-based argument for rigid-link type:</th>
</tr>
</thead>
<tbody>
<tr>
<td></td>
<td>• 'bar': only the translational degree-of-freedom will be constrained to be exactly the same as those at the master node</td>
</tr>
<tr>
<td></td>
<td>• 'beam': both the translational and rotational degrees of freedom are constrained.</td>
</tr>
<tr>
<td><strong>rNodeTag (int)</strong></td>
<td>integer tag identifying the master node</td>
</tr>
<tr>
<td><strong>cNodeTag (int)</strong></td>
<td>integer tag identifying the slave node</td>
</tr>
</tbody>
</table>

### 1.4.6 timeSeries commands

**timeSeries** (*tsType, tsTag, *tsArgs*)

This command is used to construct a TimeSeries object which represents the relationship between the time in the domain, \( t \), and the load factor applied to the loads, \( \lambda \), in the load pattern with which the TimeSeries object is associated, i.e. \( \lambda = F(t) \).

<table>
<thead>
<tr>
<th><strong>tsType (str)</strong></th>
<th>time series type.</th>
</tr>
</thead>
<tbody>
<tr>
<td><strong>tsTag (int)</strong></td>
<td>time series tag.</td>
</tr>
<tr>
<td><strong>tsArgs (list)</strong></td>
<td>a list of time series arguments</td>
</tr>
</tbody>
</table>

The following contain information about available **tsType**:

1. **Constant TimeSeries**
2. **Linear TimeSeries**
3. **Trigonometric TimeSeries**
4. **Triangular TimeSeries**
5. **Rectangular TimeSeries**
6. **Pulse TimeSeries**
7. **Path TimeSeries**
Constant TimeSeries

timeSeries ('Constant', tag, 'factor', factor=1.0)

This command is used to construct a TimeSeries object in which the load factor applied remains constant and is independent of the time in the domain, i.e. \( \lambda = f(t) = C \).

<table>
<thead>
<tr>
<th>tag (int)</th>
<th>unique tag among TimeSeries objects.</th>
</tr>
</thead>
<tbody>
<tr>
<td>factor (float)</td>
<td>the load factor applied (optional)</td>
</tr>
</tbody>
</table>

Linear TimeSeries

timeSeries ('Linear', tag, 'factor', factor=1.0)

This command is used to construct a TimeSeries object in which the load factor applied is linearly proportional to the time in the domain, i.e. \( \lambda = f(t) = cFactor * t \).

<table>
<thead>
<tr>
<th>tag (int)</th>
<th>unique tag among TimeSeries objects.</th>
</tr>
</thead>
<tbody>
<tr>
<td>factor (float)</td>
<td>Linear factor. (optional)</td>
</tr>
</tbody>
</table>

Trigonometric TimeSeries

timeSeries ('Trig', tag, tStart, tEnd, period, 'factor', factor=1.0, 'shift', shift=0.0, 'zeroShift', zeroShift=0.0)

This command is used to construct a TimeSeries object in which the load factor is some trigonometric function of the time in the domain.

\[
\lambda = f(t) = \begin{cases} 
  cFactor \sin\left(\frac{2\pi(t-tStart)}{period}\right) + \phi, & tStart \leq t \leq tEnd \\
  0.0, & \text{otherwise} 
\end{cases}
\]

\[
\phi = shift - \frac{period}{2.0\pi} \arcsin\left(\frac{zeroShift}{cFactor}\right)
\]

<table>
<thead>
<tr>
<th>tag (int)</th>
<th>unique tag among TimeSeries objects.</th>
</tr>
</thead>
<tbody>
<tr>
<td>tStart (float)</td>
<td>Starting time of non-zero load factor.</td>
</tr>
<tr>
<td>tEnd (float)</td>
<td>Ending time of non-zero load factor.</td>
</tr>
<tr>
<td>period (float)</td>
<td>Characteristic period of sine wave.</td>
</tr>
<tr>
<td>shift (float)</td>
<td>Phase shift in radians. (optional)</td>
</tr>
<tr>
<td>factor (float)</td>
<td>Load factor. (optional)</td>
</tr>
<tr>
<td>zeroShift (float)</td>
<td>Zero shift. (optional)</td>
</tr>
</tbody>
</table>

Triangular TimeSeries

timeSeries ('Triangle', tag, tStart, tEnd, period, 'factor', factor=1.0, 'shift', shift=0.0, 'zeroShift', zeroShift=0.0)

This command is used to construct a TimeSeries object in which the load factor is some triangular function of the time in the domain.

\[
\lambda = f(t) = \begin{cases} 
  \text{slope} \times k \times \text{period} + \text{zeroShift}, & k < 0.25 \\
  cFactor - \text{slope} \times (k - 0.25) \times \text{period} + \text{zeroShift}, & k < 0.75 \\
  -cFactor + \text{slope} \times (k - 0.75) \times \text{period} + \text{zeroShift}, & k < 1.0 \\
  0.0, & \text{otherwise} 
\end{cases}
\]
\[ slope = \frac{cFactor}{\text{period}/4} \]

\[ k = \frac{t + \phi - tStart}{\text{period}} - \lfloor \frac{t + \phi - tStart}{\text{period}} \rfloor \]

\[ \phi = \text{shift} - \frac{\text{zeroShift}}{\text{slope}} \]

**Rectangular TimeSeries**

\[ \lambda(t) = \begin{cases} 
  cFactor, & tStart <= t <= tEnd \\
  0.0, & \text{otherwise} 
\end{cases} \]

**Pulse TimeSeries**

\[ \lambda(t) = \begin{cases} 
  cFactor + \text{zeroShift}, & k < \text{width} \\
  \text{zeroshift}, & k < 1 \\
  0.0, & \text{otherwise} 
\end{cases} \]

\[ k = \frac{t + \text{shift} - tStart}{\text{period}} - \lfloor \frac{t + \text{shift} - tStart}{\text{period}} \rfloor \]
Path TimeSeries

timeSeries ('Path', tag, '-dt', dt=0.0, '-values', *values, '-time', *time, '-filepath', filePath='', '-fileTime', fileTime='', '-factor', factor=1.0, '-startTime', startTime=0.0, '-useLast', '-prependZero')

The relationship between load factor and time is input by the user as a series of discrete points in the 2d space (load factor, time). The input points can come from a file or from a list in the script. When the time specified does not match any of the input points, linear interpolation is used between points. There are many ways to specify the load path, for example, the load factors set with values or filePath, and the time set with dt, time, or fileTime.

<table>
<thead>
<tr>
<th>tag (int)</th>
<th>unique tag among TimeSeries objects.</th>
</tr>
</thead>
<tbody>
<tr>
<td>dt (float)</td>
<td>Time interval between specified points. (optional)</td>
</tr>
<tr>
<td>values (list (float))</td>
<td>Load factor values in a (list). (optional)</td>
</tr>
<tr>
<td>time (list (float))</td>
<td>Time values in a (list). (optional)</td>
</tr>
<tr>
<td>filePath (str)</td>
<td>File containing the load factors values. (optional)</td>
</tr>
<tr>
<td>fileTime (str)</td>
<td>File containing the time values for corresponding load factors. (optional)</td>
</tr>
<tr>
<td>factor (float)</td>
<td>A factor to multiply load factors by. (optional)</td>
</tr>
<tr>
<td>startTime (float)</td>
<td>Provide a start time for provided load factors. (optional)</td>
</tr>
<tr>
<td>'-useLast' (str)</td>
<td>Use last value after the end of the series. (optional)</td>
</tr>
<tr>
<td>'-prependZero' (str)</td>
<td>Prepend a zero value to the series of load factors. (optional)</td>
</tr>
</tbody>
</table>

- Linear interpolation between points.
- If the specified time is beyond last point (AND WATCH FOR NUMERICAL ROUNDOFF), 0.0 is returned. Specify '-useLast' to use the last data point instead of 0.0.
- The transient integration methods in OpenSees assume zero initial conditions. So it is important that any timeSeries that is being used in a transient analysis' starts from zero (first data point in the timeSeries = 0.0). To guarantee that this is the case the optional parameter '-prependZero' can be specified to prepend a zero value to the provided TimeSeries.

1.4.7 pattern commands

pattern (patternType, patternTag, *patternArgs)

The pattern command is used to construct a LoadPattern and add it to the Domain. Each LoadPattern in OpenSees has a TimeSeries associated with it. In addition it may contain ElementLoads, NodalLoads and SinglePointConstraints. Some of these SinglePoint constraints may be associated with GroundMotions.

<table>
<thead>
<tr>
<th>patternType (str)</th>
<th>pattern type.</th>
</tr>
</thead>
<tbody>
<tr>
<td>patternTag (int)</td>
<td>pattern tag.</td>
</tr>
<tr>
<td>patternArgs (list)</td>
<td>a list of pattern arguments</td>
</tr>
</tbody>
</table>

The following contain information about available patternType:

1. Plain Pattern
2. UniformExcitation Pattern
3. Multi-Support Excitation Pattern
Plain Pattern

`pattern('Plain', patternTag, tsTag, '-fact', factor)`  
This command allows the user to construct a LoadPattern object. Each plain load pattern is associated with a TimeSeries object and can contain multiple NodalLoads, ElementalLoads and SP_Constraint objects. The command to generate LoadPattern object contains in { } the commands to generate all the loads and the single-point constraints in the pattern. To construct a load pattern and populate it, the following command is used:

<table>
<thead>
<tr>
<th>patternTag (int)</th>
<th>unique tag among load patterns.</th>
</tr>
</thead>
<tbody>
<tr>
<td>tsTag (int)</td>
<td>the tag of the time series to be used in the load pattern</td>
</tr>
<tr>
<td>factor (float)</td>
<td>constant factor. (optional)</td>
</tr>
</tbody>
</table>

**Note:** the commands below to generate all the loads and sp constraints will be included in last called pattern command.

load command

`load(nodeTag, *loadValues)`  
This command is used to construct a NodalLoad object and add it to the enclosing LoadPattern.

<table>
<thead>
<tr>
<th>nodeTag (int)</th>
<th>tag of node to which load is applied.</th>
</tr>
</thead>
<tbody>
<tr>
<td>loadValues (list (float))</td>
<td>ndf reference load values.</td>
</tr>
</tbody>
</table>

**Note:** The load values are reference loads values. It is the time series that provides the load factor. The load factor times the reference values is the load that is actually applied to the node.

eleLoad command

`eleLoad('-ele', *eleTags, '-range', eleTag1, eleTag2, '-type', '-beamUniform', Wy, Wz=0.0, Wx=0.0, '-beamPoint', Py, Pz=0.0, xL, Px=0.0, '-beamThermal', *tempPts)`  
The eleLoad command is used to construct an ElementalLoad object and add it to the enclosing LoadPattern.
<table>
<thead>
<tr>
<th>eleTags</th>
<th>tag of PREVIOUSLY DEFINED element</th>
</tr>
</thead>
<tbody>
<tr>
<td>eleTag1</td>
<td>element tag</td>
</tr>
<tr>
<td>eleTag2</td>
<td>element tag</td>
</tr>
<tr>
<td>Wx (float)</td>
<td>mag of uniformly distributed ref load acting in direction along member length. (optional)</td>
</tr>
<tr>
<td>Wy (float)</td>
<td>mag of uniformly distributed ref load acting in local y direction of element</td>
</tr>
<tr>
<td>Wz (float)</td>
<td>mag of uniformly distributed ref load acting in local z direction of element. (optional and only for 3D)</td>
</tr>
<tr>
<td>Px (float)</td>
<td>mag of ref point load acting in direction along member length. (optional)</td>
</tr>
<tr>
<td>Py (float)</td>
<td>mag of ref point load acting in local y direction of element</td>
</tr>
<tr>
<td>Pz (float)</td>
<td>mag of ref point load acting in local z direction of element. (optional and only for 3D)</td>
</tr>
<tr>
<td>xL (float)</td>
<td>location of point load relative to node I, prescribed as fraction of element length</td>
</tr>
<tr>
<td>tempPts</td>
<td>temperature points: tempPts = [T1, y1, T2, y2, ..., T9, y9] Each point (T1, y1) define a temperature and location. This command may accept 2, 5 or 9 temperature points.</td>
</tr>
</tbody>
</table>

**Note:**

1. The load values are reference loads values, it is the time series that provides the load factor. The load factor times the reference values is the load that is actually applied to the node.

2. At the moment, eleLoads do not work with 3D beam-column elements if Corotational geometric transformation is used.

**sp command**

**sp (nodeTag, dof, *dofValues)**

This command is used to construct a single-point constraint object and add it to the enclosing LoadPattern.

<table>
<thead>
<tr>
<th>nodeTag (int)</th>
<th>tag of node to which load is applied.</th>
</tr>
</thead>
<tbody>
<tr>
<td>dof (int)</td>
<td>the degree-of-freedom at the node to which constraint is applied (1 through ndf)</td>
</tr>
<tr>
<td>dofValues (list (float))</td>
<td>ndf reference constraint values.</td>
</tr>
</tbody>
</table>

**Note:** The dofValue is a reference value, it is the time series that provides the load factor. The load factor times the reference value is the constraint that is actually applied to the node.

**UniformExcitation Pattern**

**pattern** (*'UniformExcitation', patternTag, dir, *'-disp', dispSeriesTag, *'-vel', velSeriesTag, *'-accel', accelSeriesTag, *'-vel0', vel0, *'-fact', factor*)

The UniformExcitation pattern allows the user to apply a uniform excitation to a model acting in a certain direction. The command is as follows:
<table>
<thead>
<tr>
<th>patternTag (int)</th>
<th>unique tag among load patterns</th>
</tr>
</thead>
<tbody>
<tr>
<td>dir (int)</td>
<td>direction in which ground motion acts</td>
</tr>
<tr>
<td></td>
<td>1. corresponds to translation along the global X axis</td>
</tr>
<tr>
<td></td>
<td>2. corresponds to translation along the global Y axis</td>
</tr>
<tr>
<td></td>
<td>3. corresponds to translation along the global Z axis</td>
</tr>
<tr>
<td></td>
<td>4. corresponds to rotation about the global X axis</td>
</tr>
<tr>
<td></td>
<td>5. corresponds to rotation about the global Y axis</td>
</tr>
<tr>
<td></td>
<td>6. corresponds to rotation about the global Z axis</td>
</tr>
<tr>
<td>dispSeriesTag (int)</td>
<td>tag of the TimeSeries series defining the displacement history. (optional)</td>
</tr>
<tr>
<td>velSeriesTag (int)</td>
<td>tag of the TimeSeries series defining the velocity history. (optional)</td>
</tr>
<tr>
<td>accelSeriesTag (int)</td>
<td>tag of the TimeSeries series defining the acceleration history. (optional)</td>
</tr>
<tr>
<td>vel0 (float)</td>
<td>the initial velocity (optional, default=0.0)</td>
</tr>
<tr>
<td>factor (float)</td>
<td>constant factor (optional, default=1.0)</td>
</tr>
</tbody>
</table>

**Note:**

1. The responses obtained from the nodes for this type of excitation are RELATIVE values, and not the absolute values obtained from a multi-support case.
2. must set one of the disp, vel or accel time series

### Multi-Support Excitation Pattern

**pattern** ('MultipleSupport', patternTag)

The Multi-Support pattern allows similar or different prescribed ground motions to be input at various supports in the structure. In OpenSees, the prescribed motion is applied using single-point constraints, the single-point constraints taking their constraint value from user created ground motions.

**Note:**

1. The results for the responses at the nodes are the ABSOLUTE values, and not relative values as in the case of a UniformExcitation.
2. The non-homogeneous single point constraints require an appropriate choice of constraint handler.

### Plain Ground Motion

**groundMotion** (gmTag, 'Plain', '-disp', dispSeriesTag, '-vel', velSeriesTag, '-accel', accelSeriesTag, '-int',

This command is used to construct a plain GroundMotion object. Each GroundMotion object is associated with a number of TimeSeries objects, which define the acceleration, velocity and displacement records for that ground motion. T
### Interpolated Ground Motion

**groundMotion** *(gmTag, 'Interpolated', *gmTags, '-fact', facts)*

This command is used to construct an interpolated GroundMotion object, where the motion is determined by combining several previously defined ground motions in the load pattern.

<table>
<thead>
<tr>
<th>gmTag (int)</th>
<th>unique tag among ground motions in load pattern</th>
</tr>
</thead>
<tbody>
<tr>
<td>gmTags (list (int))</td>
<td>the tags of existing ground motions in pattern to be used for interpolation</td>
</tr>
<tr>
<td>facts (list (float))</td>
<td>the interpolation factors. (optional)</td>
</tr>
</tbody>
</table>

### Imposed Motion

**imposedMotion** *(nodeTag, dof, gmTag)*

This command is used to construct an ImposedMotionSP constraint which is used to enforce the response of a dof at a node in the model. The response enforced at the node at any given time is obtained from the GroundMotion object associated with the constraint.

<table>
<thead>
<tr>
<th>nodeTag (int)</th>
<th>tag of node on which constraint is to be placed</th>
</tr>
</thead>
<tbody>
<tr>
<td>dof (int)</td>
<td>dof of enforced response. Valid range is from 1 through ndf at node.</td>
</tr>
<tr>
<td>gmTag (int)</td>
<td>pre-defined GroundMotion object tag</td>
</tr>
</tbody>
</table>

### 1.4.8 mass command

**mass** *(nodeTag, *massValues)*

This command is used to set the mass at a node.

<table>
<thead>
<tr>
<th>nodeTag (int)</th>
<th>integer tag identifying node whose mass is set</th>
</tr>
</thead>
<tbody>
<tr>
<td>massValues (list (float))</td>
<td>ndf nodal mass values corresponding to each DOF</td>
</tr>
</tbody>
</table>
1.4.9 region command


The region command is used to label a group of nodes and elements. This command is also used to assign rayleigh damping parameters to the nodes and elements in this region. The region is specified by either elements or nodes, not both. If elements are defined, the region includes these elements and the all connected nodes, unless the -eleOnly option is used in which case only elements are included. If nodes are specified, the region includes these nodes and all elements of which all nodes are prescribed to be in the region, unless the -nodeOnly option is used in which case only the nodes are included.

<table>
<thead>
<tr>
<th>regTag (int)</th>
<th>unique integer tag</th>
</tr>
</thead>
<tbody>
<tr>
<td>eles (list (int))</td>
<td>tags of selected elements in domain to be included in region (optional)</td>
</tr>
<tr>
<td>nodes (list (int))</td>
<td>tags of selected nodes in domain to be included in region (optional)</td>
</tr>
<tr>
<td>startEle (int)</td>
<td>tag for start element (optional)</td>
</tr>
<tr>
<td>endEle (int)</td>
<td>tag for end element (optional)</td>
</tr>
<tr>
<td>startNode (int)</td>
<td>tag for start node (optional)</td>
</tr>
<tr>
<td>endNode (int)</td>
<td>tag for end node (optional)</td>
</tr>
<tr>
<td>alphaM (float)</td>
<td>factor applied to elements or nodes mass matrix (optional)</td>
</tr>
<tr>
<td>betaK (float)</td>
<td>factor applied to elements current stiffness matrix (optional)</td>
</tr>
<tr>
<td>betaKinit (float)</td>
<td>factor applied to elements initial stiffness matrix (optional)</td>
</tr>
<tr>
<td>betaKcomm (float)</td>
<td>factor applied to elements committed stiffness matrix (optional)</td>
</tr>
</tbody>
</table>

Note: The user cannot prescribe the region by BOTH elements and nodes.

1.4.10 rayleigh command

**rayleigh** (alphaM, betaK, betaKinit, betaKcomm)

This command is used to assign damping to all previously-defined elements and nodes. When using rayleigh damping in OpenSees, the damping matrix for an element or node, D is specified as a combination of stiffness and mass-proportional damping matrices:

\[
D = \alpha_M \cdot M + \beta_K \cdot K_{\text{curr}} + \beta_{K\text{init}} \cdot K_{\text{init}} + \beta_{K\text{comm}} \cdot K_{\text{comm}}
\]

<table>
<thead>
<tr>
<th>alphaM (float)</th>
<th>factor applied to elements or nodes mass matrix</th>
</tr>
</thead>
<tbody>
<tr>
<td>betaK (float)</td>
<td>factor applied to elements current stiffness matrix.</td>
</tr>
<tr>
<td>betaKinit (float)</td>
<td>factor applied to elements initial stiffness matrix.</td>
</tr>
<tr>
<td>betaKcomm (float)</td>
<td>factor applied to elements committed stiffness matrix.</td>
</tr>
</tbody>
</table>

1.4.11 block commands

Create a block of mesh

**block2D command**

**block2D** (numX, numY, startNode, startEle, eleType, *eleArgs, *crds)

Create mesh of quadrilateral elements
### OpenSeesPy Documentation, Release 0.4.2019.7

| numX (int) | number of elements in local x directions of the block. |
| numY (int) | number of elements in local y directions of the block. |
| startNode (int) | node from which the mesh generation will start. |
| startEle (int) | element from which the mesh generation will start. |
| eleType (str) | element type ('quad', 'shell', 'bbarQuad', 'enhancedQuad', or 'SSPquad') |
| eleArgs (list) | a list of element parameters. |
| crds (list) | coordinates of the block elements with the format: [1, x1, y1, <z1>, 2, x2, y2, <z2>, 3, x3, y3, <z3>, 4, x4, y4, <z4>, <5>, <x5>, <y5>, <z5>, <6>, <x6>, <y6>, <z6>, <7>, <x7>, <y7>, <z7>, <8>, <x8>, <y8>, <z8>, <9>, <x9>, <y9>, <z9>] <> means optional |

### block3D command

**block3D** *(numX, numY, numZ, startNode, startEle, eleType, *eleArgs, *crds)*

Create mesh of quadrilateral elements
numX (int) number of elements in local x directions of the block.

numY (int) number of elements in local y directions of the block.

cnumZ (int) number of elements in local z directions of the block.

startNode (int) node from which the mesh generation will start.

startEle (int) element from which the mesh generation will start.

eleType (str) element type ('stdBrick', 'bbarBrick', 'Brick20N')

eleArgs (list) a list of element parameters.

crds (list) coordinates of the block elements with the format:


<10>, <x10>, <y10>, <z10>,
<11>, <x11>, <y11>, <z11>,
<12>, <x12>, <y12>, <z12>,
<13>, <x13>, <y13>, <z13>,
<14>, <x14>, <y14>, <z14>,
<15>, <x15>, <y15>, <z15>,
<16>, <x16>, <y16>, <z16>,
<17>, <x17>, <y17>, <z17>,
<18>, <x18>, <y18>, <z18>,
<19>, <x19>, <y19>, <z19>,
<20>, <x20>, <y20>, <z20>,
<21>, <x21>, <y21>, <z21>,
<22>, <x22>, <y22>, <z22>,
<23>, <x23>, <y23>, <z23>,
<24>, <x24>, <y24>, <z24>,
<25>, <x25>, <y25>, <z25>,
<26>, <x26>, <y26>, <z26>,
<27>, <x27>, <y27>, <z27>]
<> means optional

1.4.12 beamIntegration commands

beamIntegration (type, tag, *args)

A wide range of numerical integration options are available in OpenSees to represent distributed plasticity or non-prismatic section details in Beam-Column Elements, i.e., across the entire element domain [0, L].

Following are beamIntegration types available in the OpenSees:

Integration Methods for Distributed Plasticity. Distributed plasticity methods permit yielding at any integration point
along the element length.

1. **Lobatto**
2. **Legendre**
3. **NewtonCotes**
4. **Radau**
5. **Trapezoidal**
6. **CompositeSimpson**
7. **UserDefined**
8. **FixedLocation**
9. **LowOrder**
10. **MidDistance**

### Lobatto

**beamIntegration** ('Lobatto', *tag*, *secTag*, *N*)

Create a Gauss-Lobatto beamIntegration object. Gauss-Lobatto integration is the most common approach for evaluating the response of forceBeamColumn (Neuenhofer and Filippou 1997) because it places an integration point at each end of the element, where bending moments are largest in the absence of interior element loads.

<table>
<thead>
<tr>
<th>Argument</th>
<th>Description</th>
</tr>
</thead>
<tbody>
<tr>
<td><em>tag</em> (int)</td>
<td>tag of the beam integration.</td>
</tr>
<tr>
<td><em>secTag</em> (int)</td>
<td>A previous-defined section object.</td>
</tr>
<tr>
<td><em>N</em> (int)</td>
<td>Number of integration points along the element.</td>
</tr>
</tbody>
</table>

### Legendre

**beamIntegration** ('Legendre', *tag*, *secTag*, *N*)

Create a Gauss-Legendre beamIntegration object. Gauss-Legendre integration is more accurate than Gauss-Lobatto; however, it is not common in force-based elements because there are no integration points at the element ends.

Places $N$ Gauss-Legendre integration points along the element. The location and weight of each integration point are tabulated in references on numerical analysis. The force deformation response at each integration point is defined by the section. The order of accuracy for Gauss-Legendre integration is $2N-1$.

Arguments and examples see **Lobatto**.

### NewtonCotes

**beamIntegration** ('NewtonCotes', *tag*, *secTag*, *N*)

Create a Newton-Cotes beamIntegration object. Newton-Cotes places integration points uniformly along the element, including a point at each end of the element.

Places $N$ Newton-Cotes integration points along the element. The weights for the uniformly spaced integration points are tabulated in references on numerical analysis. The force deformation response at each integration point is defined by the section. The order of accuracy for Gauss-Radau integration is $N-1$.

Arguments and examples see **Lobatto**.
Radau

beamIntegration (‘Radau’, tag, secTag, N)
Create a Gauss-Radau beamIntegration object. Gauss-Radau integration is not common in force-based elements
because it places an integration point at only one end of the element; however, it forms the basis for optimal
plastic hinge integration methods.

Places N Gauss-Radau integration points along the element with a point constrained to be at ndI. The location
and weight of each integration point are tabulated in references on numerical analysis. The force-deformation
response at each integration point is defined by the section. The order of accuracy for Gauss-Radau integration
is 2N-2.

Arguments and examples see Lobatto.

Trapezoidal

beamIntegration (‘Trapezoidal’, tag, secTag, N)
Create a Trapezoidal beamIntegration object.

Arguments and examples see Lobatto.

CompositeSimpson

beamIntegration (‘CompositeSimpson’, tag, secTag, N)
Create a CompositeSimpson beamIntegration object.

Arguments and examples see Lobatto.

UserDefined

Create a UserDefined beamIntegration object. This option allows user-specified locations and weights of the
integration points.

<table>
<thead>
<tr>
<th>Tag (int)</th>
<th>tag of the beam integration</th>
</tr>
</thead>
<tbody>
<tr>
<td>N (int)</td>
<td>number of integration points along the element.</td>
</tr>
<tr>
<td>secTags (list (int))</td>
<td>A list previous-defined section objects.</td>
</tr>
<tr>
<td>locs (list (float))</td>
<td>Locations of integration points along the element.</td>
</tr>
<tr>
<td>wts (list (float))</td>
<td>weights of integration points.</td>
</tr>
</tbody>
</table>

locs = [0.1, 0.3, 0.5, 0.7, 0.9]
wts = [0.2, 0.15, 0.3, 0.15, 0.2]
secs = [1, 2, 2, 2, 1]
beamIntegration (‘UserDefined’, 1, len(secs), *secs, *locs, *wts)

Places N integration points along the element, which are defined in locs on the natural domain [0, 1]. The
weight of each integration point is defined in the wts also on the [0, 1] domain. The force-deformation response
at each integration point is defined by the secs. The locs, wts, and secs should be of length N. In general,
there is no accuracy for this approach to numerical integration.
FixedLocation

beamIntegration('FixedLocation', tag, N, *secTags, *locs)

Create a FixedLocation beamIntegration object. This option allows user-specified locations of the integration points. The associated integration weights are computed by the method of undetermined coefficients (Vandermonde system)

\[
\sum_{i=1}^{N} x_i^{j-1} w_i = \int_{0}^{1} x^{j-1} dx = \frac{1}{j}, \quad (j = 1, ..., N)
\]

Note that NewtonCotes integration is recovered when the integration point locations are equally spaced.

<table>
<thead>
<tr>
<th>tag (int)</th>
<th>tag of the beam integration</th>
</tr>
</thead>
<tbody>
<tr>
<td>N (int)</td>
<td>number of integration points along the element.</td>
</tr>
<tr>
<td>secTags (list (int))</td>
<td>A list previous-defined section objects.</td>
</tr>
<tr>
<td>locs (list (float))</td>
<td>Locations of integration points along the element.</td>
</tr>
</tbody>
</table>

Places \( N \) integration points along the element, whose locations are defined in \( \text{locs} \) on the natural domain \([0, 1]\). The force-deformation response at each integration point is defined by the \( \text{secs} \). Both the \( \text{locs} \) and \( \text{secs} \) should be of length \( N \). The order of accuracy for Fixed Location integration is \( N-1 \).

LowOrder

beamIntegration('LowOrder', tag, N, *secTags, *locs, *wts)

Create a LowOrder beamIntegration object. This option is a generalization of the FixedLocation and UserDefined integration approaches and is useful for moving load analysis (Kidarsa, Scott and Higgins 2008). The locations of the integration points are user defined, while a selected number of weights are specified and the remaining weights are computed by the method of undetermined coefficients.

\[
\sum_{i=1}^{N_f} x_{fi}^{j-1} w_{fi} = \frac{1}{j} - \sum_{i=1}^{N_c} x_{ci}^{j-1} w_{ci}
\]

Note that FixedLocation integration is recovered when \( N_c \) is zero.

<table>
<thead>
<tr>
<th>tag (int)</th>
<th>tag of the beam integration</th>
</tr>
</thead>
<tbody>
<tr>
<td>N (int)</td>
<td>number of integration points along the element.</td>
</tr>
<tr>
<td>secTags (list (int))</td>
<td>A list previous-defined section objects.</td>
</tr>
<tr>
<td>locs (list (float))</td>
<td>Locations of integration points along the element.</td>
</tr>
<tr>
<td>wts (list (float))</td>
<td>weights of integration points.</td>
</tr>
</tbody>
</table>

\( \text{locs} = [0.0, 0.2, 0.5, 0.8, 1.0] \)
\( \text{wts} = [0.2, 0.2] \)
\( \text{secs} = [1, 2, 2, 2, 1] \)
beamIntegration('LowOrder',1,len(secs),*secs,*locs,*wts)

Places \( N \) integration points along the element, which are defined in \( \text{locs} \) on the natural domain \([0, 1]\). The force-deformation response at each integration point is defined by the \( \text{secs} \). Both the \( \text{locs} \) and \( \text{secs} \) should be of length \( N \). The \( \text{wts} \) at user-selected integration points are specified on \([0, 1]\), which can be of length \( N_c \) equals 0 up to \( N \). These specified weights are assigned to the first \( N_c \) entries in the \( \text{locs} \) and \( \text{secs} \), respectively. The order of accuracy for Low Order integration is \( N-N_c-1 \).
**Note:** $N_c$ is determined from the length of the $wts$ list. Accordingly, *FixedLocation* integration is recovered when $wts$ is an empty list and *UserDefined* integration is recovered when the $wts$ and $locs$ lists are of equal length.

### MidDistance

**beamIntegration** ('MidDistance', tag, $N$, *secTags, *locs)

Create a MidDistance beamIntegration object. This option allows user-specified locations of the integration points. The associated integration weights are determined from the midpoints between adjacent integration point locations. $w_i = (x_{i+1} - x_{i-1})/2$ for $i = 2 \ldots N - 1$, $w_1 = (x_1 + x_2)/2$, and $w_N = 1 - (x_{N-1} + x_N)/2$.

<table>
<thead>
<tr>
<th>tag (int)</th>
<th>tag of the beam integration</th>
</tr>
</thead>
<tbody>
<tr>
<td>$N$ (int)</td>
<td>number of integration points along the element.</td>
</tr>
<tr>
<td>secTags (list (int))</td>
<td>A list previous-defined section objects.</td>
</tr>
<tr>
<td>locs (list (float))</td>
<td>Locations of integration points along the element.</td>
</tr>
</tbody>
</table>

```
locs = [0.0, 0.2, 0.5, 0.8, 1.0]
secs = [1,2,2,2,1]
beamIntegration('MidDistance',1,len(secs),*secs,*locs)
```

Places $N$ integration points along the element, whose locations are defined in $locs$ on the natural domain $[0, 1]$. The force-deformation response at each integration point is defined by the $secs$. Both the $locs$ and $secs$ should be of length $N$. This integration rule can only integrate constant functions exactly since the sum of the integration weights is one.

For the $locs$ shown above, the associated integration weights will be $[0.15, 0.2, 0.3, 0.2, 0.15]$.

### Plastic Hinge Integration Methods

Plastic hinge integration methods confine material yielding to regions of the element of specified length while the remainder of the element is linear elastic. A summary of plastic hinge integration methods is found in (Scott and Fenves 2006).

1. *UserHinge*
2. *HingeMidpoint*
3. *HingeRadau*
4. *HingeRadauTwo*
5. *HingeEndpoint*

### UserHinge


Create a UserHinge beamIntegration object.
OpenSeesPy Documentation, Release 0.4.2019.7

### HingeMidpoint

**beamIntegration** (`'HingeMidpoint', tag, secI, lpI, secJ, lpJ, secE)

Create a HingeMidpoint beamIntegration object. Midpoint integration over each hinge region is the most accurate one-point integration rule; however, it does not place integration points at the element ends and there is a small integration error for linear curvature distributions along the element.

<table>
<thead>
<tr>
<th>tag (int)</th>
<th>tag of the beam integration.</th>
</tr>
</thead>
<tbody>
<tr>
<td>secI (int)</td>
<td>A previous-defined section object for hinge at I.</td>
</tr>
<tr>
<td>lpI (float)</td>
<td>The plastic hinge length at I.</td>
</tr>
<tr>
<td>secJ (int)</td>
<td>A previous-defined section object for hinge at J.</td>
</tr>
<tr>
<td>lpJ (float)</td>
<td>The plastic hinge length at J.</td>
</tr>
<tr>
<td>secE (int)</td>
<td>A previous-defined section object for the element interior.</td>
</tr>
</tbody>
</table>

The plastic hinge length at end I (J) is equal to `lpI` (`lpJ`) and the associated force deformation response is defined by the `secI` (`secJ`). The force deformation response of the element interior is defined by the `secE`. Typically, the interior section is linear-elastic, but this is not necessary.

```
lpI = 0.1
lpJ = 0.2
beamIntegration('HingeMidpoint', secI, lpI, secJ, lpJ, secE)
```
HingeRadau

beamIntegration ('HingeRadau', tag, secI, lpI, secJ, lpJ, secE)

Create a HingeRadau beamIntegration object. Two-point Gauss-Radau integration over each hinge region places
an integration point at the element ends and at 2/3 the hinge length inside the element. This approach represents
linear curvature distributions exactly; however, the characteristic length for softening plastic hinges is not equal
to the assumed plastic hinge length.

Arguments and examples see HingeMidpoint.

HingeRadauTwo

beamIntegration ('HingeRadauTwo', tag, secI, lpI, secJ, lpJ, secE)

Create a HingeRadauTwo beamIntegration object. Modified two-point Gauss-Radau integration over each hinge
region places an integration point at the element ends and at 8/3 the hinge length inside the element. This ap-
proach represents linear curvature distributions exactly and the characteristic length for softening plastic hinges
is equal to the assumed plastic hinge length.

Arguments and examples see HingeMidpoint.

HingeEndpoint

beamhingeEndpoint (tag, secI, lpI, secJ, lpJ, secE)

Create a HingeEndpoint beamIntegration object. Endpoint integration over each hinge region moves the inte-
gration points to the element ends; however, there is a large integration error for linear curvature distributions
along the element.

Arguments and examples see HingeMidpoint.

1.4.13 uniaxialMaterial commands

uniaxialMaterial (matType, matTag, *matArgs)

This command is used to construct a UniaxialMaterial object which represents uniaxial stress-strain (or force-
deformation) relationships.

<table>
<thead>
<tr>
<th>matType (str)</th>
<th>material type</th>
</tr>
</thead>
<tbody>
<tr>
<td>matTag (int)</td>
<td>material tag.</td>
</tr>
</tbody>
</table>
| matArgs (list)| a list of material arguments, must be preceded with *.

For example,

```
matType = 'Steel01'
matTag = 1
matArgs = [Fy, E0, b]
uniaxialMaterial(matType, matTag, *matArgs)
```

The following contain information about available matType:

**Steel & Reinforcing-Steel Materials**

1. *Steel01*
2. *Steel02*
3. Steel4
4. Hysteretic
5. ReinforcingSteel
6. Dodd_Restrepo
7. RambergOsgoodSteel
8. SteelMPF
9. Steel01Thermal

**Steel01**

**uniaxialMaterial** (‘Steel01’, matTag, Fy, E0, b, a1, a2, a3, a4)
This command is used to construct a uniaxial bilinear steel material object with kinematic hardening and optional isotropic hardening described by a non-linear evolution equation (REF: Fedeas).

<table>
<thead>
<tr>
<th>Parameter</th>
<th>Description</th>
</tr>
</thead>
<tbody>
<tr>
<td>matTag</td>
<td>integer tag identifying material</td>
</tr>
<tr>
<td>Fy</td>
<td>yield strength</td>
</tr>
<tr>
<td>E0</td>
<td>initial elastic tangent</td>
</tr>
<tr>
<td>b</td>
<td>strain-hardening ratio (ratio between post-yield tangent and initial elastic tangent)</td>
</tr>
<tr>
<td>a1</td>
<td>isotropic hardening parameter, increase of compression yield envelope as proportion of yield strength after a plastic strain of $a_2 \times (F_y/E_0)$ (optional)</td>
</tr>
<tr>
<td>a2</td>
<td>isotropic hardening parameter (see explanation under a1). (optional).</td>
</tr>
<tr>
<td>a3</td>
<td>isotropic hardening parameter, increase of tension yield envelope as proportion of yield strength after a plastic strain of $a_4 \times (F_y/E_0)$. (optional)</td>
</tr>
<tr>
<td>a4</td>
<td>isotropic hardening parameter (see explanation under a3). (optional)</td>
</tr>
</tbody>
</table>

**Note:** If strain-hardening ratio is zero and you do not expect softening of your system use BandSPD solver.

**Steel02**

**uniaxialMaterial** (‘Steel02’, matTag, Fy, E0, b, *params, a1=a2*Fy/E0, a2=1.0, a3=a4*Fy/E0, a4=1.0, sigInit=0.0)
This command is used to construct a uniaxial Giuffre-Menegotto-Pinto steel material object with isotropic strain hardening.
matTag (int)  
integer tag identifying material

Fy (float)  
yield strength

E0 (float)  
initial elastic tangent

b (float)  
strain-hardening ratio (ratio between post-yield tangent and initial elastic tangent)

params (list (float))  
parameters to control the transition from elastic to plastic branches.  
params=[R0,cR1,cR2].  
Recommended values:  R0=between 10 and 20,  
cR1=0.925, cR2=0.15

a1 (float)  
isotropic hardening parameter, increase of compression yield envelope as proportion of yield strength after a plastic strain of  \( a_2 \times (F_y/E_0) \) (optional)

a2 (float)  
isotropic hardening parameter (see explanation under a1). (optional).

a3 (float)  
isotropic hardening parameter, increase of tension yield envelope as proportion of yield strength after a plastic strain of  \( a_4 \times (F_y/E_0) \). (optional)

a4 (float)  
isotropic hardening parameter (see explanation under a3). (optional)

sigInit (float)  
Initial Stress Value (optional, default: 0.0) the strain is calculated from  
\[ \text{eps} = \begin{cases} \frac{\text{sigInit}}{E} & \text{if } \text{sigInit} \neq 0.0 \\ & \text{else} \text{eps} = \text{trialStrain} \end{cases} \]

See also:
Steel02

Steel4

uniaxialMaterial('Steel4', matTag, Fy, E0, '-asym', '-kin', b_k, R_0, r_1, r_2, b_kc, R_0c, r_1c, r_2c,  
'-iso', b_i, rho_i, b_l, R_i, l_yp, b_ic, rho_ic, b_lc, R_ic, 'ult', f_u, R_u, f_uc, R_uc,  
'-init', sig_init, '-mem', cycNum)

This command is used to construct a general uniaxial material with combined kinematic and isotropic hardening and optional non-symmetric behavior.
<table>
<thead>
<tr>
<th>matTag (int)</th>
<th>integer tag identifying material</th>
</tr>
</thead>
<tbody>
<tr>
<td>Fy (float)</td>
<td>yield strength</td>
</tr>
<tr>
<td>E0 (float)</td>
<td>initial elastic tangent</td>
</tr>
<tr>
<td>'−kin' (str)</td>
<td>apply kinematic hardening</td>
</tr>
<tr>
<td>b_k (float)</td>
<td>hardening ratio ($E_k/E_0$)</td>
</tr>
<tr>
<td>R_0, r_1, r_2 (float)</td>
<td>control the exponential transition from linear elastic to hardening asymptote recommended values: $R_0 = 20$, $r_1 = 0.90$, $r_2 = 0.15$</td>
</tr>
<tr>
<td>'−iso' (str)</td>
<td>apply isotropic hardening</td>
</tr>
<tr>
<td>b_i (float)</td>
<td>initial hardening ratio ($E_i/E_0$)</td>
</tr>
<tr>
<td>b_l (float)</td>
<td>saturated hardening ratio ($E_s/E_0$)</td>
</tr>
<tr>
<td>rho_i (float)</td>
<td>specifies the position of the intersection point between initial and saturated hardening asymptotes</td>
</tr>
<tr>
<td>R_l (float)</td>
<td>control the exponential transition from initial to saturated asymptote</td>
</tr>
<tr>
<td>l_yp (float)</td>
<td>length of the yield plateau in $\text{eps}_y0 = f_y / E_0$ units</td>
</tr>
<tr>
<td>'−ult' (str)</td>
<td>apply an ultimate strength limit</td>
</tr>
<tr>
<td>f_u (float)</td>
<td>ultimate strength</td>
</tr>
<tr>
<td>R_u (float)</td>
<td>control the exponential transition from kinematic hardening to perfectly plastic asymptote</td>
</tr>
<tr>
<td>'−asym' (str)</td>
<td>assume non-symmetric behavior</td>
</tr>
<tr>
<td>'−init' (str)</td>
<td>apply initial stress</td>
</tr>
<tr>
<td>sig_init (float)</td>
<td>initial stress value</td>
</tr>
<tr>
<td>'−mem' (str)</td>
<td>configure the load history memory</td>
</tr>
<tr>
<td>cycNum (float)</td>
<td>expected number of half-cycles during the loading process Efficiency of the material can be slightly increased by correctly setting this value. The default value is $cycNum = 50$. Load history memory can be turned off by setting $cycNum = 0$.</td>
</tr>
</tbody>
</table>

See also:

Steel4
Hysteretic

`uniaxialMaterial('Hysteretic', matTag, *p1, *p2, *p3=p2, *n1, *n2, *n3=n2, pinchX, pinchY, damage1, damage2, beta)`

This command is used to construct a uniaxial bilinear hysteretic material object with pinching of force and deformation, damage due to ductility and energy, and degraded unloading stiffness based on ductility.

<table>
<thead>
<tr>
<th>matTag</th>
<th>integer tag identifying material</th>
</tr>
</thead>
<tbody>
<tr>
<td>p1, p2, p3</td>
<td>list (float) stress and strain (or force &amp; deformation) at respective points</td>
</tr>
<tr>
<td>n1, n2, n3</td>
<td>list (float) stress and strain (or force &amp; deformation) at respective points</td>
</tr>
<tr>
<td>pinchX, pinchY</td>
<td>pinching factor for strain (or deformation) during reloading</td>
</tr>
<tr>
<td>damage1, damage2</td>
<td>damage due to ductility: D1(mu-1), damage due to energy: D2(Eii/Eult)</td>
</tr>
<tr>
<td>beta</td>
<td>power used to determine the degraded unloading stiffness based on ductility, mu-beta (optional, default=0.0)</td>
</tr>
</tbody>
</table>

See also:
Steel4

ReinforcingSteel

`uniaxialMaterial('ReinforcingSteel', matTag, fy, fu, Es, Esh, esh, eult, '-GABuck', lsr, beta, r, gama, '-DMBuck', lsr, alpha=1.0, '-CMFatigue', Cf, alpha, Cd, '-IsoHard', a1=4.3, limit=1.0, '-MPCurveParams', R1=0.333, R2=18.0, R3=4.0)`

This command is used to construct a ReinforcingSteel uniaxial material object. This object is intended to be used in a reinforced concrete fiber section as the steel reinforcing material.
<table>
<thead>
<tr>
<th>matTag (int)</th>
<th>integer tag identifying material</th>
</tr>
</thead>
<tbody>
<tr>
<td>fy (float)</td>
<td>Yield stress in tension</td>
</tr>
<tr>
<td>fu (float)</td>
<td>Ultimate stress in tension</td>
</tr>
<tr>
<td>Es (float)</td>
<td>Initial elastic tangent</td>
</tr>
<tr>
<td>Esh (float)</td>
<td>Tangent at initial strain hardening</td>
</tr>
<tr>
<td>esh (float)</td>
<td>Strain corresponding to initial strain hardening</td>
</tr>
<tr>
<td>sult (float)</td>
<td>Strain at peak stress</td>
</tr>
<tr>
<td>'-GABuck' (str)</td>
<td>Buckling Model Based on Gomes and Appleton (1997)</td>
</tr>
<tr>
<td>lsr (float)</td>
<td>Slenderness Ratio</td>
</tr>
<tr>
<td>beta (float)</td>
<td>Amplification factor for the buckled stress strain curve.</td>
</tr>
<tr>
<td>r (float)</td>
<td>Buckling reduction factor</td>
</tr>
<tr>
<td>gamma (float)</td>
<td>Buckling constant</td>
</tr>
<tr>
<td>'-DMBuck' (str)</td>
<td>Buckling model based on Dhakal and Maekawa (2002)</td>
</tr>
<tr>
<td>lsr (float)</td>
<td>Slenderness Ratio</td>
</tr>
<tr>
<td>alpha (float)</td>
<td>Adjustment Constant usually between 0.75 and 1.0 Default: alpha=1.0, this parameter is optional.</td>
</tr>
<tr>
<td>'-CMFatigue' (str)</td>
<td>Coffin-Manson Fatigue and Strength Reduction</td>
</tr>
<tr>
<td>Cf (float)</td>
<td>Coffin-Manson constant C</td>
</tr>
<tr>
<td>alpha (float)</td>
<td>Coffin-Manson constant a</td>
</tr>
<tr>
<td>Cd (float)</td>
<td>Cyclic strength reduction constant</td>
</tr>
<tr>
<td>'-IsoHard;' (str)</td>
<td>Isotropic Hardening / Diminishing Yield Plateau</td>
</tr>
<tr>
<td>limit (float)</td>
<td>Hardening constant (default = 4.3)</td>
</tr>
<tr>
<td>'-MPCurveParams' (str)</td>
<td>Menegotto and Pinto Curve Parameters</td>
</tr>
<tr>
<td>R1 (float)</td>
<td>(default = 0.333)</td>
</tr>
<tr>
<td>R2 (float)</td>
<td>(default = 18)</td>
</tr>
<tr>
<td>R3 (float)</td>
<td>(default = 4)</td>
</tr>
</tbody>
</table>

See also:

Notes

**Dodd_Restrepo**

**uniaxialMaterial** (*'Dodd_Restrepo', matTag, fy, Fu, ESh, ESHI, Youngs, ESHI, FSHI, OmegaFac=1.0*)

This command is used to construct a Dodd-Restrepo steel material
### MatTag (int)
Integer tag identifying material

### Fy (float)
Yield strength

### Fsu (float)
Ultimate tensile strength (UTS)

### ESH (float)
Tensile strain at initiation of strain hardening

### ESU (float)
Tensile strain at the UTS

### Youngs (float)
Modulus of elasticity

### ESHI (float)
Tensile strain for a point on strain hardening curve, recommended range of values for ESHI: 
\[(ESU + 5*ESH)/6, (ESU + 3*ESH)/4\]

### FSXI (float)
Tensile stress at point on strain hardening curve corresponding to ESHI

### OmegaFac (float)
Roundedness factor for Bauschinger curve in cycle reversals from the strain hardening curve.
Range: [0.75, 1.15]. Largest value tends to near a bilinear Bauschinger curve. Default = 1.0.

#### See also:

#### Notes

---

### RambergOsgoodSteel

**uniaxialMaterial** (`'RambergOsgoodSteel', matTag, fy, E0, a, n)"

This command is used to construct a Ramberg–Osgood steel material object.

### ESHI (float)
Tensile strain for a point on strain hardening curve, recommended range of values for ESHI:
\[(ESU + 5*ESH)/6, (ESU + 3*ESH)/4\]

### FSXI (float)
Tensile stress at point on strain hardening curve corresponding to ESHI

#### See also:

#### Notes

---

### SteelMPF

**uniaxialMaterial** (`'SteelMPF', matTag, fyp, fyn, E0, bp, bn, R0, cR1, cR2, a1=0.0, a2=1.0, a3=0.0, a4=1.0)"

This command is used to construct a uniaxialMaterial SteelMPF (Kolozvari et al., 2015), which represents the well-known uniaxial constitutive nonlinear hysteretic material model for steel proposed by Menegotto and Pinto (1973), and extended by Filippou et al. (1983) to include isotropic strain hardening effects.

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<table>
<thead>
<tr>
<th>matTag (int)</th>
<th>integer tag identifying material</th>
</tr>
</thead>
<tbody>
<tr>
<td>fyp (float)</td>
<td>Yield strength in tension (positive loading direction)</td>
</tr>
<tr>
<td>fyn (float)</td>
<td>Yield strength in compression (negative loading direction)</td>
</tr>
<tr>
<td>E0 (float)</td>
<td>Initial tangent modulus</td>
</tr>
<tr>
<td>bp (float)</td>
<td>Strain hardening ratio in tension (positive loading direction)</td>
</tr>
<tr>
<td>bn (float)</td>
<td>Strain hardening ratio in compression (negative loading direction)</td>
</tr>
<tr>
<td>R0 (float)</td>
<td>Initial value of the curvature parameter R (R0 = 20 recommended)</td>
</tr>
<tr>
<td>cR1 (float)</td>
<td>Curvature degradation parameter (a1 = 0.925 recommended)</td>
</tr>
<tr>
<td>cR2 (float)</td>
<td>Curvature degradation parameter (a2 = 0.15 or 0.0015 recommended)</td>
</tr>
<tr>
<td>a1 (float)</td>
<td>Isotropic hardening in compression parameter (optional, default = 0.0). Shifts compression yield envelope by a proportion of compressive yield strength after a maximum plastic tensile strain of a2(fyp/E0).</td>
</tr>
<tr>
<td>a2 (float)</td>
<td>Isotropic hardening in compression parameter (optional, default = 1.0).</td>
</tr>
<tr>
<td>a3 (float)</td>
<td>Isotropic hardening in tension parameter (optional, default = 0.0). Shifts tension yield envelope by a proportion of tensile yield strength after a maximum plastic compressive strain of a3(fyn/E0).</td>
</tr>
<tr>
<td>a4 (float)</td>
<td>Isotropic hardening in tension parameter (optional, default = 1.0). See explanation of a3.</td>
</tr>
</tbody>
</table>

See also:

Notes

**Steel01Thermal**

`uniaxialMaterial ('Steel01Thermal', matTag, Fy, E0, b, a1, a2, a3, a4)`

This command is the thermal version for 'Steel01'.

<table>
<thead>
<tr>
<th>matTag (int)</th>
<th>integer tag identifying material</th>
</tr>
</thead>
<tbody>
<tr>
<td>Fy (float)</td>
<td>yield strength</td>
</tr>
<tr>
<td>E0 (float)</td>
<td>initial elastic tangent</td>
</tr>
<tr>
<td>b (float)</td>
<td>strain-hardening ratio (ratio between post-yield tangent and initial elastic tangent)</td>
</tr>
<tr>
<td>a1 (float)</td>
<td>isotropic hardening parameter, increase of compression yield envelope as proportion of yield strength after a plastic strain of a2 * (Fy/E0) (optional)</td>
</tr>
<tr>
<td>a2 (float)</td>
<td>isotropic hardening parameter (see explanation under a1), (optional).</td>
</tr>
<tr>
<td>a3 (float)</td>
<td>isotropic hardening parameter, increase of tension yield envelope as proportion of yield strength after a plastic strain of a4 * (Fy/E0). (optional)</td>
</tr>
<tr>
<td>a4 (float)</td>
<td>isotropic hardening parameter (see explanation under a3), (optional)</td>
</tr>
</tbody>
</table>
Concrete Materials

1. Concrete01
2. Concrete02
3. Concrete04
4. Concrete06
5. Concrete07
6. Concrete01WithSITC
7. ConfinedConcrete01
8. ConcreteD
9. FRPConfinedConcrete
10. ConcreteCM

Concrete01

uniaxialMaterial ('Concrete01', matTag, fpc, epsc0, fpcu, epsU)

This command is used to construct a uniaxial Kent-Scott-Park concrete material object with degraded linear unloading/reloading stiffness according to the work of Karsan-Jirsa and no tensile strength. (REF: Fedeas).

<table>
<thead>
<tr>
<th>matTag (int)</th>
<th>integer tag identifying material</th>
</tr>
</thead>
<tbody>
<tr>
<td>fpc (float)</td>
<td>concrete compressive strength at 28 days (compression is negative)</td>
</tr>
<tr>
<td>epsc0 (float)</td>
<td>concrete strain at maximum strength</td>
</tr>
<tr>
<td>fpcu (float)</td>
<td>concrete crushing strength</td>
</tr>
<tr>
<td>epsU (float)</td>
<td>concrete strain at crushing strength</td>
</tr>
</tbody>
</table>

Note:

1. Compressive concrete parameters should be input as negative values (if input as positive, they will be converted to negative internally).
2. The initial slope for this model is (2*fpc/epsc0)

See also:

Notes

Concrete02

uniaxialMaterial ('Concrete02', matTag, fpc, epsc0, fpcu, epsU, lambda, ft, Ets)

This command is used to construct a uniaxial Kent-Scott-Park concrete material object with degraded linear unloading/reloading stiffness according to the work of Karsan-Jirsa and no tensile strength. (REF: Fedeas).
matTag (int)  | integer tag identifying material
fpc  (float) | concrete compressive strength at 28 days (compression is negative)
epsc0  (float) | concrete strain at maximum strength
fpcu  (float) | concrete crushing strength
epscU  (float) | concrete strain at crushing strength
lambda  (float) | ratio between unloading slope at $\text{epscu}$ and initial slope
ft  (float) | tensile strength
Ets  (float) | tension softening stiffness (absolute value) (slope of the linear tension softening branch)

Note:

1. Compressive concrete parameters should be input as negative values (if input as positive, they will be converted to negative internally).
2. The initial slope for this model is $(2*fpc/epsc0)$

See also:

Concrete04

uniaxialMaterial ('Concrete04', matTag, fc, ec, ecu, Ec, fct, et, beta)

This command is used to construct a uniaxial Popovics concrete material object with degraded linear unloading/reloading stiffness according to the work of Karsan-Jirsa and tensile strength with exponential decay.

matTag (int)  | integer tag identifying material
fc  (float) | floating point values defining concrete compressive strength at 28 days (compression is negative)
ec  (float) | floating point values defining concrete strain at maximum strength
ecu  (float) | floating point values defining concrete strain at crushing strength
Ec  (float) | floating point values defining initial stiffness
fct  (float) | floating point values defining the maximum tensile strength of concrete (optional)
et  (float) | floating point values defining ultimate tensile strain of concrete (optional)
beta  (float) | floating point value defining the exponential curve parameter to define the residual stress (as a factor of ft) at etu

Note:

1. Compressive concrete parameters should be input as negative values.
2. The envelope of the compressive stress-strain response is defined using the model proposed by Popovics (1973). If the user defines $Ec = 57000 * sqrt(|fcc|)$ (in psi) then the envelope curve is identical to proposed by Mander et al. (1988).
3. Model Characteristic: For loading in compression, the envelope to the stress-strain curve follows the model proposed by Popovics (1973) until the concrete crushing strength is achieved and also for strains beyond that corresponding to the crushing strength. For unloading and reloading in compression, the Karsan-Jirsa model (1969) is used to determine the slope of the curve. For tensile loading, an exponential curve is used to define the envelope to the stress-strain curve. For unloading and reloading in tensile, the secant stiffness is used to define the path.
See also:

Notes

Concrete06

**uniaxialMaterial** *(Concrete06, matTag, fc, e0, n, k, alpha1, fcr, ecr, b, alpha2)*

This command is used to construct a uniaxial concrete material object with tensile strength, nonlinear tension stiffening and compressive behavior based on Thorenfeldt curve.

<table>
<thead>
<tr>
<th>matTag (int)</th>
<th>integer tag identifying material</th>
</tr>
</thead>
<tbody>
<tr>
<td>fc (float)</td>
<td>concrete compressive strength (compression is negative)</td>
</tr>
<tr>
<td>e0 (float)</td>
<td>strain at compressive strength</td>
</tr>
<tr>
<td>n (float)</td>
<td>compressive shape factor</td>
</tr>
<tr>
<td>k (float)</td>
<td>post-peak compressive shape factor</td>
</tr>
<tr>
<td>alpha1 (float)</td>
<td>$\alpha_1$ parameter for compressive plastic strain definition</td>
</tr>
<tr>
<td>fcr (float)</td>
<td>tensile strength</td>
</tr>
<tr>
<td>ecr (float)</td>
<td>tensile strain at peak stress (fcr)</td>
</tr>
<tr>
<td>b (float)</td>
<td>exponent of the tension stiffening curve</td>
</tr>
<tr>
<td>alpha2 (float)</td>
<td>$\alpha_2$ parameter for tensile plastic strain definition</td>
</tr>
</tbody>
</table>

Note:

1. Compressive concrete parameters should be input as negative values.

See also:

Notes

Concrete07

**uniaxialMaterial** *(Concrete07, matTag, fc, ec, Ec, ft, et, xp, xn, r)*

Concrete07 is an implementation of Chang & Mander's 1994 concrete model with simplified unloading and reloading curves. Additionally the tension envelope shift with respect to the origin proposed by Chang and Mander has been removed. The model requires eight input parameters to define the monotonic envelope of confined and unconfined concrete in the following form:

<table>
<thead>
<tr>
<th>matTag (int)</th>
<th>integer tag identifying material</th>
</tr>
</thead>
<tbody>
<tr>
<td>fc (float)</td>
<td>concrete compressive strength (compression is negative)</td>
</tr>
<tr>
<td>ec (float)</td>
<td>concrete strain at maximum compressive strength</td>
</tr>
<tr>
<td>Ec (float)</td>
<td>Initial Elastic modulus of the concrete</td>
</tr>
<tr>
<td>ft (float)</td>
<td>tensile strength of concrete (tension is positive)</td>
</tr>
<tr>
<td>et (float)</td>
<td>tensile strain at max tensile strength of concrete</td>
</tr>
<tr>
<td>xp (float)</td>
<td>Non-dimensional term that defines the strain at which the straight line descent begins in tension</td>
</tr>
<tr>
<td>xn (float)</td>
<td>Non-dimensional term that defines the strain at which the straight line descent begins in compression</td>
</tr>
<tr>
<td>r (float)</td>
<td>Parameter that controls the nonlinear descending branch</td>
</tr>
</tbody>
</table>
Concrete01WithSITC

This command is used to construct a modified uniaxial Kent-Scott-Park concrete material object with degraded linear unloading/reloading stiffness according to the work of Karsan-Jirsa and no tensile strength. The modification is to model the effect of Stuff In The Cracks (SITC).

| matTag (int) | integer tag identifying material |
| fpc (float) | concrete compressive strength at 28 days (compression is negative) |
| epsc0 (float) | concrete strain at maximum strength |
| fpcu (float) | concrete crushing strength |
| epsU (float) | concrete strain at crushing strength |
| endStrainSITC (float) | optional, default = 0.03 |

Note:

1. Compressive concrete parameters should be input as negative values (if input as positive, they will be converted to negative internally).

2. The initial slope for this model is \( 2fpc/epsc0 \).

See also:

Notes

ConfinedConcrete01

ConfinedConcrete01 (‘ConfinedConcrete01’, matTag, secType, fpc, Ec, ‘-epscu’, epscu, ‘-gamma’, gamma, ‘-nu’, nu, ‘-varub’, ‘-varnoub’, L1, L2, L3, phiS, S, fyh, Es0, haRatio, mu, phiLOn, ‘-internal’, *intArg, ‘-wrap’, *wrapArg, ‘-gravel’, ‘-silica’, ‘-tol’, tol, ‘-maxNumIter’, maxNumIter, ‘-epscuLimit’, epscuLimit, ‘-stRatio’, stRatio)
matTag (int)  
integer tag identifying material

secType (str)  
tag for the transverse reinforcement configuration. see image below.
- 'S1' square section with S1 type of transverse reinforcement with or without external FRP wrapping
- 'S2' square section with S2 type of transverse reinforcement with or without external FRP wrapping
- 'S3' square section with S3 type of transverse reinforcement with or without external FRP wrapping
- 'S4a' square section with S4a type of transverse reinforcement with or without external FRP wrapping
- 'S4b' square section with S4b type of transverse reinforcement with or without external FRP wrapping
- 'S5' square section with S5 type of transverse reinforcement with or without external FRP wrapping
- 'C' circular section with or without external FRP wrapping
- 'R' rectangular section with or without external FRP wrapping.

fpc (float)  
unconfined cylindrical strength of concrete specimen.

Ec (float)  
initial elastic modulus of unconfined concrete.

epscu (float)  
confined concrete ultimate strain. (optional)

gamma (float)  
the ratio of the strength corresponding to ultimate strain to the peak strength of the confined concrete stress-strain curve. If \( \gamma \) cannot be achieved in the range \([0, \text{epscuLimit}]\) then \( \text{epscuLimit} \) (optional, default: 0.05) will be assumed as ultimate strain.

nu (float)  
Poisson’s Ratio.

'-varub' (float)  
Poisson’s ratio is defined as a function of axial strain by means of the expression proposed by Braga et al. (2006) with the upper bound equal to 0.5

'-varnoub' (float)  
Poisson’s ratio is defined as a function of axial strain by means of the expression proposed by Braga et al. (2006) without any upper bound.

L1 (float)  
length/diameter of square/circular core section measured respect to the hoop center line.

L2 (float)  
additional dimensions when multiple hoops are being used.

L3 (float)  
additional dimensions when multiple hoops are being used.

phis (float)  
hoop diameter. If section arrangement has multiple hoops it refers to the external hoop.

S (float)  
hoop spacing.

fyh (float)  
yielding strength of the hoop steel.

Es0 (float)  
elastic modulus of the hoop steel.

haRatio (float)  
hardening ratio of the hoop steel.

mu (float)  
ductility factor of the hoop steel.

phiLon (float)  
diameter of longitudinal bars.

intArgs (list (float))  
intArgs= [phis, S, fyh, Es0, haRatio, mui] optional parameters for defining the internal transverse reinforcement. If they are not specified, they will be assumed equal to the external ones (for S2, S3, S4a, S4b and S5 typed).
ConcreteD

uniaxialMaterial ('ConcreteD', matTag, fc, epsc, ft, epst, Ec, alphac, alphat, cesp=0.25, etap=1.15)

This command is used to construct a concrete material based on the Chinese design code.

<table>
<thead>
<tr>
<th>Parameter</th>
<th>Description</th>
</tr>
</thead>
<tbody>
<tr>
<td>matTag (int)</td>
<td>integer tag identifying material</td>
</tr>
<tr>
<td>fc (float)</td>
<td>concrete compressive strength</td>
</tr>
<tr>
<td>epsc (float)</td>
<td>concrete strain at corresponding to compressive strength</td>
</tr>
<tr>
<td>ft (float)</td>
<td>concrete tensile strength</td>
</tr>
<tr>
<td>epst (float)</td>
<td>concrete strain at corresponding to tensile strength</td>
</tr>
<tr>
<td>Ec (float)</td>
<td>concrete initial Elastic modulus</td>
</tr>
<tr>
<td>alphac (float)</td>
<td>compressive descending parameter</td>
</tr>
<tr>
<td>alphat (float)</td>
<td>tensile descending parameter</td>
</tr>
<tr>
<td>cesp (float)</td>
<td>plastic parameter, recommended values: 0.2–0.3</td>
</tr>
<tr>
<td>etap (float)</td>
<td>plastic parameter, recommended values: 1.0–1.3</td>
</tr>
</tbody>
</table>

Note:

1. Concrete compressive strength and the corresponding strain should be input as negative values.
2. The value fc/epsc and ft/epst should be smaller than Ec.

See also:
Notes

FRPConfinedConcrete

**uniaxialMaterial** (`'FRPConfinedConcrete'`, matTag, fpc1, fpc2, epsc0, D, c, Ej, Sj, tj, eju, S, fyl, fyh, dlong, dtrans, Es, vo, k, useBuck)

This command is used to construct a uniaxial Megalooikonomou-Monti-Santini concrete material object with degraded linear unloading/reloading stiffness according to the work of Karsan-Jirsa and no tensile strength.

<table>
<thead>
<tr>
<th>matTag (int)</th>
<th>integer tag identifying material</th>
</tr>
</thead>
<tbody>
<tr>
<td>fpc1 (float)</td>
<td>concrete core compressive strength.</td>
</tr>
<tr>
<td>fpc2 (float)</td>
<td>concrete cover compressive strength.</td>
</tr>
<tr>
<td>epsc0 (float)</td>
<td>strain corresponding to unconfined concrete strength.</td>
</tr>
<tr>
<td>D (float)</td>
<td>diameter of the circular section.</td>
</tr>
<tr>
<td>c (float)</td>
<td>dimension of concrete cover (until the outer edge of steel stirrups)</td>
</tr>
<tr>
<td>Ej (float)</td>
<td>elastic modulus of the fiber reinforced polymer (FRP) jacket.</td>
</tr>
<tr>
<td>Sj (float)</td>
<td>clear spacing of the FRP strips - zero if FRP jacket is continuous.</td>
</tr>
<tr>
<td>tj (float)</td>
<td>total thickness of the FRP jacket.</td>
</tr>
<tr>
<td>eju (float)</td>
<td>rupture strain of the FRP jacket from tensile coupons.</td>
</tr>
<tr>
<td>S (float)</td>
<td>spacing of the steel spiral/stirrups.</td>
</tr>
<tr>
<td>fyl (float)</td>
<td>yielding strength of longitudinal steel bars.</td>
</tr>
<tr>
<td>fyh (float)</td>
<td>yielding strength of the steel spiral/stirrups.</td>
</tr>
<tr>
<td>dlong (float)</td>
<td>diameter of the longitudinal bars of the circular section.</td>
</tr>
<tr>
<td>dtrans (float)</td>
<td>diameter of the steel spiral/stirrups.</td>
</tr>
<tr>
<td>Es (float)</td>
<td>elastic modulus of steel.</td>
</tr>
<tr>
<td>vo (float)</td>
<td>initial Poisson’s coefficient for concrete.</td>
</tr>
<tr>
<td>k (float)</td>
<td>reduction factor for the rupture strain of the FRP jacket, recommended values 0.5-0.8.</td>
</tr>
<tr>
<td>useBuck (float)</td>
<td>FRP jacket failure criterion due to buckling of longitudinal compressive steel bars (0 = not include it, 1= to include it).</td>
</tr>
</tbody>
</table>

**Note:** #IMPORTANT: The units of the input parameters should be in MPa, N, mm. #Concrete compressive strengths and the corresponding strain should be input as positive values. #When rupture of FRP jacket occurs due to dilation of concrete (lateral concrete strain exceeding reduced rupture strain of FRP jacket), the analysis is not terminated. Only a message “FRP Rupture” is plotted on the screen. #.When $useBuck input parameter is on (equal to 1) and the model’s longitudinal steel buckling conditions are fulfilled, a message “Initiation of Buckling of Long.Bar under Compression” is plotted on the screen. #.When rupture of FRP jacket occurs due to its interaction with buckled longitudinal compressive steel bars, the analysis is not terminated. Only a message “FRP Rupture due to Buckling of Long.Bar under compression” is plotted on the screen.

See also:

Notes
ConcreteCM

**uniaxialMaterial** (‘ConcreteCM’, matTag, fpcc, epcc, Ec, rc, xcrn, ft, et, rt, xcrp, ‘-GapClose’, gap=0)

This command is used to construct a uniaxialMaterial ConcreteCM (Kolozvari et al., 2015), which is a uniaxial hysteretic constitutive model for concrete developed by Chang and Mander (1994).

<table>
<thead>
<tr>
<th>matTag (int)</th>
<th>integer tag identifying material</th>
</tr>
</thead>
<tbody>
<tr>
<td>fpcc (float)</td>
<td>Compressive strength (f'_c)</td>
</tr>
<tr>
<td>epcc (float)</td>
<td>Strain at compressive strength (\epsilon'_c)</td>
</tr>
<tr>
<td>Ec (float)</td>
<td>Initial tangent modulus (E_c)</td>
</tr>
<tr>
<td>rc (float)</td>
<td>Shape parameter in Tsai’s equation defined for compression (r_c)</td>
</tr>
<tr>
<td>xcrn (float)</td>
<td>Non-dimensional critical strain on compression envelope (\epsilon_{cr}^-) where the envelope curve starts following a straight line</td>
</tr>
<tr>
<td>ft (float)</td>
<td>Tensile strength (f_t)</td>
</tr>
<tr>
<td>et (float)</td>
<td>Strain at tensile strength (\epsilon_t)</td>
</tr>
<tr>
<td>rt (float)</td>
<td>Shape parameter in Tsai’s equation defined for tension (r_t)</td>
</tr>
<tr>
<td>xcrp (float)</td>
<td>Non-dimensional critical strain on tension envelope (\epsilon_{cr}^+) where the envelope curve starts following a straight line – large value [e.g., 10000] recommended when tension stiffening is considered</td>
</tr>
<tr>
<td>gap (float)</td>
<td>gap = 0, less gradual gap closure (default); gap = 1, more gradual gap closure</td>
</tr>
</tbody>
</table>

See also:

Notes

**Standard Uniaxial Materials**

1. **Elastic Uniaxial Material**
2. **Elastic-Perfectly Plastic Material**
3. **Elastic-Perfectly Plastic Gap Material**
4. **Elastic-No Tension Material**
5. **Parallel Material**
6. **Series Material**

**Elastic Uniaxial Material**

**uniaxialMaterial** (‘Elastic’, matTag, E, eta=0.0, Eneg=E)  

This command is used to construct an elastic uniaxial material object.
matTag (int) | integer tag identifying material
---|---
E (float) | tangent
eta (float) | damping tangent (optional, default=0.0)
Energ (float) | tangent in compression (optional, default=E)

See also:

Notes

Elastic-Perfectly Plastic Material

uniaxialMaterial (‘ElasticPP’, matTag, E, epsyP, epsyN=epsyP, eps0=0.0)
This command is used to construct an elastic perfectly-plastic uniaxial material object.

See also:

Notes

Elastic-Perfectly Plastic Gap Material

uniaxialMaterial (‘ElasticPPGap’, matTag, E, Fy, gap, eta=0.0, damage=’noDamage’)
This command is used to construct an elastic perfectly-plastic gap uniaxial material object.

See also:

Notes
Elastic-No Tension Material

**uniaxialMaterial** (‘ENT’, matTag, E)

This command is used to construct a uniaxial elastic-no tension material object.

<table>
<thead>
<tr>
<th>matTag (int)</th>
<th>integer tag identifying material</th>
</tr>
</thead>
<tbody>
<tr>
<td>E (float)</td>
<td>tangent</td>
</tr>
</tbody>
</table>

See also:

Notes

Parallel Material

**uniaxialMaterial** (‘Parallel’, matTag, *tags, ‘.factor’, *facts)

This command is used to construct a parallel material object made up of an arbitrary number of previously-constructed UniaxialMaterial objects.

<table>
<thead>
<tr>
<th>matTag (int)</th>
<th>integer tag identifying material</th>
</tr>
</thead>
<tbody>
<tr>
<td>tags (list (int))</td>
<td>identification tags of materials making up the material model</td>
</tr>
<tr>
<td>facts (list (float))</td>
<td>factors to create a linear combination of the specified materials. Factors can be negative to subtract one material from another. (optional, default = 1.0)</td>
</tr>
</tbody>
</table>

See also:

Notes

Series Material

**uniaxialMaterial** (‘Series’, matTag, *tags)

This command is used to construct a series material object made up of an arbitrary number of previously-constructed UniaxialMaterial objects.

<table>
<thead>
<tr>
<th>matTag (int)</th>
<th>integer tag identifying material</th>
</tr>
</thead>
<tbody>
<tr>
<td>tags (list (int))</td>
<td>identification tags of materials making up the material model</td>
</tr>
</tbody>
</table>

See also:

Notes

**PyTzQz** uniaxial materials for p-y, t-z and q-z elements for modeling soil-structure interaction through the piles in a structural foundation

1. **PySimple1 Material**
2. **TzSimple1 Material**
3. **QzSimple1 Material**
4. **PyLiq1 Material**
5. **TzLiq1 Material**

**PySimple1 Material**

uniaxialMaterial (‘PySimple1’, matTag, soilType, pult, Y50, C_d, c=0.0)

This command is used to construct a PySimple1 uniaxial material object.

<table>
<thead>
<tr>
<th>Parameter</th>
<th>Description</th>
</tr>
</thead>
<tbody>
<tr>
<td>matTag</td>
<td>integer tag identifying material</td>
</tr>
<tr>
<td>soilType</td>
<td>soilType = 1 Backbone of p-y curve approximates Matlock (1970) soft clay relation. soilType = 2 Backbone of p-y curve approximates API (1993) sand relation.</td>
</tr>
<tr>
<td>pult</td>
<td>Ultimate capacity of the p-y material. Note that “p” or “pult” are distributed loads [force per length of pile] in common design equations, but are both loads for this uniaxialMaterial [i.e., distributed load times the tributary length of the pile].</td>
</tr>
<tr>
<td>Y50</td>
<td>Displacement at which 50% of pult is mobilized in monotonic loading.</td>
</tr>
<tr>
<td>C_d</td>
<td>Variable that sets the drag resistance within a fully-mobilized gap as C_d*pult.</td>
</tr>
<tr>
<td>c</td>
<td>The viscous damping term (dashpot) on the far-field (elastic) component of the displacement rate (velocity). (optional Default = 0.0). Nonzero c values are used to represent radiation damping effects</td>
</tr>
</tbody>
</table>

See also: **Notes**

**TzSimple1 Material**

uniaxialMaterial (‘TzSimple1’, matTag, tzType, tult, z50, c=0.0)

This command is used to construct a TzSimple1 uniaxial material object.

<table>
<thead>
<tr>
<th>Parameter</th>
<th>Description</th>
</tr>
</thead>
<tbody>
<tr>
<td>matTag</td>
<td>integer tag identifying material</td>
</tr>
<tr>
<td>soilType</td>
<td>soilType = 1 Backbone of t-z curve approximates Reese and O’Neill (1987). soilType = 2 Backbone of t-z curve approximates Mosher (1984) relation.</td>
</tr>
<tr>
<td>tult</td>
<td>Ultimate capacity of the t-z material. SEE NOTE 1.</td>
</tr>
<tr>
<td>z50</td>
<td>Displacement at which 50% of tult is mobilized in monotonic loading.</td>
</tr>
<tr>
<td>c</td>
<td>The viscous damping term (dashpot) on the far-field (elastic) component of the displacement rate (velocity). (optional Default = 0.0). See NOTE 2.</td>
</tr>
</tbody>
</table>

Note:

1. The argument tult is the ultimate capacity of the t-z material. Note that “t” or “tult” are shear stresses [force per unit area of pile surface] in common design equations, but are both loads for this uniaxialMaterial [i.e., shear stress times the tributary area of the pile].

2. Nonzero c values are used to represent radiation damping effects

See also:
Notes

QzSimple1 Material

`uniaxialMaterial` (`'QzSimple1'`, `matTag`, `qzType`, `qult`, `Z50`, `suction=0.0`, `c=0.0`)

This command is used to construct a QzSimple1 uniaxial material object.

<table>
<thead>
<tr>
<th>matTag (int)</th>
<th>integer tag identifying material</th>
</tr>
</thead>
<tbody>
<tr>
<td>qzType (int)</td>
<td>qzType = 1 Backbone of q-z curve approximates Reese and O’Neill’s (1987) relation for drilled shafts in clay. qzType = 2 Backbone of q-z curve approximates Vijayvergiya’s (1977) relation for piles in sand.</td>
</tr>
<tr>
<td>qult (float)</td>
<td>Ultimate capacity of the q-z material. SEE NOTE 1.</td>
</tr>
<tr>
<td>Z50 (float)</td>
<td>Displacement at which 50% of qult is mobilized in monotonic loading. SEE NOTE 2.</td>
</tr>
<tr>
<td>suction (float)</td>
<td>Uplift resistance is equal to suction<em>qult. Default = 0.0. The value of suction must be 0.0 to 0.1.</em></td>
</tr>
<tr>
<td>c (float)</td>
<td>The viscous damping term (dashpot) on the far-field (elastic) component of the displacement rate (velocity). Default = 0.0. Nonzero c values are used to represent radiation damping effects.*</td>
</tr>
</tbody>
</table>

Note:

1. `qult`: Ultimate capacity of the q-z material. Note that qi or qult are stresses [force per unit area of pile tip] in common design equations, but are both loads for this uniaxialMaterial [i.e., stress times tip area].

2. Y50: Displacement at which 50% of pult is mobilized in monotonic loading. Note that Vijayvergiya’s relation (qzType=2) refers to a “critical” displacement (zcrit) at which qult is fully mobilized, and that the corresponding z50 would be 0.125zcrit.

3. optional args `suction` and `c` must either both be omitted or both provided.

See also:

Notes

PyLiq1 Material

`uniaxialMaterial` (`'PyLiq1'`, `matTag`, `soilType`, `pult`, `Y50`, `Cd`, `pRes`, `ele1`, `ele2`)

This command constructs a uniaxial p-y material that incorporates liquefaction effects. This p-y material is used with a zeroLength element to connect a pile (beam-column element) to a 2 D plane-strain FE mesh or displacement boundary condition. The p-y material obtains the average mean effective stress (which decreases with increasing excess pore pressure) either from two specified soil elements, or from a time series. Currently, the implementation requires that the specified soil elements consist of FluidSolidPorousMaterials in FourNode-Quad elements, or PressureDependMultiYield or PressureDependMultiYield02 materials in FourNodeQuadUP or NineFourQuadUP elements. There are two possible forms:
matTag (int)  integer tag identifying material
soilType (int)  soilType = 1 Backbone of p-y curve approximates Matlock (1970) soft clay relation. soilType = 2 Backbone of p-y curve approximates API (1993) sand relation.
pult (float)  Ultimate capacity of the p-y material. Note that “p” or “pult” are distributed loads [force per length of pile] in common design equations, but are both loads for this uniaxialMaterial [i.e., distributed load times the tributary length of the pile].
Y50 (float)  Displacement at which 50% of pult is mobilized in monotonic loading.
Cd (float)  Variable that sets the drag resistance within a fully-mobilized gap as Cd*pult.
c (float)  The viscous damping term (dashpot) on the far-field (elastic) component of the displacement rate (velocity). (optional Default = 0.0). Nonzero c values are used to represent radiation damping effects.
pRes (float)  sets the minimum (or residual) peak resistance that the material retains as the adjacent solid soil elements liquefy.
ele1 ele2 (float)  are the eleTag (element numbers) for the two solid elements from which PyLiq1 will obtain mean effective stresses and excess pore pressures.
seriesTag (float)  Alternatively, mean effective stress can be supplied by a time series by specifying the text string '-'-timeSeries' and the tag of the series seriesTag.

See also:
Notes

TzLiq1 Material

uniaxialMaterial ("TzLiq1", matTag, tzType, tult, z50, c, ele1, ele2)
uniaxialMaterial ("TzLiq1", matTag, tzType, tult, z50, c, '-timeSeries', seriesTag)

The command constructs a uniaxial t-z material that incorporates liquefaction effects. This t-z material is used with a zeroLength element to connect a pile (beam-column element) to a 2 D plane-strain FE mesh. The t-z material obtains the average mean effective stress (which decreases with increasing excess pore pressure) from two specified soil elements. Currently, the implementation requires that the specified soil elements consist of FluidSolidPorousMaterials in FourNodeQuad elements.
Note:

1. The argument $t_{ult}$ is the ultimate capacity of the t-z material. Note that “t” or “tult” are shear stresses [force per unit area of pile surface] in common design equations, but are both loads for this uniaxialMaterial [i.e., shear stress times the tributary area of the pile].

2. Nonzero $c$ values are used to represent radiation damping effects

3. To model the effects of liquefaction with $T_{z_{Liq}}$, it is necessary to use the material stage updating command:

See also:

Notes

Other Uniaxial Materials

1. Hardening Material
2. CastFuse Material
3. ViscousDamper Material
4. BilinearOilDamper Material
5. Modified Ibarra-Medina-Krawinkler Deterioration Model with Bilinear Hysteretic Response (Bilin Material)
6. Modified Ibarra-Medina-Krawinkler Deterioration Model with Peak-Oriented Hysteretic Response (ModIMK-PeakOriented Material)
7. Modified Ibarra-Medina-Krawinkler Deterioration Model with Pinched Hysteretic Response (ModIMKPinching Material)
8. SAWS Material
9. BarSlip Material
10. Bond SP01 - - Strain Penetration Model for Fully Anchored Steel Reinforcing Bars
11. Fatigue Material
12. Impact Material
13. Hyperbolic Gap Material
14. Limit State Material
15. MinMax Material
16. ElasticBilin Material
17. ElasticMultiLinear Material
18. MultiLinear
19. Initial Strain Material
20. Initial Stress Material
21. PathIndependent Material
22. Pinching4 Material
23. Engineered Cementitious Composites Material
24. SelfCentering Material
Viscous Material

BoucWen Material

BWBN Material

KikuchiAikenHDR Material

KikuchiAikenLRB Material

AxialSp Material

AxialSpHD Material

Pinching Limit State Material

CFSWSWP Wood-Sheathed Cold-Formed Steel Shear Wall Panel

CFSSWP Steel-Sheathed Cold-formed Steel Shear Wall Panel

Hardening Material

```
uniaxialMaterial('Hardening', matTag, E, sigmaY, H_iso, H_kin, eta=0.0)
```

This command is used to construct a uniaxial material object with combined linear kinematic and isotropic hardening. The model includes optional visco-plasticity using a Perzyna formulation.

<table>
<thead>
<tr>
<th>matTag (int)</th>
<th>integer tag identifying material</th>
</tr>
</thead>
<tbody>
<tr>
<td>E (float)</td>
<td>tangent stiffness</td>
</tr>
<tr>
<td>sigmaY (float)</td>
<td>yield stress or force</td>
</tr>
<tr>
<td>H_iso (float)</td>
<td>isotropic hardening Modulus</td>
</tr>
<tr>
<td>H_kin (float)</td>
<td>kinematic hardening Modulus</td>
</tr>
<tr>
<td>eta (float)</td>
<td>visco-plastic coefficient (optional, default=0.0)</td>
</tr>
</tbody>
</table>

See also:

Notes

CastFuse Material

```
uniaxialMaterial('Cast', matTag, n, bo, h, fy, E, L, b, Ro, cR1, cR2, a1=s2*Pp/Kp, a2=1.0, a3=ad*Pp/Kp, a4=1.0)
```

This command is used to construct a parallel material object made up of an arbitrary number of previously-constructed UniaxialMaterial objects.
<table>
<thead>
<tr>
<th>matTag (int)</th>
<th>integer tag identifying material</th>
</tr>
</thead>
<tbody>
<tr>
<td>n (int)</td>
<td>Number of yield fingers of the CSF-brace</td>
</tr>
<tr>
<td>bo (float)</td>
<td>Width of an individual yielding finger at its base of the CSF-brace</td>
</tr>
<tr>
<td>h (float)</td>
<td>Thickness of an individual yielding finger</td>
</tr>
<tr>
<td>fy (float)</td>
<td>Yield strength of the steel material of the yielding finger</td>
</tr>
<tr>
<td>E (float)</td>
<td>Modulus of elasticity of the steel material of the yielding finger</td>
</tr>
<tr>
<td>L (float)</td>
<td>Height of an individual yielding finger</td>
</tr>
<tr>
<td>b (float)</td>
<td>Strain hardening ratio</td>
</tr>
<tr>
<td>Ro (float)</td>
<td>Parameter that controls the Bauschinger effect. Recommended Values for $Ro$ between 10 to 30</td>
</tr>
<tr>
<td>cR1 (float)</td>
<td>Parameter that controls the Bauschinger effect. Recommended Value $cR1=0.925$</td>
</tr>
<tr>
<td>cR2 (float)</td>
<td>Parameter that controls the Bauschinger effect. Recommended Value $cR2=0.150$</td>
</tr>
<tr>
<td>a1 (float)</td>
<td>isotropic hardening parameter, increase of compression yield envelope as proportion of yield strength after a plastic deformation of $a2*(Pp/Kp)$</td>
</tr>
<tr>
<td>a2 (float)</td>
<td>isotropic hardening parameter (see explanation under a1). (optional default = 1.0)</td>
</tr>
<tr>
<td>a3 (float)</td>
<td>isotropic hardening parameter, increase of tension yield envelope as proportion of yield strength after a plastic deformation of $a4*(Pp/Kp)$</td>
</tr>
<tr>
<td>a4 (float)</td>
<td>isotropic hardening parameter (see explanation under a3). (optional default = 1.0)</td>
</tr>
</tbody>
</table>

Gray et al. [1] showed that the monotonic backbone curve of a CSF-brace with known properties $(n, bo, h, L, fy, E)$ after yielding can be expressed as a close-form solution that is given by, $P = P_p / \cos(2d/L)$, in which $d$ is the axial deformation of the brace at increment $i$ and $P_p$ is the yield strength of the CSF-brace and is given by the following expression

$$P_p = nb_o h^2 f_y / 4L$$

The elastic stiffness of the CSF-brace is given by,

$$K_p = nb_o E h^3 f_y / 6L^3$$

See also:

Notes

**ViscousDamper Material**

**uniaxialMaterial** ('ViscousDamper', matTag, K, Cd, alpha, LGap=0.0, NM=1, RelTol=1e-6, AbsTol=1e-10, MaxHalf=15)

This command is used to construct a ViscousDamper material, which represents the Maxwell Model (linear spring and nonlinear dashpot in series). The ViscousDamper material simulates the hysteretic response of nonlinear viscous dampers. An adaptive iterative algorithm has been implemented and validated to solve numerically the constitutive equations within a nonlinear viscous damper with a high-precision accuracy.
<table>
<thead>
<tr>
<th>Parameter</th>
<th>Description</th>
</tr>
</thead>
<tbody>
<tr>
<td>matTag (int)</td>
<td>integer tag identifying material</td>
</tr>
<tr>
<td>K (float)</td>
<td>Elastic stiffness of linear spring to model the axial flexibility of a viscous damper (e.g. combined stiffness of the supporting brace and internal damper portion)</td>
</tr>
<tr>
<td>Cd (float)</td>
<td>Damping coefficient</td>
</tr>
<tr>
<td>alpha (float)</td>
<td>Velocity exponent</td>
</tr>
<tr>
<td>LGap (float)</td>
<td>Gap length to simulate the gap length due to the pin tolerance</td>
</tr>
<tr>
<td>NM (int)</td>
<td>Employed adaptive numerical algorithm (default value NM = 1; 1 = Dormand-Prince54, 2 = 6th order Adams-Bashforth-Moulton, 3 = modified Rosenbrock Triple)</td>
</tr>
<tr>
<td>RelTol (float)</td>
<td>Tolerance for absolute relative error control of the adaptive iterative algorithm (default value 10^-6)</td>
</tr>
<tr>
<td>AbsTol (float)</td>
<td>Tolerance for absolute error control of adaptive iterative algorithm (default value 10^-10)</td>
</tr>
<tr>
<td>MaxHalf (int)</td>
<td>Maximum number of sub-step iterations within an integration step (default value 15)</td>
</tr>
</tbody>
</table>

See also:

Notes

**BilinearOilDamper Material**

**uniaxialMaterial** ('BilinearOilDamper', matTag, K, Cd, Fr=1.0, p=1.0, LGap=0.0, NM=1, RelTol=1e-6, AbsTol=1e-10, MaxHalf=15)

This command is used to construct a BilinearOilDamper material, which simulates the hysteretic response of bilinear oil dampers with relief valve. Two adaptive iterative algorithms have been implemented and validated to solve numerically the constitutive equations within a bilinear oil damper with a high-precision accuracy.
<table>
<thead>
<tr>
<th>matTag (int)</th>
<th>integer tag identifying material</th>
</tr>
</thead>
<tbody>
<tr>
<td>K (float)</td>
<td>Elastic stiffness of linear spring to model the axial flexibility of a viscous damper (e.g. combined stiffness of the supporting brace and internal damper portion)</td>
</tr>
<tr>
<td>Cd (float)</td>
<td>Damping coefficient</td>
</tr>
<tr>
<td>Fr (float)</td>
<td>Damper relief load (default=1.0, Damper property)</td>
</tr>
<tr>
<td>p (float)</td>
<td>Post-relief viscous damping coefficient ratio (default=1.0, linear oil damper)</td>
</tr>
<tr>
<td>LGap (float)</td>
<td>Gap length to simulate the gap length due to the pin tolerance</td>
</tr>
<tr>
<td>NM (int)</td>
<td>Employed adaptive numerical algorithm (default value NM = 1; • 1 = Dormand-Prince54, • 2 = 6th order Adams-Bashforth-Moulton, • 3 = modified Rosenbrock Triple)</td>
</tr>
<tr>
<td>RelTol (float)</td>
<td>Tolerance for absolute relative error control of the adaptive iterative algorithm (default value 10^-6)</td>
</tr>
<tr>
<td>AbsTol (float)</td>
<td>Tolerance for absolute error control of adaptive iterative algorithm (default value 10^-10)</td>
</tr>
<tr>
<td>MaxHalf (int)</td>
<td>Maximum number of sub-step iterations within an integration step (default value 15)</td>
</tr>
</tbody>
</table>

See also:

Notes

**Modified Ibarra-Medina-Krawinkler Deterioration Model with Bilinear Hysteretic Response (Bilin Material)**

```python
uniaxialMaterial('Bilin', matTag, K0, as_Plus, as_Neg, My_Plus, My_Neg, Lamda_S, Lamda_C, Lamda_A, Lamda_K, c_S, c_C, c_A, c_K, theta_p_Plus, theta_p_Neg, theta_pc_Plus, theta_pc_Neg, Res_Pos, Res_Neg, theta_u_Plus, theta_u_Neg, D_Plus, D_Neg, nFactor=0.0)
```

This command is used to construct a bilin material. The bilin material simulates the modified Ibarra-Krawinkler deterioration model with bilinear hysteretic response. Note that the hysteretic response of this material has been calibrated with respect to more than 350 experimental data of steel beam-to-column connections and multivariate regression formulas are provided to estimate the deterioration parameters of the model for different connection types. These relationships were developed by Lignos and Krawinkler (2009, 2011) and have been adopted by PEER/ATC (2010). The input parameters for this component model can be computed interactively from this link. Use the module Component Model.
<table>
<thead>
<tr>
<th>matTag</th>
<th>integer tag identifying material</th>
</tr>
</thead>
<tbody>
<tr>
<td>K0</td>
<td>float</td>
</tr>
<tr>
<td>as_Plus</td>
<td>float</td>
</tr>
<tr>
<td>as_Neg</td>
<td>float</td>
</tr>
<tr>
<td>My_Plus</td>
<td>float</td>
</tr>
<tr>
<td>My_Neg</td>
<td>float</td>
</tr>
<tr>
<td>Lamda_S</td>
<td>float</td>
</tr>
<tr>
<td>Lamda_C</td>
<td>float</td>
</tr>
<tr>
<td>Lamda_A</td>
<td>float</td>
</tr>
<tr>
<td>Lamda_K</td>
<td>float</td>
</tr>
<tr>
<td>c_S</td>
<td>float</td>
</tr>
<tr>
<td>c_C</td>
<td>float</td>
</tr>
<tr>
<td>c_A</td>
<td>float</td>
</tr>
<tr>
<td>c_K</td>
<td>float</td>
</tr>
<tr>
<td>theta_p_Plus</td>
<td>float</td>
</tr>
<tr>
<td>theta_p_Neg</td>
<td>float</td>
</tr>
<tr>
<td>theta_pc_Plus</td>
<td>float</td>
</tr>
<tr>
<td>theta_pc_Neg</td>
<td>float</td>
</tr>
<tr>
<td>Res_Pos</td>
<td>float</td>
</tr>
<tr>
<td>Res_Neg</td>
<td>float</td>
</tr>
<tr>
<td>theta_u_Plus</td>
<td>float</td>
</tr>
<tr>
<td>theta_u_Neg</td>
<td>float</td>
</tr>
<tr>
<td>D_Plus</td>
<td>float</td>
</tr>
<tr>
<td>D_Neg</td>
<td>float</td>
</tr>
<tr>
<td>nFactor</td>
<td>float</td>
</tr>
</tbody>
</table>

See also:
Notes
Modified Ibarra-Medina-Krawinkler Deterioration Model with Peak-Oriented Hysteretic Response (ModIMKPeakOriented Material)

\textbf{uniaxialMaterial} ( 'ModIMKPeakOriented', matTag, K0, as_Plus, as_Neg, My_Plus, My_Neg, Lamda_S, Lamda_C, Lamda_A, Lamda_K, c_S, c_C, c_A, c_K, theta_p_Plus, theta_p_Neg, theta_pc_Plus, theta_pc_Neg, Res_Pos, Res_Neg, theta_u_Plus, theta_u_Neg, D_Plus, D_Neg)

This command is used to construct a ModIMKPeakOriented material. This material simulates the modified Ibarra-Medina-Krawinkler deterioration model with peak-oriented hysteretic response. Note that the hysteretic response of this material has been calibrated with respect to 200 experimental data of RC beams in order to estimate the deterioration parameters of the model. This information was developed by Lignos and Krawinkler (2012). NOTE: before you use this material make sure that you have downloaded the latest OpenSees version. A youtube video presents a summary of this model including the way to be used within openSees youtube link.
<table>
<thead>
<tr>
<th><strong>matTag</strong> (int)</th>
<th>integer tag identifying material</th>
</tr>
</thead>
<tbody>
<tr>
<td><strong>K0</strong> (float)</td>
<td>elastic stiffness</td>
</tr>
<tr>
<td><strong>as_Plus</strong> (float)</td>
<td>strain hardening ratio for positive loading direction</td>
</tr>
<tr>
<td><strong>as_Neg</strong> (float)</td>
<td>strain hardening ratio for negative loading direction</td>
</tr>
<tr>
<td><strong>My_Plus</strong> (float)</td>
<td>effective yield strength for positive loading direction</td>
</tr>
<tr>
<td><strong>My_Neg</strong> (float)</td>
<td>effective yield strength for negative loading direction (negative value)</td>
</tr>
<tr>
<td><strong>Lamda_S</strong> (float)</td>
<td>Cyclic deterioration parameter for strength deterioration ( E_t = \text{Lamda}_S * M_y ), see Lignos and Krawinkler (2011); set Lamda_S = 0 to disable this mode of deterioration</td>
</tr>
<tr>
<td><strong>Lamda_C</strong> (float)</td>
<td>Cyclic deterioration parameter for post-capping strength deterioration ( E_t = \text{Lamda}_C * M_y ), see Lignos and Krawinkler (2011); set Lamda_C = 0 to disable this mode of deterioration</td>
</tr>
<tr>
<td><strong>Lamda_A</strong> (float)</td>
<td>Cyclic deterioration parameter for accelerated reloading stiffness deterioration ( E_t = \text{Lamda}_A * M_y ), see Lignos and Krawinkler (2011); set Lamda_A = 0 to disable this mode of deterioration</td>
</tr>
<tr>
<td><strong>Lamda_K</strong> (float)</td>
<td>Cyclic deterioration parameter for unloading stiffness deterioration ( E_t = \text{Lamda}_K * M_y ), see Lignos and Krawinkler (2011); set Lamda_K = 0 to disable this mode of deterioration</td>
</tr>
<tr>
<td><strong>c_S</strong> (float)</td>
<td>rate of strength deterioration. The default value is 1.0.</td>
</tr>
<tr>
<td><strong>c_C</strong> (float)</td>
<td>rate of post-capping strength deterioration. The default value is 1.0.</td>
</tr>
<tr>
<td><strong>c_A</strong> (float)</td>
<td>rate of accelerated reloading deterioration. The default value is 1.0.</td>
</tr>
<tr>
<td><strong>c_K</strong> (float)</td>
<td>rate of unloading stiffness deterioration. The default value is 1.0.</td>
</tr>
<tr>
<td><strong>theta_p_Plus</strong> (float)</td>
<td>pre-capping rotation for positive loading direction (often noted as plastic rotation capacity)</td>
</tr>
<tr>
<td><strong>theta_p_Neg</strong> (float)</td>
<td>pre-capping rotation for negative loading direction (often noted as plastic rotation capacity) (must be defined as a positive value)</td>
</tr>
<tr>
<td><strong>theta_pc_Plus</strong> (float)</td>
<td>post-capping rotation for positive loading direction</td>
</tr>
<tr>
<td><strong>theta_pc_Neg</strong> (float)</td>
<td>post-capping rotation for negative loading direction (must be defined as a positive value)</td>
</tr>
<tr>
<td><strong>Res_Pos</strong> (float)</td>
<td>residual strength ratio for positive loading direction</td>
</tr>
<tr>
<td><strong>Res_Neg</strong> (float)</td>
<td>residual strength ratio for negative loading direction (must be defined as a positive value)</td>
</tr>
<tr>
<td><strong>theta_u_Plus</strong> (float)</td>
<td>ultimate rotation capacity for positive loading direction</td>
</tr>
<tr>
<td><strong>theta_u_Neg</strong> (float)</td>
<td>ultimate rotation capacity for negative loading direction (must be defined as a positive value)</td>
</tr>
<tr>
<td><strong>D_Plus</strong> (float)</td>
<td>rate of cyclic deterioration in the positive loading direction (this parameter is used to create asymmetric hysteretic behavior for the case of a composite beam). For symmetric hysteretic response use 1.0.</td>
</tr>
<tr>
<td><strong>D_Neg</strong> (float)</td>
<td>rate of cyclic deterioration in the negative loading direction (this parameter is used to create asymmetric hysteretic behavior for the case of a composite beam). For symmetric hysteretic response use 1.0.</td>
</tr>
</tbody>
</table>

See also:

Notes

1.4. Model Commands 131
Modified Ibarra-Medina-Krawinkler Deterioration Model with Pinched Hysteretic Response (ModIMKPinching Material)

`uniaxialMaterial('ModIMKPinching', matTag, K0, as_Plus, as_Neg, My_Plus, My_Neg, FprPos, FprNeg, A_pinch, Lamda_S, Lamda_C, Lamda_A, Lamda_K, c_S, c_C, c_A, c_K, theta_p_Plus, theta_p_Neg, theta_pc_Plus, theta_pc_Neg, Res_Pos, Res_Neg, theta_u_Plus, theta_u_Neg, D_Plus, D_Neg)`

This command is used to construct a ModIMKPinching material. This material simulates the modified Ibarra-Medina-Krawinkler deterioration model with pinching hysteretic response. NOTE: before you use this material make sure that you have downloaded the latest OpenSees version. A youtube video presents a summary of this model including the way to be used within openSees youtube link.
<table>
<thead>
<tr>
<th>matTag</th>
<th>integer tag identifying material</th>
</tr>
</thead>
<tbody>
<tr>
<td>K0 (float)</td>
<td>elastic stiffness</td>
</tr>
<tr>
<td>as_Plus  (float)</td>
<td>strain hardening ratio for positive loading direction</td>
</tr>
<tr>
<td>as_Neg  (float)</td>
<td>strain hardening ratio for negative loading direction</td>
</tr>
<tr>
<td>My_Plus (float)</td>
<td>effective yield strength for positive loading direction</td>
</tr>
<tr>
<td>My_Neg  (float)</td>
<td>effective yield strength for negative loading direction (Must be defined as a negative value)</td>
</tr>
<tr>
<td>FprPos  (float)</td>
<td>Ratio of the force at which reloading begins to force corresponding to the maximum historic deformation demand (positive loading direction)</td>
</tr>
<tr>
<td>FprNeg  (float)</td>
<td>Ratio of the force at which reloading begins to force corresponding to the absolute maximum historic deformation demand (negative loading direction)</td>
</tr>
<tr>
<td>A_Pinch (float)</td>
<td>Ratio of reloading stiffness</td>
</tr>
<tr>
<td>Lamda_S (float)</td>
<td>Cyclic deterioration parameter for strength deterioration ([E_t=Lamda_S*M_y, \text{ see } \text{Lignos and Krawinkler (2011); set } Lamda_S=0 \text{ to disable this mode of deterioration}])</td>
</tr>
<tr>
<td>Lamda_C (float)</td>
<td>Cyclic deterioration parameter for post-capping strength deterioration ([E_t=Lamda_C*M_y, \text{ see } \text{Lignos and Krawinkler (2011); set } Lamda_C=0 \text{ to disable this mode of deterioration}])</td>
</tr>
<tr>
<td>Lamda_A (float)</td>
<td>Cyclic deterioration parameter for accelerated reloading stiffness deterioration ([E_t=Lamda_A*M_y, \text{ see } \text{Lignos and Krawinkler (2011); set } Lamda_A=0 \text{ to disable this mode of deterioration}])</td>
</tr>
<tr>
<td>Lamda_K (float)</td>
<td>Cyclic deterioration parameter for unloading stiffness deterioration ([E_t=Lamda_K*M_y, \text{ see } \text{Lignos and Krawinkler (2011); set } Lamda_K=0 \text{ to disable this mode of deterioration}])</td>
</tr>
<tr>
<td>c_S (float)</td>
<td>rate of strength deterioration. The default value is 1.0.</td>
</tr>
<tr>
<td>c_C (float)</td>
<td>rate of post-capping strength deterioration. The default value is 1.0.</td>
</tr>
<tr>
<td>c_A (float)</td>
<td>rate of accelerated reloading deterioration. The default value is 1.0.</td>
</tr>
<tr>
<td>c_K (float)</td>
<td>rate of unloading stiffness deterioration. The default value is 1.0.</td>
</tr>
<tr>
<td>theta_p_P (float)</td>
<td>pre-capping rotation for positive loading direction (often noted as plastic rotation capacity)</td>
</tr>
<tr>
<td>theta_p_N (float)</td>
<td>pre-capping rotation for negative loading direction (often noted as plastic rotation capacity) (must be defined as a positive value)</td>
</tr>
<tr>
<td>theta_pc_P (float)</td>
<td>post-capping rotation for positive loading direction</td>
</tr>
<tr>
<td>theta_pc_N (float)</td>
<td>post-capping rotation for negative loading direction (must be defined as a positive value)</td>
</tr>
<tr>
<td>Res_Pos (float)</td>
<td>residual strength ratio for positive loading direction</td>
</tr>
<tr>
<td>Res_Neg (float)</td>
<td>residual strength ratio for negative loading direction (must be defined as a positive value)</td>
</tr>
<tr>
<td>theta_u_P (float)</td>
<td>ultimate rotation capacity for positive loading direction</td>
</tr>
<tr>
<td>theta_u_N (float)</td>
<td>ultimate rotation capacity for negative loading direction (must be defined as a positive value)</td>
</tr>
<tr>
<td>D_Plus (float)</td>
<td>rate of cyclic deterioration in the positive loading direction (this parameter is used to create asymmetric hysteretic behavior for the case of a composite beam). For symmetric hysteretic response use 1.0.</td>
</tr>
<tr>
<td>D_Neg (float)</td>
<td>rate of cyclic deterioration in the negative loading direction (this parameter is used to create asymmetric hysteretic behavior for the case of a composite beam). For symmetric hysteretic response use 1.0.</td>
</tr>
</tbody>
</table>
See also:

Notes

SAWS Material

uniaxialMaterial ('SAWS', matTag, F0, F1, DU, S0, R1, R2, R3, R4, alph, beta)
This file contains the class definition for SAWSMaterial. SAWSMaterial provides the implementation of a
one-dimensional hysteretic model developed as part of the CUREe Caltech wood frame project.

| matTag (int) | integer tag identifying material |
| F0 (float) | Intercept strength of the shear wall spring element for the asymptotic line to the envelope curve F0 > F1 > 0 |
| F1 (float) | Intercept strength of the spring element for the pinching branch of the hysteretic curve. (F1 > 0). |
| DU (float) | Spring element displacement at ultimate load. (DU > 0). |
| S0 (float) | Initial stiffness of the shear wall spring element (S0 > 0). |
| R1 (float) | Stiffness ratio of the asymptotic line to the spring element envelope curve. The slope of this line is R1 S0. (0 < R1 < 1.0). |
| R2 (float) | Stiffness ratio of the descending branch of the spring element envelope curve. The slope of this line is R2 S0. (R2 < 0). |
| R3 (float) | Stiffness ratio of the unloading branch of the spring element envelope curve. The slope of this line is R3 S0. (R3 > 1). |
| R4 (float) | Stiffness ratio of the pinching branch for the spring element. The slope of this line is R4 S0. (R4 > 0). |
| alph (float) | Stiffness degradation parameter for the shear wall spring element. (ALPHA > 0). |
| beta (float) | Stiffness degradation parameter for the spring element. (BETA > 0). |

See also:

Notes

BarSlip Material

uniaxialMaterial ('BarSlip', matTag, fc, fy, Es, fu, Eh, db, ld, nb, depth, height, anclratio=1.0, bsFlag, type, damage='Damage', unit='psi')
This command is used to construct a uniaxial material that simulates the bar force versus slip response of a
reinforcing bar anchored in a beam-column joint. The model exhibits degradation under cyclic loading. Cyclic
degradation of strength and stiffness occurs in three ways: unloading stiffness degradation, reloading stiffness
degradation, strength degradation.
**matTag** *(int)* integer tag identifying material

**fc** *(float)* positive floating point value defining the compressive strength of the concrete in which the reinforcing bar is anchored

**fy** *(float)* positive floating point value defining the yield strength of the reinforcing steel

**Es** *(float)* floating point value defining the modulus of elasticity of the reinforcing steel

**fu** *(float)* positive floating point value defining the ultimate strength of the reinforcing steel

**Eh** *(float)* floating point value defining the hardening modulus of the reinforcing steel

**ld** *(float)* floating point value defining the development length of the reinforcing steel

**db** *(float)* point value defining the diameter of reinforcing steel

**nb** *(float)* an integer defining the number of anchored bars

**depth** *(float)* floating point value defining the dimension of the member (beam or column) perpendicular to the dimension of the plane of the paper

**height** *(float)* floating point value defining the height of the flexural member, perpendicular to direction in which the reinforcing steel is placed, but in the plane of the paper

**ancLratio** *(float)* floating point value defining the ratio of anchorage length used for the reinforcing bar to the dimension of the joint in the direction of the reinforcing bar (optional, default: 1.0)

**bsFlag** *(str)* string indicating relative bond strength for the anchored reinforcing bar (options: 'Strong' or 'Weak')

**type** *(str)* string indicating where the reinforcing bar is placed. (options: 'beamtop', 'beambot' or 'column')

**damage** *(str)* string indicating type of damage: whether there is full damage in the material or no damage (optional, options: 'Damage', 'NoDamage'; default: 'Damage')

**unit** *(str)* string indicating the type of unit system used (optional, options: 'psi', 'MPa', 'Pa', 'psf', 'ksi', 'ksf') (default: 'psi'/'MPa')

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**See also:**

**Notes**

**Bond SP01 - Strain Penetration Model for Fully Anchored Steel Reinforcing Bars**

**uniaxialMaterial** *(Bond_SP01, matTag, Fy, Sy, Fu, Su, b, R)*

This command is used to construct a uniaxial material object for capturing strain penetration effects at the column-to-footing, column-to-bridge bent caps, and wall-to-footing intersections. In these cases, the bond slip associated with strain penetration typically occurs along a portion of the anchorage length. This model can also be applied to the beam end regions, where the strain penetration may include slippage of the bar along the entire anchorage length, but the model parameters should be chosen appropriately.

This model is for fully anchored steel reinforcement bars that experience bond slip along a portion of the anchorage length due to strain penetration effects, which are usually the case for column and wall longitudinal bars anchored into footings or bridge joints

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**See also:**

1.4. Model Commands 135
Notes

Fatigue Material

uniaxialMaterial('Fatigue', matTag, tag, '-E0', $E_0=0.191$, '-m', $m=-0.458$, '-min', min=-1e16, '-max', max=1e16)

The fatigue material uses a modified rainflow cycle counting algorithm to accumulate damage in a material using Miner’s Rule. Element stress/strain relationships become zero when fatigue life is exhausted.

<table>
<thead>
<tr>
<th>matTag (int)</th>
<th>integer tag identifying material</th>
</tr>
</thead>
<tbody>
<tr>
<td>tag (float)</td>
<td>Unique material object integer tag for the material that is being wrapped</td>
</tr>
<tr>
<td>E0 (float)</td>
<td>Value of strain at which one cycle will cause failure (default 0.191)</td>
</tr>
<tr>
<td>m (float)</td>
<td>Slope of Coffin-Manson curve in log-log space (default -0.458)</td>
</tr>
<tr>
<td>min (float)</td>
<td>Global minimum value for strain or deformation (default -1e16)</td>
</tr>
<tr>
<td>max (float)</td>
<td>Global maximum value for strain or deformation (default 1e16)</td>
</tr>
</tbody>
</table>

See also:

Impact Material

uniaxialMaterial('ImpactMaterial', matTag, $K_1$, $K_2$, sigy, gap)

This command is used to construct an impact material object

<table>
<thead>
<tr>
<th>matTag (int)</th>
<th>integer tag identifying material</th>
</tr>
</thead>
<tbody>
<tr>
<td>$K_1$ (float)</td>
<td>initial stiffness</td>
</tr>
<tr>
<td>$K_2$ (float)</td>
<td>secondary stiffness</td>
</tr>
<tr>
<td>sigy (float)</td>
<td>yield displacement</td>
</tr>
<tr>
<td>gap (float)</td>
<td>initial gap</td>
</tr>
</tbody>
</table>

See also:

Notes

Hyperbolic Gap Material

uniaxialMaterial('HyperbolicGapMaterial', matTag, $K_{max}$, $K_{ur}$, $R_f$, $F_{ult}$, gap)

This command is used to construct a hyperbolic gap material object.

<table>
<thead>
<tr>
<th>matTag (int)</th>
<th>integer tag identifying material</th>
</tr>
</thead>
<tbody>
<tr>
<td>$K_{max}$ (float)</td>
<td>initial stiffness</td>
</tr>
<tr>
<td>$K_{ur}$ (float)</td>
<td>unloading/reloading stiffness</td>
</tr>
<tr>
<td>$R_f$ (float)</td>
<td>failure ratio</td>
</tr>
<tr>
<td>$F_{ult}$ (float)</td>
<td>ultimate (maximum) passive resistance</td>
</tr>
<tr>
<td>gap (float)</td>
<td>initial gap</td>
</tr>
</tbody>
</table>

Note:
1. This material is implemented as a compression-only gap material. $F_{ult}$ and $gap$ should be input as negative values.

2. Recommended Values:
   - $K_{max} = 20300 \text{ kN/m of abutment width}$
   - $K_{cur} = K_{max}$
   - $R_f = 0.7$
   - $F_{ult} = -326 \text{ kN per meter of abutment width}$
   - $gap = -2.54 \text{ cm}$

See also:

Notes

Limit State Material

uniaxialMaterial ("LimitState", matTag, $s1p$, $e1p$, $s2p$, $e2p$, $s3p$, $e3p$, $s1n$, $e1n$, $s2n$, $e2n$, $s3n$, $e3n$, pinchX, pinchY, damage1, damage2, beta, curveTag, curveType)

This command is used to construct a uniaxial hysteretic material object with pinching of force and deformation, damage due to ductility and energy, and degraded unloading stiffness based on ductility. Failure of the material is defined by the associated Limit Curve.

<table>
<thead>
<tr>
<th>matTag (int)</th>
<th>integer tag identifying material</th>
</tr>
</thead>
<tbody>
<tr>
<td>$s1p$ e1p (float)</td>
<td>stress and strain (or force &amp; deformation) at first point of the envelope in the positive direction</td>
</tr>
<tr>
<td>$s2p$ e2p (float)</td>
<td>stress and strain (or force &amp; deformation) at second point of the envelope in the positive direction</td>
</tr>
<tr>
<td>$s3p$ e3p (float)</td>
<td>stress and strain (or force &amp; deformation) at third point of the envelope in the positive direction</td>
</tr>
<tr>
<td>$s1n$ e1n (float)</td>
<td>stress and strain (or force &amp; deformation) at first point of the envelope in the negative direction</td>
</tr>
<tr>
<td>$s2n$ e2n (float)</td>
<td>stress and strain (or force &amp; deformation) at second point of the envelope in the negative direction</td>
</tr>
<tr>
<td>$s3n$ e3n (float)</td>
<td>stress and strain (or force &amp; deformation) at third point of the envelope in the negative direction</td>
</tr>
<tr>
<td>pinchX (float)</td>
<td>pinching factor for strain (or deformation) during reloading</td>
</tr>
<tr>
<td>pinchY (float)</td>
<td>pinching factor for stress (or force) during reloading</td>
</tr>
<tr>
<td>damage1 (float)</td>
<td>damage due to ductility: $D_1(m-1)$</td>
</tr>
<tr>
<td>damage2 (float)</td>
<td>damage due to energy: $D_2(Ei/E_{ult})$</td>
</tr>
<tr>
<td>beta (float)</td>
<td>power used to determine the degraded unloading stiffness based on ductility, m-b (optional, default=0.0)</td>
</tr>
<tr>
<td>curveTag (int)</td>
<td>an integer tag for the Limit Curve defining the limit surface</td>
</tr>
<tr>
<td>curveType (int)</td>
<td>an integer defining the type of LimitCurve (0 = no curve, 1 = axial curve, all other curves can be any other integer)</td>
</tr>
</tbody>
</table>
Note:

- negative backbone points should be entered as negative numeric values

See also:

Notes

**MinMax Material**

`uniaxialMaterial` (`'MinMax`, `matTag`, `otherTag`, `'-min'`, `minStrain=1e-16`, `'-max'`, `maxStrain=1e16`)

This command is used to construct a MinMax material object. This stress-strain behaviour for this material is provided by another material. If however the strain ever falls below or above certain threshold values, the other material is assumed to have failed. From that point on, values of 0.0 are returned for the tangent and stress.

<table>
<thead>
<tr>
<th>matTag (int)</th>
<th>integer tag identifying material</th>
</tr>
</thead>
<tbody>
<tr>
<td>otherTag (float)</td>
<td>tag of the other material</td>
</tr>
<tr>
<td>minStrain (float)</td>
<td>minimum value of strain. optional default = -1.0e16.</td>
</tr>
<tr>
<td>maxStrain (float)</td>
<td>max value of strain. optional default = 1.0e16.</td>
</tr>
</tbody>
</table>

See also:

Notes

**ElasticBilin Material**

`uniaxialMaterial` (`'ElasticBilin`, `matTag`, `EP1`, `EP2`, `epsP2`, `EN1=EP1`, `EN2=EP2`, `epsN2=-epsP2`)

This command is used to construct an elastic bilinear uniaxial material object. Unlike all other bilinear materials, the unloading curve follows the loading curve exactly.

<table>
<thead>
<tr>
<th>matTag (int)</th>
<th>integer tag identifying material</th>
</tr>
</thead>
<tbody>
<tr>
<td>EP1 (float)</td>
<td>tangent in tension for stains: 0 &lt;= strains &lt;= epsP2</td>
</tr>
<tr>
<td>EP2 (float)</td>
<td>tangent when material in tension with strains &gt; epsP2</td>
</tr>
<tr>
<td>epsP2 (float)</td>
<td>strain at which material changes tangent in tension.</td>
</tr>
<tr>
<td>EN1 (float)</td>
<td>optional, default = EP1. tangent in compression for stains: 0 &lt; strains &lt;= epsN2</td>
</tr>
<tr>
<td>EN2 (float)</td>
<td>optional, default = EP2. tangent in compression with strains &lt; epsN2</td>
</tr>
<tr>
<td>epsN2 (float)</td>
<td>optional, default = -epsP2. strain at which material changes tangent in compression.</td>
</tr>
</tbody>
</table>

Note: `eps0` can not be controlled. It is always zero.

See also:

Notes

**ElasticMultiLinear Material**

`uniaxialMaterial` (`'ElasticMultiLinear`, `matTag`, `eta=0.0`, `'-strain'`, `*strainPoints`, `'-stress'`, `*stressPoints`)

This command is used to construct a multi-linear elastic uniaxial material object. The nonlinear stress-strain
relationship is given by a multi-linear curve that is define by a set of points. The behavior is nonlinear but it is elastic. This means that the material loads and unloads along the same curve, and no energy is dissipated. The slope given by the last two specified points on the positive strain axis is extrapolated to infinite positive strain. Similarly, the slope given by the last two specified points on the negative strain axis is extrapolated to infinite negative strain. The number of provided strain points needs to be equal to the number of provided stress points.

<table>
<thead>
<tr>
<th>matTag (int)</th>
<th>integer tag identifying material</th>
</tr>
</thead>
<tbody>
<tr>
<td>eta (float)</td>
<td>damping tangent (optional, default=0.0)</td>
</tr>
<tr>
<td>strainPoints (list (float))</td>
<td>list of strain points along stress-strain curve</td>
</tr>
<tr>
<td>stressPoints (list (float))</td>
<td>list of stress points along stress-strain curve</td>
</tr>
</tbody>
</table>

See also:

Notes

MultiLinear

uniaxialMaterial ('MultiLinear', matTag, *pts)

This command is used to construct a uniaxial multilinear material object.

<table>
<thead>
<tr>
<th>matTag (int)</th>
<th>integer tag identifying material</th>
</tr>
</thead>
<tbody>
<tr>
<td>pts (list (float))</td>
<td>a list of strain and stress points</td>
</tr>
<tr>
<td>pts = [strain1, stress1, strain2, stress2, ...]</td>
<td></td>
</tr>
</tbody>
</table>

See also:

Notes

Initial Strain Material

uniaxialMaterial ('InitStrainMaterial', matTag, otherTag, initStrain)

This command is used to construct an Initial Strain material object. The stress-strain behaviour for this material is defined by another material. Initial Strain Material enables definition of initial strains for the material under consideration. The stress that corresponds to the initial strain will be calculated from the other material.

<table>
<thead>
<tr>
<th>matTag (int)</th>
<th>integer tag identifying material</th>
</tr>
</thead>
<tbody>
<tr>
<td>otherTag (int)</td>
<td>tag of the other material</td>
</tr>
<tr>
<td>initStrain (float)</td>
<td>initial strain</td>
</tr>
</tbody>
</table>

See also:

Notes

Initial Stress Material

uniaxialMaterial ('InitStressMaterial', matTag, otherTag, initStress)

This command is used to construct an Initial Stress material object. The stress-strain behaviour for this material is defined by another material. Initial Stress Material enables definition of initial stress for the material under consideration. The strain that corresponds to the initial stress will be calculated from the other material.
**PathIndependent Material**

`uniaxialMaterial` (*PathIndependent*, `matTag`, `tag`)

This command is to create a PathIndependent material.

<table>
<thead>
<tr>
<th>matTag (int)</th>
<th>integer tag identifying material</th>
</tr>
</thead>
<tbody>
<tr>
<td>otherTag (float)</td>
<td>tag of the other material</td>
</tr>
<tr>
<td>initStress (float)</td>
<td>initial stress</td>
</tr>
</tbody>
</table>

**Pinching4 Material**

`uniaxialMaterial` (*Pinching4*, `matTag`, `ePf1`, `ePd1`, `ePf2`, `ePd2`, `ePf3`, `ePd3`, `ePf4`, `ePd4`, `eNf1`, `eNd1`, `eNf2`, `eNd2`, `eNf3`, `eNd3`, `eNf4`, `eNd4`, `rDispP`, `rForceP`, `uForceP`, `rDispN`, `rForceN`, `uForceN`, `gK1`, `gK2`, `gK3`, `gK4`, `gKLim`, `gD1`, `gD2`, `gD3`, `gD4`, `gDLim`, `gF1`, `gF2`, `gF3`, `gF4`, `gFLim`, `gE`, `dmgType`)

This command is used to construct a uniaxial material that represents a ‘pinched’ load-deformation response and exhibits degradation under cyclic loading. Cyclic degradation of strength and stiffness occurs in three ways: unloading stiffness degradation, reloading stiffness degradation, strength degradation.
<table>
<thead>
<tr>
<th>matTag (int)</th>
<th>integer tag identifying material</th>
</tr>
</thead>
<tbody>
<tr>
<td>ePf1 ePf2 ePf3 ePf4 (float)</td>
<td>floating point values defining force points on the positive response envelope</td>
</tr>
<tr>
<td>ePd1 ePd2 ePd3 ePd4 (float)</td>
<td>floating point values defining deformation points on the positive response envelope</td>
</tr>
<tr>
<td>eNf1 eNf2 eNf3 eNf4 (float)</td>
<td>floating point values defining force points on the negative response envelope</td>
</tr>
<tr>
<td>eNd1 eNd2 eNd3 eNd4 (float)</td>
<td>floating point values defining deformation points on the negative response envelope</td>
</tr>
<tr>
<td>rDispP (float)</td>
<td>floating value defining the ratio of the deformation at which reloading occurs to the maximum historic deformation demand</td>
</tr>
<tr>
<td>fForceP (float)</td>
<td>floating value defining the ratio of the force at which reloading begins to force corresponding to the maximum historic deformation demand</td>
</tr>
<tr>
<td>uForceP (float)</td>
<td>floating value defining the ratio of strength developed upon unloading from negative load to the maximum strength developed under monotonic loading</td>
</tr>
<tr>
<td>rDispN (float)</td>
<td>floating value defining the ratio of the deformation at which reloading occurs to the minimum historic deformation demand</td>
</tr>
<tr>
<td>fForceN (float)</td>
<td>floating value defining the ratio of the force at which reloading begins to force corresponding to the minimum historic deformation demand</td>
</tr>
<tr>
<td>uForceN (float)</td>
<td>floating value defining the ratio of strength developed upon unloading from negative load to the minimum strength developed under monotonic loading</td>
</tr>
<tr>
<td>gK1 gK2 gK3 gK4 gKLim (float)</td>
<td>floating point values controlling cyclic degradation model for unloading stiffness degradation</td>
</tr>
<tr>
<td>gD1 gD2 gD3 gD4 gDLim (float)</td>
<td>floating point values controlling cyclic degradation model for reloading stiffness degradation</td>
</tr>
<tr>
<td>gF1 gF2 gF3 gF4 gFLim (float)</td>
<td>floating point values controlling cyclic degradation model for strength degradation</td>
</tr>
<tr>
<td>gE (float)</td>
<td>floating point value used to define maximum energy dissipation under cyclic loading. Total energy dissipation capacity is defined as this factor multiplied by the energy dissipated under monotonic loading.</td>
</tr>
<tr>
<td>dmgType (str)</td>
<td>string to indicate type of damage (option: 'cycle', 'energy')</td>
</tr>
</tbody>
</table>

See also:

Notes

Engineered Cementitious Composites Material

uniaxialMaterial ('ECC01', matTag, sigt0, epst0, sigt1, epst1, sigc0, epsc0, epsc1, alphaT1, alphaT2, alphaC, alphaCU, betaT, betaC)

This command is used to construct a uniaxial Engineered Cementitious Composites (ECC) material object based on the ECC material model of Han, et al. (see references). reloading in tension and compression is linear.
matTag (int) integer tag identifying material
sigt0 (float) tensile cracking stress
epst0 (float) strain at tensile cracking stress
sigt1 (float) peak tensile stress
epst1 (float) strain at peak tensile stress
sigt2 (float) ultimate tensile strain
siggc0 (float) compressive strength (see NOTES)
epsc0 (float) strain at compressive strength (see NOTES)
epscl (float) ultimate compressive strain (see NOTES)
alphaT1 (float) exponent of the unloading curve in tensile strain hardening region
alphaT2 (float) exponent of the unloading curve in tensile softening region
alphaC (float) exponent of the unloading curve in the compressive softening
alphaCU (float) exponent of the compressive softening curve (use 1 for linear softening)
betaT (float) parameter to determine permanent strain in tension
betaC (float) parameter to determine permanent strain in compression

See also:
Notes

SelfCentering Material

uniaxialMaterial('SelfCentering', matTag, k1, k2, sigAct, beta, epsSlip, epsBear, rBear)
This command is used to construct a uniaxial self-centering (flag-shaped) material object with optional non-
recoverable slip behaviour and an optional stiffness increase at high strains (bearing behaviour).

matTag (int) integer tag identifying material
k1 (float) Initial Stiffness
k2 (float) Post-Activation Stiffness (0 < k2 < k1)
sigAct (float) Forward Activation Stress/Force
beta (float) Ratio of Forward to Reverse Activation Stress/Force
epsSlip (float) slip Strain/Deformation (if epsSlip = 0, there will be no slippage)
epsBear (float) Bearing Strain/Deformation (if epsBear = 0, there will be no bearing)
rBear (float) Ratio of Bearing Stiffness to Initial Stiffness k1

See also:
Notes

Viscous Material

uniaxialMaterial('Viscous', matTag)
This command is used to construct a uniaxial viscous material object. stress = C(strain-rate)^alpha

matTag (int) integer tag identifying material
C (float) damping coefficient
alpha (float) power factor (=1 means linear damping)

Note:
1. This material can only be assigned to truss and zeroLength elements.
2. This material can not be combined in parallel/series with other materials. When defined in parallel with other materials it is ignored.

See also:

Notes

**BoucWen Material**

uniaxialMaterial ('BoucWen', matTag, alpha, ko, n, gamma, beta, Ao, deltaA, deltaNu, deltaEta)

This command is used to construct a uniaxial Bouc-Wen smooth hysteretic material object. This material model is an extension of the original Bouc-Wen model that includes stiffness and strength degradation (Baber and Noori (1985)).

<table>
<thead>
<tr>
<th>matTag (int)</th>
<th>integer tag identifying material</th>
</tr>
</thead>
<tbody>
<tr>
<td>alpha (float)</td>
<td>ratio of post-yield stiffness to the initial elastic stiffness (0&lt; α &lt;1)</td>
</tr>
<tr>
<td>ko (float)</td>
<td>initial elastic stiffness</td>
</tr>
<tr>
<td>n (float)</td>
<td>parameter that controls transition from linear to nonlinear range (as n increases the transition becomes sharper; n is usually greater or equal to 1)</td>
</tr>
<tr>
<td>gamma beta (float)</td>
<td>parameters that control shape of hysteresis loop; depending on the values of γ and β softening, hardening or quasi-linearity can be simulated (look at the NOTES)</td>
</tr>
<tr>
<td>Ao deltaA (float)</td>
<td>parameters that control tangent stiffness</td>
</tr>
<tr>
<td>deltaNu deltaEta (float)</td>
<td>parameters that control material degradation</td>
</tr>
</tbody>
</table>

See also:

Notes

**BWBN Material**

uniaxialMaterial ('BWBN', matTag, alpha, ko, n, gamma, beta, Ao, q, zetas, p, Shi, deltaShi, lambda, tol, maxIter)

This command is used to construct a uniaxial Bouc-Wen pinching hysteretic material object. This material model is an extension of the original Bouc-Wen model that includes pinching (Baber and Noori (1986) and Foliente (1995)).
OpenSeesPy Documentation, Release 0.4.2019.7

### matTag (int)
integer tag identifying material

### alpha (float)
ratio of post-yield stiffness to the initial elastic stiffness (0 < α < 1)

### ko (float)
initial elastic stiffness

### n (float)
parameter that controls transition from linear to nonlinear range (as n increases the transition becomes sharper; n is usually greater or equal to 1)

### gamma, beta (float)
parameters that control shape of hysteresis loop; depending on the values of γ and β softening, hardening or quasi-linearity can be simulated (look at the BoucWen Material)

### Ao (float)
parameter that controls tangent stiffness

### q, zeta, p, Shi, delta, Shi, lambda (float)
parameters that control pinching

### tol (float)
tolerance

### maxIter (float)
maximum iterations

**See also:**

#### Notes

**KikuchiAikenHDR Material**

`uni axialMaterial ("KikuchiAikenHDR", matTag, tp, ar, hr[, \'-coGHU\', cg, ch, cu], \'-coMSS\', rs, rf)`

This command is used to construct a uniaxial KikuchiAikenHDR material object. This material model produces nonlinear hysteretic curves of high damping rubber bearings (HDRs).

| matTag (int) | integer tag identifying material |
| tp (str) | rubber type (see note 1) |
| ar (float) | area of rubber [unit: m²] (see note 2) |
| hr (float) | total thickness of rubber [unit: m] (see note 2) |
| cg, ch, cu (float) | correction coefficients for equivalent shear modulus (cg), equivalent viscous damping ratio (ch), ratio of shear force at zero displacement (cu). |
| rs, rf (float) | reduction rate for stiffness (rs) and force (rf) (see note 3) |

**Note:**

1) Following rubber types for `tp` are available:

- 'X0.6' Bridgestone X0.6, standard compressive stress, up to 400% shear strain
- 'X0.6-0MPa' Bridgestone X0.6, zero compressive stress, up to 400% shear strain
- 'X0.4' Bridgestone X0.4, standard compressive stress, up to 400% shear strain
- 'X0.4-0MPa' Bridgestone X0.4, zero compressive stress, up to 400% shear strain
- 'X0.3' Bridgestone X0.3, standard compressive stress, up to 400% shear strain
- 'X0.3-0MPa' Bridgestone X0.3, zero compressive stress, up to 400% shear strain

2) This material uses SI unit in calculation formula. ar and hr must be converted into [m²] and [m], respectively.
3) \( rs \) and \( rf \) are available if this material is applied to multipleShearSpring (MSS) element. Recommended values are \( rs = \frac{1}{\sum_{i=0}^{n-1} \sin(\pi i/n)^2} \) and \( rf = \frac{1}{\sum_{i=0}^{n-1} \sin(\pi i/n)} \), where \( n \) is the number of springs in the MSS. For example, when \( n=8 \), \( rs = 0.2500 \) and \( rf = 0.1989 \).

See also:

Notes

KikuchiAikenLRB Material

\texttt{uniaxialMaterial ('KikuchiAikenLRB', matTag, type, ar, hr, gr, ap, tp, alph, beta[, 'T', temp][, 'coKQ', rk, rq][, 'coMSS', rs, rf])}

This command is used to construct a uniaxial KikuchiAikenLRB material object. This material model produces nonlinear hysteretic curves of lead-rubber bearings.

| matTag (int) | integer tag identifying material |
| type (int) | rubber type (see note 1) |
| ar (float) | area of rubber [unit: \( m^2 \)] |
| hr (float) | total thickness of rubber [unit: \( m \)] |
| gr (float) | shear modulus of rubber [unit: \( N/m^2 \)] |
| ap (float) | area of lead plug [unit: \( m^2 \)] |
| tp (float) | yield stress of lead plug [unit: \( N/m^2 \)] |
| alph (float) | shear modulus of lead plug [unit: \( N/m^2 \)] |
| beta (float) | ratio of initial stiffness to yielding stiffness |
| temp (float) | temperature [unit: \( ^\circ C \)] |
| rk rq (float) | reduction rate for yielding stiffness (\( rk \)) and force at zero displacement (\( rq \)) |
| rs rf (float) | reduction rate for stiffness (\( rs \)) and force (\( rf \)) (see note 3) |

Note:

1) Following rubber types for \( type \) are available:
   - 1 lead-rubber bearing, up to 400% shear strain [Kikuchi et al., 2010 & 2012]

2) This material uses SI unit in calculation formula. Input arguments must be converted into [\( m \)], [\( m^2 \)], [\( N/m^2 \)].

3) \( rs \) and \( rf \) are available if this material is applied to multipleShearSpring (MSS) element. Recommended values are \( rs = \frac{1}{\sum_{i=0}^{n-1} \sin(\pi i/n)^2} \) and \( rf = \frac{1}{\sum_{i=0}^{n-1} \sin(\pi i/n)} \), where \( n \) is the number of springs in the MSS. For example, when \( n=8 \), \( rs = 0.2500 \) and \( rf = 0.1989 \).

See also:

Notes

AxialSp Material

\texttt{uniaxialMaterial ('AxialSp', matTag, sce, fty, fcy[, bte, bty, bcy, fcr])}

This command is used to construct a uniaxial AxialSp material object. This material model produces axial stress-strain curve of elastomeric bearings.

1.4. Model Commands 145
| matTag (int) | integer tag identifying material |
| scl (float) | compressive modulus |
| fty fcy (float) | yield stress under tension (fty) and compression (fcy) (see note 1) |
| bte bty bcy (float) | reduction rate for tensile elastic range (bte), tensile yielding (bty) and compressive yielding (bcy) (see note 1) |
| fcr (float) | target point stress (see note 1) |

**Note:**

1. Input parameters are required to satisfy followings.

\[
\begin{align*}
fcy &< 0.0 < fty \\
0.0 &\leq bty < bte \leq 1.0 \\
0.0 &\leq bcy \leq 1.0 \\
fcy &\leq fcr \leq 0.0
\end{align*}
\]

See also:

Notes

**AxialSpHD Material**

`uniaxialMaterial ('AxialSpHD', matTag, sce, fty, fcy[, bte, bty, bth, bcy, fcr, ath])`

This command is used to construct a uniaxial AxialSpHD material object. This material model produces axial stress-strain curve of elastomeric bearings including hardening behavior.

| matTag (int) | integer tag identifying material |
| scl (float) | compressive modulus |
| fty fcy (float) | yield stress under tension (fty) and compression (fcy) (see note 1) |
| bte bty bth bcy (float) | reduction rate for tensile elastic range (bte), tensile yielding (bty), tensile hardening (bth) and compressive yielding (bcy) (see note 1) |
| fcr (float) | target point stress (see note 1) |

**Note:**

1. Input parameters are required to satisfy followings.

\[
\begin{align*}
fcy &< 0.0 < fty \\
0.0 &\leq bty < bth < bte \leq 1.0 \\
0.0 &\leq bcy \leq 1.0 \\
fcy &\leq fcr \leq 0.0 \\
1.0 &\leq ath
\end{align*}
\]

See also:

Notes
Pinching Limit State Material

This command is used to construct a uniaxial material that simulates a pinched load-deformation response and exhibits degradation under cyclic loading. This material works with the RotationShearCurve limit surface that can monitor a key deformation and/or a key force in an associated frame element and trigger a degrading behavior in this material when a limiting value of the deformation and/or force are reached. The material can be used in two modes: 1) direct input mode, where pinching and damage parameters are directly input; and 2) calibrated mode for shear-critical concrete columns, where only key column properties are input for model to fully define pinching and damage parameters.

```
uniaxialMaterial('PinchingLimitStateMaterial', matTag, nodeT, nodeB, driftAxis, Kelas, crvTyp, crvTag, YpinchUPN, YpinchRPN, XpinchRPN, YpinchUNP, XpinchRNP, dmgStrsLimE, dmgDispMax, dmgE1, dmgE2, dmgE3, dmgE4, dmgELim, dmgR1, dmgR2, dmgR3, dmgR4, dmgRLim, dmgRCyc, dmgS1, dmgS2, dmgS3, dmgS4, dmgSLim, dmgSCyc)
```

MODE 1: Direct Input
<table>
<thead>
<tr>
<th>Description</th>
<th>Type</th>
<th>Notes</th>
</tr>
</thead>
<tbody>
<tr>
<td>matTag</td>
<td>int</td>
<td>integer tag identifying material</td>
</tr>
<tr>
<td>nodeT</td>
<td>int</td>
<td>integer node tag to define the first node at the extreme end of the associated flexural frame member</td>
</tr>
<tr>
<td>nodeB</td>
<td>int</td>
<td>integer node tag to define the last node at the extreme end of the associated flexural frame member</td>
</tr>
<tr>
<td>driftAxis</td>
<td>int</td>
<td>integer to indicate the drift axis in which lateral-strength degradation will occur. This axis should be orthogonal to the axis of measured rotation (see rotAxis in Rotation Shear Curve definition)</td>
</tr>
<tr>
<td>Kelas</td>
<td>float</td>
<td>floating point value to define the initial material elastic stiffness (Kelastic); Kelas &gt; 0</td>
</tr>
<tr>
<td>crvTyp</td>
<td>int</td>
<td>integer flag to indicate the type of limit curve associated with this material.</td>
</tr>
<tr>
<td>YpinchUP</td>
<td>float</td>
<td>floating point unloading force pinching factor for loading in the negative direction. Note: This value must be between zero and unity</td>
</tr>
<tr>
<td>YpinchRPN</td>
<td>float</td>
<td>floating point reloading force pinching factor for loading in the negative direction. Note: This value must be between negative one and unity</td>
</tr>
<tr>
<td>XpinchRPN</td>
<td>float</td>
<td>floating point reloading displacement pinching factor for loading in the negative direction. Note: This value must be between negative one and unity</td>
</tr>
<tr>
<td>YpinchUP</td>
<td>float</td>
<td>floating point unloading force pinching factor for loading in the positive direction. Note: This value must be between zero and unity</td>
</tr>
<tr>
<td>YpinchRPN</td>
<td>float</td>
<td>floating point reloading force pinching factor for loading in the positive direction. Note: This value must be between negative one and unity</td>
</tr>
<tr>
<td>dmgS1</td>
<td>float</td>
<td>floating point backbone strength damage factors α1,α2,α3,α4 shown in Eq. 1</td>
</tr>
<tr>
<td>dmgS2</td>
<td>float</td>
<td>floating point backbone strength damage limit Dlim shown in Eq. 1; Note: This value must be between zero and unity</td>
</tr>
<tr>
<td>dmgS3</td>
<td>float</td>
<td>floating point backbone strength damage factors α1,α2,α3,α4 shown in Eq. 1</td>
</tr>
<tr>
<td>dmgS4</td>
<td>float</td>
<td>floating point backbone strength damage limit Dlim shown in Eq. 1; Note: This value must be between zero and unity</td>
</tr>
<tr>
<td>dmgR1</td>
<td>float</td>
<td>floating point reloading stiffness damage index; Note: This value must be between zero and unity</td>
</tr>
<tr>
<td>dmgR2</td>
<td>float</td>
<td>floating point reloading stiffness damage factors α1,α2,α3,α4 shown in Eq. 1</td>
</tr>
<tr>
<td>dmgR3</td>
<td>float</td>
<td>floating point reloading stiffness damage limit Dlim shown in Eq. 1; Note: This value must be between zero and unity</td>
</tr>
<tr>
<td>dmgR4</td>
<td>float</td>
<td>floating point reloading stiffness damage factors α1,α2,α3,α4 shown in Eq. 1</td>
</tr>
<tr>
<td>dmgE1</td>
<td>float</td>
<td>floating point elastic stiffness damage factors α1,α2,α3,α4 shown in Eq. 1</td>
</tr>
<tr>
<td>dmgE2</td>
<td>float</td>
<td>floating point elastic stiffness damage limit Dlim shown in Eq. 1; Note: This value must be between zero and unity</td>
</tr>
<tr>
<td>dmgE3</td>
<td>float</td>
<td>floating point elastic stiffness damage limit Dlim shown in Eq. 1; Note: This value must be between zero and unity</td>
</tr>
<tr>
<td>dmgE4</td>
<td>float</td>
<td>floating point elastic stiffness damage limit Dlim shown in Eq. 1; Note: This value must be between zero and unity</td>
</tr>
<tr>
<td>dmgE1, dmgE2, dmgE3, dmgE4</td>
<td>float</td>
<td>floating point elastic stiffness damage factors α1,α2,α3,α4 shown in Eq. 1</td>
</tr>
<tr>
<td>dmgR1, dmgR2, dmgR3, dmgR4, dmgRLim</td>
<td>float</td>
<td>floating point reloading stiffness damage factors α1,α2,α3,α4 shown in Eq. 1</td>
</tr>
<tr>
<td>dmgE1, dmgE2, dmgE3, dmgE4, dmgELim</td>
<td>float</td>
<td>floating point elastic stiffness damage limit Dlim shown in Eq. 1; Note: This value must be between zero and unity</td>
</tr>
<tr>
<td>dmgR1, dmgR2, dmgR3, dmgR4, dmgRLim</td>
<td>float</td>
<td>floating point reloading stiffness damage factors α1,α2,α3,α4 shown in Eq. 1</td>
</tr>
<tr>
<td>dmgE1, dmgE2, dmgE3, dmgE4, dmgELim</td>
<td>float</td>
<td>floating point elastic stiffness damage limit Dlim shown in Eq. 1; Note: This value must be between zero and unity</td>
</tr>
<tr>
<td>dmgR1, dmgR2, dmgR3, dmgR4, dmgRLim</td>
<td>float</td>
<td>floating point reloading stiffness damage factors α1,α2,α3,α4 shown in Eq. 1</td>
</tr>
<tr>
<td>dmgE1, dmgE2, dmgE3, dmgE4, dmgELim</td>
<td>float</td>
<td>floating point elastic stiffness damage limit Dlim shown in Eq. 1; Note: This value must be between zero and unity</td>
</tr>
<tr>
<td>dmgR1, dmgR2, dmgR3, dmgR4, dmgRLim</td>
<td>float</td>
<td>floating point reloading stiffness damage factors α1,α2,α3,α4 shown in Eq. 1</td>
</tr>
<tr>
<td>dmgE1, dmgE2, dmgE3, dmgE4, dmgELim</td>
<td>float</td>
<td>floating point elastic stiffness damage limit Dlim shown in Eq. 1; Note: This value must be between zero and unity</td>
</tr>
</tbody>
</table>
uniAxialMaterial (‘PinchingLimitStateMaterial’, matTag, dnodeT, nodeB, driftAxis, Kelas, crvTyp, crvTag, eleTag, b, d, h, a, st, As, Acc, ld, db, rhoT, fc, fy, fyt)

MODE 2: Calibrated Model for Shear-Critical Concrete Columns
<table>
<thead>
<tr>
<th>matTag</th>
<th>integer tag identifying material</th>
</tr>
</thead>
<tbody>
<tr>
<td>nodeT</td>
<td>integer node tag to define the first node at the extreme end of the associated flexural frame member (L3 or D5 in Figure)</td>
</tr>
<tr>
<td>nodeB</td>
<td>integer node tag to define the last node at the extreme end of the associated flexural frame member (L2 or D2 in Figure)</td>
</tr>
<tr>
<td>driftAxis</td>
<td>integer to indicate the drift axis in which lateral-strength degradation will occur. This axis should be orthogonal to the axis of measured rotation (see rotAxis’ in Rotation Shear Curve definition): driftAxis = 1 – Drift along the x-axis, driftAxis = 2 – Drift along the y-axis, driftAxis = 3 – Drift along the z-axis</td>
</tr>
<tr>
<td>Kelas</td>
<td>floating point value to define the shear stiffness (Kelastic) of the shear spring prior to shear failure: Kelas = -4 – Shear stiffness calculated assuming double curvature and shear springs at both column element ends, Kelas = -3 – Shear stiffness calculated assuming double curvature and a shear spring at one column element end, Kelas = -2 – Shear stiffness calculated assuming single curvature and shear springs at both column element ends, Kelas = -1 – Shear stiffness calculated assuming single curvature and a shear spring at one column element end, Kelas &gt; 0 – Shear stiffness is the input value. Note: integer inputs allow the model to know whether column height equals the shear span (cantelever) or twice the shear span (double curvature). For columns in frames, input the value for the case that best approximates column end conditions or manually input shear stiffness (typically double curvature better estimates framed column behavior)</td>
</tr>
<tr>
<td>crvTag</td>
<td>integer tag for the unique limit curve object associated with this material</td>
</tr>
<tr>
<td>eleTag</td>
<td>integer element tag to define the associated beam-column element used to extract axial load</td>
</tr>
<tr>
<td>b</td>
<td>floating point column width (inches)</td>
</tr>
<tr>
<td>d</td>
<td>floating point column depth (inches)</td>
</tr>
<tr>
<td>h</td>
<td>floating point column height (inches)</td>
</tr>
<tr>
<td>a</td>
<td>floating point shear span length (inches)</td>
</tr>
<tr>
<td>st</td>
<td>floating point transverse reinforcement spacing (inches) along column height</td>
</tr>
<tr>
<td>As</td>
<td>floating point total area (inches squared) of longitudinal steel bars in section</td>
</tr>
<tr>
<td>Acc</td>
<td>floating point gross confined concrete area (inches squared) bounded by the transverse reinforcement in column section</td>
</tr>
<tr>
<td>ld</td>
<td>floating point development length (inches) of longitudinal bars using ACI 318-11 Eq. 12-1 and Eq. 12-2</td>
</tr>
<tr>
<td>db</td>
<td>floating point diameter (inches) of longitudinal bars in column section</td>
</tr>
<tr>
<td>rhot</td>
<td>floating point transverse reinforcement ratio (Ast/st.db)</td>
</tr>
<tr>
<td>f'c</td>
<td>floating point concrete compressive strength (ksi)</td>
</tr>
<tr>
<td>fy</td>
<td>floating point longitudinal steel yield strength (ksi)</td>
</tr>
<tr>
<td>fyt</td>
<td>floating point transverse steel yield strength (ksi)</td>
</tr>
</tbody>
</table>
CFSWSWP Wood-Sheathed Cold-Formed Steel Shear Wall Panel

uniaxialMaterial ('CFSWSWP', matTag, height, width, fut, tf, Ife, Ifi, ts, np, ds, Vs, sc, nc, type,
onopeningArea, openingLength)

This command is used to construct a uniaxialMaterial model that simulates the hysteresis response (Shear strength-Lateral displacement) of a wood-sheathed cold-formed steel shear wall panel (CFS-SWP). The hysteresis model has smooth curves and takes into account the strength and stiffness degradation, as well as pinching effect.

This uniaxialMaterial gives results in Newton and Meter units, for strength and displacement, respectively.

<table>
<thead>
<tr>
<th>matTag (int)</th>
<th>integer tag identifying material</th>
</tr>
</thead>
<tbody>
<tr>
<td>height (float)</td>
<td>SWP’s height (mm)</td>
</tr>
<tr>
<td>width (float)</td>
<td>SWP’s width (mm)</td>
</tr>
<tr>
<td>fut (float)</td>
<td>Tensile strength of framing members (MPa)</td>
</tr>
<tr>
<td>tf (float)</td>
<td>Framing thickness (mm)</td>
</tr>
<tr>
<td>Ife (float)</td>
<td>Moment of inertia of the double end-stud (mm4)</td>
</tr>
<tr>
<td>Ifi (float)</td>
<td>Moment of inertia of the intermediate stud (mm4)</td>
</tr>
<tr>
<td>ts (float)</td>
<td>Sheathing thickness (mm)</td>
</tr>
<tr>
<td>np (float)</td>
<td>Sheathing number (one or two sides sheathed)</td>
</tr>
<tr>
<td>ds (float)</td>
<td>Screws diameter (mm)</td>
</tr>
<tr>
<td>Vs (float)</td>
<td>Screws shear strength (N)</td>
</tr>
<tr>
<td>sc (float)</td>
<td>Screw spacing on the SWP perimeter (mm)</td>
</tr>
<tr>
<td>nc (float)</td>
<td>Total number of screws located on the SWP perimeter</td>
</tr>
<tr>
<td>type (int)</td>
<td>Integer identifier used to define wood sheathing type (DFP=1, OSB=2, CSP=3)</td>
</tr>
<tr>
<td>openingArea (float)</td>
<td>Total area of openings (mm²)</td>
</tr>
<tr>
<td>openingLength (float)</td>
<td>Cumulative length of openings (mm)</td>
</tr>
</tbody>
</table>

Notes

CFSSS WP Steel-Sheathed Cold-formed Steel Shear Wall Panel

uniaxialMaterial ('CFSSSWP', matTag, height, width, fut, fyf, tf, Af, fus, fys, ts, np, ds, Vs, sc, dt,
onopeningArea, openingLength)

This command is used to construct a uniaxialMaterial model that simulates the hysteresis response (Shear strength-lateral Displacement) of a Steel-Sheathed Cold-Formed Steel Shear Wall Panel (CFS-SWP). The hysteresis model has smooth curves and takes into account the strength and stiffness degradation, as well as pinching effect.

This uniaxialMaterial gives results in Newton and Meter units, for strength and displacement, respectively.

Notes
### matTag (int)
- integer tag identifying material

### height (float)
- SWP’s height (mm)

### width (float)
- SWP’s width (mm)

### fuf (float)
- Tensile strength of framing members (MPa)

### fyf (float)
- Yield strength of framing members (MPa)

### tf (float)
- Framing thickness (mm)

### Af (float)
- Framing cross section area (mm²)

### fus (float)
- Tensile strength of steel sheet sheathing (MPa)

### fys (float)
- Yield strength of steel sheet sheathing (MPa)

### ts (float)
- Sheathing thickness (mm)

### np (float)
- Sheathing number (one or two sides sheathed)

### ds (float)
- Screws diameter (mm)

### Vs (float)
- Screws shear strength (N)

### sc (float)
- Screw spacing on the SWP perimeter (mm)

### dt (float)
- Anchor bolt’s diameter (mm)

### openingArea (float)
- Total area of openings (mm²)

### openingLength (float)
- Cumulative length of openings (mm)

---

**See also:**

**Notes**

### 1.4.14 nDMaterial commands

**nDMaterial** *(matType, matTag, *matArgs)*

This command is used to construct an NDMaterial object which represents the stress-strain relationship at the gauss-point of a continuum element.

<table>
<thead>
<tr>
<th>matType (str)</th>
<th>material type</th>
</tr>
</thead>
<tbody>
<tr>
<td>matTag (int)</td>
<td>material tag.</td>
</tr>
<tr>
<td>matArgs (list)</td>
<td>a list of material arguments, must be preceded with *..</td>
</tr>
</tbody>
</table>

For example,

```python
matType = 'ElasticIsotropic'
mTag = 1
matArgs = [E, v]
nDMaterial(matType, matTag, *matArgs)
```

The following contain information about available **matType**:

1. **ElasticIsotropic**
2. **ElasticOrthotropic**
3. **J2Plasticity**
4. **DrukerPrager**
5. **Damage2p**
6. **PlaneStress**
7. **PlaneStrain**
8. **MultiaxialCyclicPlasticity**
9. *BoundingCamClay*
10. *PlateFiber*
11. *FSAM*
12. *ManzariDafalias*
13. *PM4Sand*
14. *StressDensityModel*
15. *AcousticMedium*

**ElasticIsotropic**

```python
nDMaterial('ElasticIsotropic', matTag, E, v, rho=0.0)
```

This command is used to construct an ElasticIsotropic material object.

<table>
<thead>
<tr>
<th>matTag (int)</th>
<th>integer tag identifying material</th>
</tr>
</thead>
<tbody>
<tr>
<td>E (float)</td>
<td>elastic modulus</td>
</tr>
<tr>
<td>v (float)</td>
<td>Poisson’s ratio</td>
</tr>
<tr>
<td>rho (float)</td>
<td>mass density (optional)</td>
</tr>
</tbody>
</table>

The material formulations for the ElasticIsotropic object are:

- 'ThreeDimensional'
- 'PlaneStrain'
- 'Plane Stress'
- 'AxiSymmetric'
- 'PlateFiber'

**ElasticOrthotropic**

```python
nDMaterial('ElasticOrthotropic', matTag, Ex, Ey, Ez, vxy, vyz, vzx, Gxy, Gyz, Gzx, rho=0.0)
```

This command is used to construct an ElasticOrthotropic material object.

<table>
<thead>
<tr>
<th>matTag (int)</th>
<th>integer tag identifying material</th>
</tr>
</thead>
<tbody>
<tr>
<td>Ex (float)</td>
<td>elastic modulus in x direction</td>
</tr>
<tr>
<td>Ey (float)</td>
<td>elastic modulus in y direction</td>
</tr>
<tr>
<td>Ez (float)</td>
<td>elastic modulus in z direction</td>
</tr>
<tr>
<td>vxy (float)</td>
<td>Poisson’s ratios in x and y plane</td>
</tr>
<tr>
<td>vyz (float)</td>
<td>Poisson’s ratios in y and z plane</td>
</tr>
<tr>
<td>vzx (float)</td>
<td>Poisson’s ratios in z and x plane</td>
</tr>
<tr>
<td>Gxy (float)</td>
<td>shear modulii in x and y plane</td>
</tr>
<tr>
<td>Gyz (float)</td>
<td>shear modulii in y and z plane</td>
</tr>
<tr>
<td>Gzx (float)</td>
<td>shear modulii in z and x plane</td>
</tr>
<tr>
<td>rho (float)</td>
<td>mass density (optional)</td>
</tr>
</tbody>
</table>

The material formulations for the ElasticOrthotropic object are:

- 'ThreeDimensional'
- 'PlaneStrain'
This command is used to construct a multi dimensional material object that has a von Mises (J2) yield criterion and isotropic hardening.

<table>
<thead>
<tr>
<th>matTag (int)</th>
<th>integer tag identifying material</th>
</tr>
</thead>
<tbody>
<tr>
<td>K (float)</td>
<td>bulk modulus</td>
</tr>
<tr>
<td>G (float)</td>
<td>shear modulus</td>
</tr>
<tr>
<td>sig0 (float)</td>
<td>initial yield stress</td>
</tr>
<tr>
<td>sigInf (float)</td>
<td>final saturation yield stress</td>
</tr>
<tr>
<td>delta (float)</td>
<td>exponential hardening parameter</td>
</tr>
<tr>
<td>H (float)</td>
<td>linear hardening parameter</td>
</tr>
</tbody>
</table>

The material formulations for the J2Plasticity object are:

- 'ThreeDimensional'
- 'PlaneStrain'
- 'Plane Stress'
- 'AxiSymmetric'
- 'PlateFiber'

J2 isotropic hardening material class

Elastic Model

\[
\sigma = K \cdot \text{trace}(\epsilon_e) + (2 \cdot G) \cdot \text{dev}(\epsilon_e)
\]

Yield Function

\[
\phi(\sigma, q) = ||\text{dev}(\sigma)|| - \sqrt{\frac{2}{3} \cdot q(x_i)}
\]

Saturation Isotropic Hardening with linear term

\[
q(x_i) = \sigma_0 + (\sigma_\infty - \sigma_0) \cdot \exp(-\delta \cdot \xi) + H \cdot \xi
\]

Flow Rules

\[
\dot{\epsilon}_p = \gamma \frac{\partial \phi}{\partial \sigma}
\]

\[
\dot{\xi} = -\gamma \frac{\partial \phi}{\partial q}
\]

Linear Viscosity

\[
\gamma = \frac{\partial \phi}{\eta} (if \phi > 0)
\]

Backward Euler Integration Routine Yield condition enforced at time n+1

set \( \eta = 0 \) for rate independent case
DrukerPrager

**nDMaterial** ('DrukerPrager', matTag, K, G, sigmaY, rho, rhoBar, Kinf, Ko, delta1, delta2, H, theta, density, atmPressure=101e3)

This command is used to construct an multi dimensional material object that has a Drucker-Prager yield criterion.

<table>
<thead>
<tr>
<th>Parameter</th>
<th>Description</th>
</tr>
</thead>
<tbody>
<tr>
<td>matTag (int)</td>
<td>integer tag identifying material</td>
</tr>
<tr>
<td>K (float)</td>
<td>bulk modulus</td>
</tr>
<tr>
<td>G (float)</td>
<td>shear modulus</td>
</tr>
<tr>
<td>sigmaY (float)</td>
<td>yield stress</td>
</tr>
<tr>
<td>rho (float)</td>
<td>frictional strength parameter</td>
</tr>
<tr>
<td>rhoBar (float)</td>
<td>controls evolution of plastic volume change, $0 \leq \rhoBar \leq \rho$</td>
</tr>
<tr>
<td>Kinf (float)</td>
<td>nonlinear isotropic strain hardening parameter, $Kinf \geq 0$</td>
</tr>
<tr>
<td>Ko (float)</td>
<td>nonlinear isotropic strain hardening parameter, $Ko \geq 0$</td>
</tr>
<tr>
<td>delta1 (float)</td>
<td>nonlinear isotropic strain hardening parameter, $\delta_1 \geq 0$</td>
</tr>
<tr>
<td>delta2 (float)</td>
<td>tension softening parameter, $\delta_2 \geq 0$</td>
</tr>
<tr>
<td>H (float)</td>
<td>linear hardening parameter, $H \geq 0$</td>
</tr>
<tr>
<td>theta (float)</td>
<td>controls relative proportions of isotropic and kinematic hardening, $0 \leq \theta \leq 1$</td>
</tr>
<tr>
<td>density (float)</td>
<td>mass density of the material</td>
</tr>
<tr>
<td>atmPressure (float)</td>
<td>optional atmospheric pressure for update of elastic bulk and shear moduli</td>
</tr>
</tbody>
</table>

The material formulations for the DrukerPrager object are:

- 'ThreeDimensional'
- 'PlaneStrain'

See theory.

**Damage2p**

**nDMaterial** ('Damage2p', matTag, fcc, '-fct', fct, '-E', E, '-ni', ni, '-Gt', Gt, '-Gc', Gc, '-rho_bar', rho_bar, '-H', H, '-theta', theta, '-tangent', tangent)

This command is used to construct a three-dimensional material object that has a Drucker-Prager plasticity model coupled with a two-parameter damage model.
The material formulations for the Damage2p object are:

- 'ThreeDimensional'
- 'PlaneStrain'
- 'Plane Stress'
- 'AxiSymmetric'
- 'PlateFiber'

See also here.

**PlaneStress**

**nDMaterial** ('PlaneStress', matTag, threeDtag)

This command is used to construct a plane-stress material wrapper which converts any three-dimensional material into a plane stress material via static condensation.

<table>
<thead>
<tr>
<th>matTag (int)</th>
<th>integer tag identifying material</th>
</tr>
</thead>
<tbody>
<tr>
<td>threeDtag (int)</td>
<td>tag of perviously defined 3d ndMaterial material</td>
</tr>
</tbody>
</table>

The material formulations for the PlaneStress object are:

- 'Plane Stress'
PlaneStrain

nDMaterial ("PlaneStrain", matTag, threeDtag)
This command is used to construct a plane-stress material wrapper which converts any three-dimensional material into a plane strain material by imposing plain strain conditions on the three-dimensional material.

<table>
<thead>
<tr>
<th>matTag (int)</th>
<th>integer tag identifying material</th>
</tr>
</thead>
<tbody>
<tr>
<td>threeDtag (int)</td>
<td>integer tag of previously defined 3d ndMaterial material</td>
</tr>
</tbody>
</table>

The material formulations for the PlaneStrain object are:
- 'PlaneStrain'

MultiaxialCyclicPlasticity

nDMaterial ("MultiaxialCyclicPlasticity", matTag, rho, K, G, Su, Ho, h, m, beta, KCoeff)
This command is used to construct an multiaxial Cyclic Plasticity model for clays

<table>
<thead>
<tr>
<th>matTag (int)</th>
<th>integer tag identifying material</th>
</tr>
</thead>
<tbody>
<tr>
<td>rho (float)</td>
<td>density</td>
</tr>
<tr>
<td>K (float)</td>
<td>buck modulus</td>
</tr>
<tr>
<td>G (float)</td>
<td>maximum (small strain) shear modulus</td>
</tr>
<tr>
<td>Su (float)</td>
<td>undrained shear strength, size of bounding surface $R = \sqrt{\frac{8}{3}} * Su$</td>
</tr>
<tr>
<td>Ho (float)</td>
<td>linear kinematic hardening modulus of bounding surface</td>
</tr>
<tr>
<td>h (float)</td>
<td>hardening parameter</td>
</tr>
<tr>
<td>m (float)</td>
<td>hardening parameter</td>
</tr>
<tr>
<td>beta (float)</td>
<td>integration parameter, usually beta=0.5</td>
</tr>
<tr>
<td>KCoeff (float)</td>
<td>coefficient of earth pressure, K0</td>
</tr>
</tbody>
</table>

BoundingCamClay

nDMaterial ("BoundingCamClay", matTag, massDensity, C, bulkMod, OCR, mu_o, alpha, lambda, h, m)
This command is used to construct a multi-dimensional bounding surface Cam Clay material object after Borja et al. (2001).

<table>
<thead>
<tr>
<th>matTag (int)</th>
<th>integer tag identifying material</th>
</tr>
</thead>
<tbody>
<tr>
<td>massDensity (float)</td>
<td>mass density</td>
</tr>
<tr>
<td>C (float)</td>
<td>ellipsoidal axis ratio (defines shape of ellipsoidal loading/bounding surfaces)</td>
</tr>
<tr>
<td>bulkMod (float)</td>
<td>initial bulk modulus</td>
</tr>
<tr>
<td>OCR (float)</td>
<td>overconsolidation ratio</td>
</tr>
<tr>
<td>mu_o (float)</td>
<td>initial shear modulus</td>
</tr>
<tr>
<td>alpha (float)</td>
<td>pressure-dependency parameter for modulii (greater than or equal to zero)</td>
</tr>
<tr>
<td>lambda (float)</td>
<td>soil compressibility index for virgin loading</td>
</tr>
<tr>
<td>h (float)</td>
<td>hardening parameter for plastic response inside of bounding surface (if h = 0, no hardening)</td>
</tr>
<tr>
<td>m (float)</td>
<td>hardening parameter (exponent) for plastic response inside of bounding surface (if m = 0, only linear hardening)</td>
</tr>
</tbody>
</table>

The material formulations for the BoundingCamClay object are:
'ThreeDimensional'
'PlaneStrain'

See also for information

PlateFiber

**nDMaterial** ('PlateFiber', matTag, threeDTag)

This command is used to construct a plate-fiber material wrapper which converts any three-dimensional material into a plate fiber material (by static condensation) appropriate for shell analysis.

<table>
<thead>
<tr>
<th>matTag (int)</th>
<th>integer tag identifying material</th>
</tr>
</thead>
<tbody>
<tr>
<td>threeDTag (float)</td>
<td>material tag for a previously-defined three-dimensional material</td>
</tr>
</tbody>
</table>

FSAM

**nDMaterial** ('FSAM', matTag, rho, sX, sY, conc, rouX, rouY, nu, alfadow)

This command is used to construct a nDMaterial FSAM (Fixed-Strut-Angle-Model, Figure 1, Kolozvari et al., 2015), which is a plane-stress constitutive model for simulating the behavior of RC panel elements under generalized, in-plane, reversed-cyclic loading conditions (Ulugtekin, 2010; Orakcal et al., 2012). In the FSAM constitutive model, the strain fields acting on concrete and reinforcing steel components of a RC panel are assumed to be equal to each other, implying perfect bond assumption between concrete and reinforcing steel bars. While the reinforcing steel bars develop uniaxial stresses under strains in their longitudinal direction, the behavior of concrete is defined using stress–strain relationships in biaxial directions, the orientation of which is governed by the state of cracking in concrete. Although the concrete stress–strain relationship used in the FSAM is fundamentally uniaxial in nature, it also incorporates biaxial softening effects including compression softening and biaxial damage. For transfer of shear stresses across the cracks, a friction-based elasto-plastic shear aggregate interlock model is adopted, together with a linear elastic model for representing dowel action on the reinforcing steel bars (Kolozvari, 2013). Note that FSAM constitutive model is implemented to be used with Shear-Flexure Interaction model for RC walls (SFI_MVLEM), but it could be also used elsewhere.

<table>
<thead>
<tr>
<th>matTag (int)</th>
<th>integer tag identifying material</th>
</tr>
</thead>
<tbody>
<tr>
<td>rho (float)</td>
<td>Material density</td>
</tr>
<tr>
<td>sX (float)</td>
<td>Tag of uniaxialMaterial simulating horizontal (x) reinforcement</td>
</tr>
<tr>
<td>sY (float)</td>
<td>Tag of uniaxialMaterial simulating vertical (y) reinforcement</td>
</tr>
<tr>
<td>conc (float)</td>
<td>Tag of uniaxialMaterial simulating concrete, shall be used with uniaxialMaterial ConcreteCM</td>
</tr>
<tr>
<td>rouX (float)</td>
<td>Reinforcing ratio in horizontal (x) direction (( \text{rouX} = \frac{s_{x}}{A_{\text{gross,x}}} ))</td>
</tr>
<tr>
<td>rouY (float)</td>
<td>Reinforcing ratio in vertical (x) direction (( \text{rouY} = \frac{s_{y}}{A_{\text{gross,y}}} ))</td>
</tr>
<tr>
<td>nu (float)</td>
<td>Concrete friction coefficient (0.0 &lt; ( \nu ) &lt; 1.5)</td>
</tr>
<tr>
<td>alfadow (float)</td>
<td>Stiffness coefficient of reinforcement dowel action (0.0 &lt; ( \alpha_{\text{dowel}} ) &lt; 0.05)</td>
</tr>
</tbody>
</table>

See also here

References:


**ManzariDafalias**

`nDMaterial` ("ManzariDafalias", matTag, G0, nu, e_init, Mc, c, lambda_c, e0, ksi, P_atm, m, h0, ch, nb, A0, nd, z_max, cz, Den)

This command is used to construct a multi-dimensional Manzari-Dafalias(2004) material.

<table>
<thead>
<tr>
<th>Parameter</th>
<th>Description</th>
</tr>
</thead>
<tbody>
<tr>
<td>matTag (int)</td>
<td>integer tag identifying material</td>
</tr>
<tr>
<td>G0 (float)</td>
<td>shear modulus constant</td>
</tr>
<tr>
<td>nu (float)</td>
<td>poisson ratio</td>
</tr>
<tr>
<td>e_init (float)</td>
<td>initial void ratio</td>
</tr>
<tr>
<td>Mc (float)</td>
<td>critical state stress ratio</td>
</tr>
<tr>
<td>c (float)</td>
<td>ratio of critical state stress ratio in extension and compression</td>
</tr>
<tr>
<td>lambda_c (float)</td>
<td>critical state line constant</td>
</tr>
<tr>
<td>e0 (float)</td>
<td>critical void ratio at p = 0</td>
</tr>
<tr>
<td>ksi (float)</td>
<td>critical state line constant</td>
</tr>
<tr>
<td>P_atm (float)</td>
<td>atmospheric pressure</td>
</tr>
<tr>
<td>m (float)</td>
<td>yield surface constant (radius of yield surface in stress ratio space)</td>
</tr>
<tr>
<td>h0 (float)</td>
<td>constant parameter</td>
</tr>
<tr>
<td>ch (float)</td>
<td>constant parameter</td>
</tr>
<tr>
<td>nb (float)</td>
<td>bounding surface parameter, ( nb \geq 0 )</td>
</tr>
<tr>
<td>A0 (float)</td>
<td>dilatancy parameter</td>
</tr>
<tr>
<td>nd (float)</td>
<td>dilatancy surface parameter ( nd \geq 0 )</td>
</tr>
<tr>
<td>z_max (float)</td>
<td>fabric-dilatancy tensor parameter</td>
</tr>
<tr>
<td>cz (float)</td>
<td>fabric-dilatancy tensor parameter</td>
</tr>
<tr>
<td>Den (float)</td>
<td>mass density of the material</td>
</tr>
</tbody>
</table>

The material formulations for the ManzariDafalias object are:

- 'ThreeDimensional'
- 'PlaneStrain'

See also [here](#)

**References**


**PM4Sand**

`nDMaterial` ("PM4Sand", matTag, Dr, G0, hpo, Den, patm, h0, emin, nb, nd, Ado, zmax, cz, ce, phic, nu, cgdr, cdr, ckaf, Q, R, m, Fsed_min, p_sedo)

This command is used to construct a 2-dimensional PM4Sand material.
| matTag (int) | integer tag identifying material |
| Dr (float) | Relative density, in fraction |
| G0 (float) | Shear modulus constant |
| hpc (float) | Contraction rate parameter |
| Den (float) | Mass density of the material |
| P_atm (float) | Optional, Atmospheric pressure |
| h0 (float) | Optional, Variable that adjusts the ratio of plastic modulus to elastic modulus |
| emax (float) | Optional, Maximum and minimum void ratios |
| emin (float) | Optional, Maximum and minimum void ratios |
| nb (float) | Optional, Bounding surface parameter, \( nb > 0 \) |
| nd (float) | Optional, Dilatancy surface parameter \( nd \geq 0 \) |
| Ado (float) | Optional, Dilatancy parameter, will be computed at the time of initialization if input value is negative |
| z_max (float) | Optional, Fabric-dilatancy tensor parameter |
| cz (float) | Optional, Fabric-dilatancy tensor parameter |
| ce (float) | Optional, Variable that adjusts the rate of strain accumulation in cyclic loading |
| phi_c (float) | Optional, Critical state effective friction angle |
| nu (float) | Optional, Poisson’s ratio |
| cgd (float) | Optional, Variable that adjusts degradation of elastic modulus with accumulation of fabric |
| cdr (float) | Optional, Variable that controls the rotated dilatancy surface |
| ckaf (float) | Optional, Variable that controls the effect that sustained static shear stresses have on plastic modulus |
| Q (float) | Optional, Critical state line parameter |
| R (float) | Optional, Critical state line parameter |
| m (float) | Optional, Yield surface constant (radius of yield surface in stress ratio space) |
| Fsed_min (float) | Optional, Variable that controls the minimum value the reduction factor of the elastic moduli can get during reconsolidation |
| p_sedo (float) | Optional, Mean effective stress up to which reconsolidation strains are enhanced |

The material formulations for the PM4Sand object are:

- 'PlaneStrain'

See also here

References


**StressDensityModel**

nDMaterial (‘StressDensityModel’, matTag, mDen, eNot, A, n, nu, a1, b1, a2, b2, a3, b3, fd, muNot, muCyc, sc, M, patm, ssl1, ssl2, ssl3, ssl4, ssl5, ssl6, ssl7, ssl8, ssl9, ssl10, hsl, p1, p2, p3, p4, p5, p6, p7, p8, p9, p10)

This command is used to construct a multi-dimensional stress density material object for modeling sand behaviour following the work of Cubrinovski and Ishihara (1998a,b).
Table 1 – continued from previous page

<table>
<thead>
<tr>
<th>mDen (float)</th>
<th>mass density</th>
</tr>
</thead>
<tbody>
<tr>
<td>eNot (float)</td>
<td>initial void ratio</td>
</tr>
<tr>
<td>A (float)</td>
<td>constant for elastic shear modulus</td>
</tr>
<tr>
<td>n (float)</td>
<td>pressure dependency exponent for elastic shear modulus</td>
</tr>
<tr>
<td>nu (float)</td>
<td>Poisson’s ratio</td>
</tr>
<tr>
<td>a1 (float)</td>
<td>peak stress ratio coefficient ($\eta_{max} = a1 + b1 * I_s$)</td>
</tr>
<tr>
<td>b1 (float)</td>
<td>peak stress ratio coefficient ($\eta_{max} = a1 + b1 * I_s$)</td>
</tr>
<tr>
<td>a2 (float)</td>
<td>max shear modulus coefficient ($G_{max} = a2 + b2 * I_s$)</td>
</tr>
<tr>
<td>b2 (float)</td>
<td>max shear modulus coefficient ($G_{max} = a2 + b2 * I_s$)</td>
</tr>
<tr>
<td>a3 (float)</td>
<td>min shear modulus coefficient ($G_{min} = a3 + b3 * I_s$)</td>
</tr>
<tr>
<td>b3 (float)</td>
<td>min shear modulus coefficient ($G_{min} = a3 + b3 * I_s$)</td>
</tr>
<tr>
<td>fd (float)</td>
<td>degradation constant</td>
</tr>
<tr>
<td>muNot (float)</td>
<td>dilatancy coefficient (monotonic loading)</td>
</tr>
<tr>
<td>muCyc (float)</td>
<td>dilatancy coefficient (cyclic loading)</td>
</tr>
<tr>
<td>sc (float)</td>
<td>dilatancy strain</td>
</tr>
<tr>
<td>M (float)</td>
<td>critical state stress ratio</td>
</tr>
<tr>
<td>patm (float)</td>
<td>atmospheric pressure (in appropriate units)</td>
</tr>
<tr>
<td>ssl1 (float)</td>
<td>void ratio of quasi steady state (QSS-line) at pressure p1 (default = 0.877)</td>
</tr>
<tr>
<td>ssl2 (float)</td>
<td>void ratio of quasi steady state (QSS-line) at pressure p2 (default = 0.877)</td>
</tr>
<tr>
<td>ssl3 (float)</td>
<td>void ratio of quasi steady state (QSS-line) at pressure p3 (default = 0.873)</td>
</tr>
<tr>
<td>ssl4 (float)</td>
<td>void ratio of quasi steady state (QSS-line) at pressure p4 (default = 0.870)</td>
</tr>
<tr>
<td>ssl5 (float)</td>
<td>void ratio of quasi steady state (QSS-line) at pressure p5 (default = 0.860)</td>
</tr>
<tr>
<td>ssl6 (float)</td>
<td>void ratio of quasi steady state (QSS-line) at pressure p6 (default = 0.850)</td>
</tr>
<tr>
<td>ssl7 (float)</td>
<td>void ratio of quasi steady state (QSS-line) at pressure p7 (default = 0.833)</td>
</tr>
<tr>
<td>ssl8 (float)</td>
<td>void ratio of quasi steady state (QSS-line) at pressure p8 (default = 0.833)</td>
</tr>
<tr>
<td>ssl9 (float)</td>
<td>void ratio of quasi steady state (QSS-line) at pressure p9 (default = 0.833)</td>
</tr>
<tr>
<td>ssl10 (float)</td>
<td>void ratio of quasi steady state (QSS-line) at pressure p10 (default = 0.833)</td>
</tr>
<tr>
<td>hsl1 (float)</td>
<td>void ratio of upper reference state (UR-line) for all pressures (default = 0.895)</td>
</tr>
<tr>
<td>p1 (float)</td>
<td>pressure corresponding to ssl1 (default = 1.0 kPa)</td>
</tr>
<tr>
<td>p2 (float)</td>
<td>pressure corresponding to ssl1 (default = 10.0 kPa)</td>
</tr>
<tr>
<td>p3 (float)</td>
<td>pressure corresponding to ssl1 (default = 30.0 kPa)</td>
</tr>
<tr>
<td>p4 (float)</td>
<td>pressure corresponding to ssl1 (default = 50.0 kPa)</td>
</tr>
<tr>
<td>p5 (float)</td>
<td>pressure corresponding to ssl1 (default = 100.0 kPa)</td>
</tr>
<tr>
<td>p6 (float)</td>
<td>pressure corresponding to ssl1 (default = 200.0 kPa)</td>
</tr>
<tr>
<td>p7 (float)</td>
<td>pressure corresponding to ssl1 (default = 400.0 kPa)</td>
</tr>
<tr>
<td>p8 (float)</td>
<td>pressure corresponding to ssl1 (default = 400.0 kPa)</td>
</tr>
<tr>
<td>p9 (float)</td>
<td>pressure corresponding to ssl1 (default = 400.0 kPa)</td>
</tr>
<tr>
<td>p10 (float)</td>
<td>pressure corresponding to ssl1 (default = 400.0 kPa)</td>
</tr>
</tbody>
</table>

The material formulations for the StressDensityModel object are:

- 'ThreeDimensional'
- 'PlaneStrain'

References


**AcousticMedium**

**nDMaterial** ('AcousticMedium', matTag, K, rho)
This command is used to construct an acoustic medium NDMaterial object.

<table>
<thead>
<tr>
<th>matTag (int)</th>
<th>integer tag identifying material</th>
</tr>
</thead>
<tbody>
<tr>
<td>K (float)</td>
<td>bulk module of the acoustic medium</td>
</tr>
<tr>
<td>rho (float)</td>
<td>mass density of the acoustic medium</td>
</tr>
</tbody>
</table>

**Tsinghua Sand Models**

1. **CycLiqCP**
2. **CycLiqCPSP**

**CycLiqCP**

**nDMaterial** ('CycLiqCP', matTag, G0, kappa, h, Mfc, dre1, Mdc, dre2, rdr, alpha, dir, ein, rho)
This command is used to construct a multi-dimensional material object that follows the constitutive behavior of a cyclic elastoplasticity model for large post-liquefaction deformation.

CycLiqCP material is a cyclic elastoplasticity model for large post-liquefaction deformation, and is implemented using a cutting plane algorithm. The model is capable of reproducing small to large deformation in the pre- to post-liquefaction regime. The elastic moduli of the model are pressure dependent. The plasticity in the model is developed within the framework of bounding surface plasticity, with special consideration to the formulation of reversible and irreversible dilatancy.

The model does not take into consideration of the state of sand, and requires different parameters for sand under different densities and confining pressures. The surfaces (i.e. failure and maximum pre-stress) are considered as circles in the pi plane.

The model has been validated against VELACS centrifuge model tests and has used on numerous simulations of liquefaction related problems.

When this material is employed in regular solid elements (e.g., FourNodeQuad, Brick), it simulates drained soil response. When solid-fluid coupled elements (u-p elements and SSP u-p elements) are used, the model is able to simulate undrained and partially drained behavior of soil.

<table>
<thead>
<tr>
<th>matTag (int)</th>
<th>integer tag identifying material</th>
</tr>
</thead>
<tbody>
<tr>
<td>G0 (float)</td>
<td>A constant related to elastic shear modulus</td>
</tr>
<tr>
<td>kappa (float)</td>
<td>bulk modulus</td>
</tr>
<tr>
<td>h (float)</td>
<td>Model parameter for plastic modulus</td>
</tr>
<tr>
<td>Mfc (float)</td>
<td>Stress ratio at failure in triaxial compression</td>
</tr>
<tr>
<td>dre1 (float)</td>
<td>Coefficient for reversible dilatancy generation</td>
</tr>
<tr>
<td>Mdc (float)</td>
<td>Stress ratio at which the reversible dilatancy sign changes</td>
</tr>
<tr>
<td>dre2 (float)</td>
<td>Coefficient for reversible dilatancy release</td>
</tr>
<tr>
<td>rdr (float)</td>
<td>Reference shear strain length</td>
</tr>
<tr>
<td>alpha (float)</td>
<td>Parameter controlling the decrease rate of irreversible dilatancy</td>
</tr>
<tr>
<td>dir (float)</td>
<td>Coefficient for irreversible dilatancy potential</td>
</tr>
<tr>
<td>ein (float)</td>
<td>Initial void ratio</td>
</tr>
<tr>
<td>rho (float)</td>
<td>Saturated mass density</td>
</tr>
</tbody>
</table>

The material formulations for the CycLiqCP object are:
This command is used to construct a multi-dimensional material object that follows the constitutive behavior of a cyclic elastoplasticity model for large post-liquefaction deformation.

CycLiqCPSP material is a constitutive model for sand with special considerations for cyclic behavior and accumulation of large post-liquefaction shear deformation, and is implemented using a cutting plane algorithm. The model: (1) achieves the simulation of post-liquefaction shear deformation based on its physics, allowing the unified description of pre- and post-liquefaction behavior of sand; (2) directly links the cyclic mobility of sand with reversible and irreversible dilatancy, enabling the unified description of monotonic and cyclic loading; (3) introduces critical state soil mechanics concepts to achieve unified modelling of sand under different states.

The critical, maximum stress ratio and reversible dilatancy surfaces follow a rounded triangle in the pi plane similar to the Matsuoka-Nakai criterion.

When this material is employed in regular solid elements (e.g., FourNodeQuad, Brick), it simulates drained soil response. When solid-fluid coupled elements (u-p elements and SSP u-p elements) are used, the model is able to simulate undrained and partially drained behavior of soil.

<table>
<thead>
<tr>
<th>Parameter</th>
<th>Description</th>
</tr>
</thead>
<tbody>
<tr>
<td>matTag (int)</td>
<td>integer tag identifying material</td>
</tr>
<tr>
<td>G0 (float)</td>
<td>A constant related to elastic shear modulus</td>
</tr>
<tr>
<td>kappa (float)</td>
<td>bulk modulus</td>
</tr>
<tr>
<td>h (float)</td>
<td>Model parameter for plastic modulus</td>
</tr>
<tr>
<td>M (float)</td>
<td>Critical state stress ratio</td>
</tr>
<tr>
<td>dre1 (float)</td>
<td>Coefficient for reversible dilatancy generation</td>
</tr>
<tr>
<td>dre2 (float)</td>
<td>Coefficient for reversible dilatancy release</td>
</tr>
<tr>
<td>rdr (float)</td>
<td>Reference shear strain length</td>
</tr>
<tr>
<td>alpha (float)</td>
<td>Parameter controlling the decrease rate of irreversible dilatancy</td>
</tr>
<tr>
<td>dir (float)</td>
<td>Coefficient for irreversible dilatancy potential</td>
</tr>
<tr>
<td>lambdac (float)</td>
<td>Critical state constant</td>
</tr>
<tr>
<td>ksi (float)</td>
<td>Critical state constant</td>
</tr>
<tr>
<td>e0 (float)</td>
<td>Void ratio at pc=0</td>
</tr>
<tr>
<td>np (float)</td>
<td>Material constant for peak mobilized stress ratio</td>
</tr>
<tr>
<td>nd (float)</td>
<td>Material constant for reversible dilatancy generation stress ratio</td>
</tr>
<tr>
<td>ein (float)</td>
<td>Initial void ratio</td>
</tr>
<tr>
<td>rho (float)</td>
<td>Saturated mass density</td>
</tr>
</tbody>
</table>

The material formulations for the CycLiqCP object are:

- 'ThreeDimensional'
- 'PlaneStrain'

See also here

Materials for Modeling Concrete Walls

1. *PlaneStressUserMaterial*
2. *PlateFromPlaneStress*
3. *PlateRebar*

**PlaneStressUserMaterial**

\begin{verbatim}
 nDMaterial ('PlaneStressUserMaterial', matTag, fc, ft, fcu, epsc0, epscu, epsstu, stc)
\end{verbatim}

This command is used to create the multi-dimensional concrete material model that is based on the damage mechanism and smeared crack model.

<table>
<thead>
<tr>
<th>Parameter</th>
<th>Description</th>
</tr>
</thead>
<tbody>
<tr>
<td>matTag</td>
<td>integer tag identifying material</td>
</tr>
<tr>
<td>fc</td>
<td>concrete compressive strength at 28 days (positive)</td>
</tr>
<tr>
<td>ft</td>
<td>concrete tensile strength (positive)</td>
</tr>
<tr>
<td>fcu</td>
<td>concrete crushing strength (negative)</td>
</tr>
<tr>
<td>epsc0</td>
<td>concrete strain at maximum strength (negative)</td>
</tr>
<tr>
<td>epscu</td>
<td>concrete strain at crushing strength (negative)</td>
</tr>
<tr>
<td>epsstu</td>
<td>ultimate tensile strain (positive)</td>
</tr>
<tr>
<td>stc</td>
<td>shear retention factor</td>
</tr>
</tbody>
</table>

**PlateFromPlaneStress**

\begin{verbatim}
 nDMaterial ('PlateFromPlaneStress', matTag, newmatTag, OutofPlaneModulus)
\end{verbatim}

This command is used to create the multi-dimensional concrete material model that is based on the damage mechanism and smeared crack model.

<table>
<thead>
<tr>
<th>Parameter</th>
<th>Description</th>
</tr>
</thead>
<tbody>
<tr>
<td>matTag</td>
<td>integer tag identifying material</td>
</tr>
<tr>
<td>newmatTag</td>
<td>new integer tag identifying material deriving from pre-defined PlaneStressUserMaterial</td>
</tr>
<tr>
<td>OutofPlaneModulus</td>
<td>shear modulus of out plane</td>
</tr>
</tbody>
</table>

**PlateRebar**

\begin{verbatim}
 nDMaterial ('PlateRebar', matTag, newmatTag, sita)
\end{verbatim}

This command is used to create the multi-dimensional reinforcement material.

<table>
<thead>
<tr>
<th>Parameter</th>
<th>Description</th>
</tr>
</thead>
<tbody>
<tr>
<td>matTag</td>
<td>integer tag identifying material</td>
</tr>
<tr>
<td>newmatTag</td>
<td>new integer tag identifying material deriving from pre-defined uniaxial steel material</td>
</tr>
<tr>
<td>sita</td>
<td>define the angle of steel layer, 90 (longitudinal steel), 0 (tranverse steel)</td>
</tr>
</tbody>
</table>

**Contact Materials for 2D and 3D**

1. *ContactMaterial2D*
2. **ContactMaterial3D**

**ContactMaterial2D**

`nDMaterial` (`'ContactMaterial2D', matTag, mu, G, c, t`)

This command is used to construct a ContactMaterial2D nDMaterial object.

<table>
<thead>
<tr>
<th>matTag (int)</th>
<th>integer tag identifying material</th>
</tr>
</thead>
<tbody>
<tr>
<td>mu (float)</td>
<td>interface frictional coefficient</td>
</tr>
<tr>
<td>G (float)</td>
<td>interface stiffness parameter</td>
</tr>
<tr>
<td>c (float)</td>
<td>interface cohesive intercept</td>
</tr>
<tr>
<td>t (float)</td>
<td>interface tensile strength</td>
</tr>
</tbody>
</table>

The ContactMaterial2D nDMaterial defines the constitutive behavior of a frictional interface between two bodies in contact. The interface defined by this material object allows for sticking, frictional slip, and separation between the two bodies in a two-dimensional analysis. A regularized Coulomb frictional law is assumed. Information on the theory behind this material can be found in, e.g. Wriggers (2002).

**Note:**

1. The ContactMaterial2D nDMaterial has been written to work with the SimpleContact2D and BeamContact2D element objects.
2. There are no valid recorder queries for this material other than those which are listed with those elements.

**References:**


**ContactMaterial3D**

`nDMaterial` (`'ContactMaterial3D', matTag, mu, G, c, t`)

This command is used to construct a ContactMaterial3D nDMaterial object.

<table>
<thead>
<tr>
<th>matTag (int)</th>
<th>integer tag identifying material</th>
</tr>
</thead>
<tbody>
<tr>
<td>mu (float)</td>
<td>interface frictional coefficient</td>
</tr>
<tr>
<td>G (float)</td>
<td>interface stiffness parameter</td>
</tr>
<tr>
<td>c (float)</td>
<td>interface cohesive intercept</td>
</tr>
<tr>
<td>t (float)</td>
<td>interface tensile strength</td>
</tr>
</tbody>
</table>

The ContactMaterial3D nDMaterial defines the constitutive behavior of a frictional interface between two bodies in contact. The interface defined by this material object allows for sticking, frictional slip, and separation between the two bodies in a three-dimensional analysis. A regularized Coulomb frictional law is assumed. Information on the theory behind this material can be found in, e.g. Wriggers (2002).

**Note:**

1. The ContactMaterial3D nDMaterial has been written to work with the SimpleContact3D and BeamContact3D element objects.
2. There are no valid recorder queries for this material other than those which are listed with those elements.
References:

Wrapper material for Initial State Analysis

1. InitialStateAnalysisWrapper

InitialStateAnalysisWrapper

nDMaterial ('InitialStateAnalysisWrapper', matTag, nDMatTag, nDim)

The InitialStateAnalysisWrapper nDMaterial allows for the use of the InitialStateAnalysis command for setting initial conditions. The InitialStateAnalysisWrapper can be used with any nDMaterial. This material wrapper allows for the development of an initial stress field while maintaining the original geometry of the problem. An example analysis is provided below to demonstrate the use of this material wrapper object.

<table>
<thead>
<tr>
<th>matTag (int)</th>
<th>integer tag identifying material</th>
</tr>
</thead>
<tbody>
<tr>
<td>nDMatTag (int)</td>
<td>the tag of the associated nDMaterial object</td>
</tr>
<tr>
<td>nDim (int)</td>
<td>number of dimensions (2 for 2D, 3 for 3D)</td>
</tr>
</tbody>
</table>

Note:
1. There are no valid recorder queries for the InitialStateAnalysisWrapper.
2. The InitialStateAnalysis off command removes all previously defined recorders. Two sets of recorders are needed if the results before and after this command are desired. See the example below for more.
3. The InitialStateAnalysisWrapper material is somewhat tricky to use in dynamic analysis. Sometimes setting the displacement to zero appears to be interpreted as an initial displacement in subsequent steps, resulting in undesirable vibrations.

UC San Diego soil models

1. PressureIndependMultiYield
2. PressureDependMultiYield
3. PressureDependMultiYield02

PressureIndependMultiYield

nDMaterial ('PressureIndependMultiYield', matTag, nd, rho, refShearModul, refBulkModul, cohesi, peakShearStr, frictionAng=0., refPress=100., pressDependCoe=0., noYieldSurf=20. , *yieldSurf)

PressureIndependMultiYield material is an elastic-plastic material in which plasticity exhibits only in the deviatoric stress-strain response. The volumetric stress-strain response is linear-elastic and is independent of the deviatoric response. This material is implemented to simulate monotonic or cyclic response of materials whose shear behavior is insensitive to the confinement change. Such materials include, for example, organic soils or clay under fast (undrained) loading conditions.
<table>
<thead>
<tr>
<th>matTag</th>
<th>integer tag identifying material</th>
</tr>
</thead>
<tbody>
<tr>
<td>nd</td>
<td>Number of dimensions, 2 for plane-strain, and 3 for 3D analysis.</td>
</tr>
<tr>
<td>rho</td>
<td>Saturated soil mass density.</td>
</tr>
<tr>
<td>refShearModul</td>
<td>Reference low-strain shear modulus, specified at a reference mean effective confining pressure refPress of p'r (see below).</td>
</tr>
<tr>
<td>refBulkModul</td>
<td>Reference bulk modulus, specified at a reference mean effective confining pressure refPress of p'r (see below).</td>
</tr>
<tr>
<td>cohesi</td>
<td>Apparent cohesion at zero effective confinement.</td>
</tr>
<tr>
<td>peakShearStra</td>
<td>An octahedral shear strain at which the maximum shear strength is reached, specified at a reference mean effective confining pressure refPress of p'r (see below).</td>
</tr>
<tr>
<td>frictionAng</td>
<td>Friction angle at peak shear strength in degrees, optional (default is 0.0).</td>
</tr>
<tr>
<td>refPress</td>
<td>Reference mean effective confining pressure at which ( G_r ), ( B_r ), and ( \gamma_{\max} ) are defined, optional (default is 100. kPa).</td>
</tr>
</tbody>
</table>
| pressDependCoe | A positive constant defining variations of \( G \) and \( B \) as a function of instantaneous effective confinement \( p' \) (default is 0.0) \[
G = G_r \left( \frac{p'}{p'_r} \right)^d \\
B = B_r \left( \frac{p'}{p'_r} \right)^d \\
\text{If } \phi = 0, \text{ } d \text{ is reset to 0.0.}
\]
| noYieldSurf | Number of yield surfaces, optional (must be less than 40, default is 20). The surfaces are generated based on the hyperbolic relation defined in Note 2 below. |
| yieldSurf | Instead of automatic surfaces generation (Note 2), you can define yield surfaces directly based on desired shear modulus reduction curve. To do so, add a minus sign in front of noYieldSurf, then provide noYieldSurf pairs of shear strain (r) and modulus ratio (Gs) values. For example, to define 10 surfaces: yieldSurf = [r1, Gs1, \ldots, r10, Gs10] |

See also notes

**PressureDependMultiYield**

```python
nDMaterial('PressureDependMultiYield', matTag, nd, rho, refShearModul, refBulkModul, frictionAng, peakShearStra, refPress, pressDependCoe, PTAng, contrac, *dilat, *liquefac, noYieldSurf=20.0, *yieldSurf=[], e=0.6, *params=[0.9, 0.02, 0.7, 101.0], c=0.3)
```

PressureDependMultiYield material is an elastic-plastic material for simulating the essential response characteristics of pressure sensitive soil materials under general loading conditions. Such characteristics include dilatancy (shear-induced volume contraction or dilation) and non-flow liquefaction (cyclic mobility), typically exhibited in sands or silts during monotonic or cyclic loading.
---

matTag (int)
integer tag identifying material

nd (float)
Number of dimensions, 2 for plane-strain, and 3 for 3D analysis.

rho (float)
Saturated soil mass density.

refShearModul (float) \((G_\text{r})\)
Reference low-strain shear modulus, specified at a reference mean effective confining pressure \(p'_r\) (see below).

refBulkModul (float) \((B_\text{r})\)
Reference bulk modulus, specified at a reference mean effective confining pressure \(p'_r\) (see below).

frictionAng (float) \((\phi)\)
Friction angle at peak shear strength in degrees, optional (default is 0.0).

peakShearStra (float) \((\gamma_{\text{max}})\)
An octahedral shear strain at which the maximum shear strength is reached, specified at a reference mean effective confining pressure \(p'_r\) (see below).

refPress (float) \((p'_r)\)
Reference mean effective confining pressure at which \(G_\text{r}, B_\text{r}, \text{and} \gamma_{\text{max}}\) are defined, optional (default is 100. kPa).

pressDependCoe (float) \((d)\)
A positive constant defining variations of \(G\) and \(B\) as a function of instantaneous effective confinement \(p'_r\) (default is 0.0)

\[
G = G_\text{r}\left(\frac{p'}{p'_r}\right)^d \\
B = B_\text{r}\left(\frac{p'}{p'_r}\right)^d
\]

If \(\phi = 0\), \(d\) is reset to 0.0.

PTAng (float) \((\phi_{PT})\)
Phase transformation angle, in degrees.

contrac (float)
A non-negative constant defining the rate of shear-induced volume decrease (contraction) or pore pressure buildup. A larger value corresponds to faster contraction rate.

dilat (list (float))
Non-negative constants defining the rate of shear-induced volume increase (dilation). Larger values correspond to stronger dilation rate. \(\text{dilat} = [\text{dilat1}, \text{dilat2}]\).

liquefac (list (float))
Parameters controlling the mechanism of liquefaction-induced perfectly plastic shear strain accumulation, i.e., cyclic mobility. Set \(\text{ liquefac}[0] = 0\) to deactivate this mechanism altogether.

\(\text{ liquefac}[0]\) defines the effective confining pressure (e.g., 10 kPa in SI units or 1.45 psi in English units) below which the mechanism is in effect. Smaller values should be assigned to denser sands.

\(\text{ liquefac}[1]\) defines the maximum amount of perfectly plastic shear strain developed at zero effective confinement during each loading phase. Smaller values should be assigned to denser sands.

\(\text{ liquefac}[2]\) defines the maximum amount of biased perfectly plastic shear strain \(\gamma_b\) accumulated at each loading phase under biased shear loading conditions, as \(\gamma_b = \text{ liquefac}[1] \times \text{ liquefac}[2]\). Typically, \(\text{ liquefac}[2]\) takes a value between 0.0 and 3.0. Smaller values should be assigned to denser sands. See the references listed at the end of this chapter for more information.

noYieldSurf (float)
Number of yield surfaces, optional (must be less than 40, default is 20). The surfaces are generated based on the hyperbolic relation defined in Note 2 below.

yieldSurf (list (float))
If \(\text{noYieldSurf} < 0 \&\ & > -100\), the user defined yield surface is used. You have to provide a list of \(2 \times (-\text{noYieldSurf})\), otherwise, the arguments will be messed up. Also don’t provide user defined yield surface if \(\text{noYieldSurf} > 0\), it will mess up the argument list too. Instead of automatic surfaces generation (Note 2), you can define yield surfaces directly based on desired shear modulus reduction curve. To do so, add a minus sign in front of \(\text{noYieldSurf}\), then provide \(\text{noYieldSurf}\) pairs of shear strain \((r)\) and modulus ratio \((G_s)\) values. For example, to define 10 surfaces: \(\text{yieldSurf} = [r1, Gs1, \ldots, r10, Gs10]\)

e (float)
Initial void ratio, optional (default is 0.6).

params=[cs1, cs2, cs3, pa]
Defining a straight critical-state line \(ec\) in \(e-p'\) space.

\[
\text{if cs3}=0, \\
ec = \text{cs1} - \text{cs2} \log(p'/pa) \\
\text{else (Li and Wang, JGGE, 124(12)),} \\
ec = \text{cs1} - \text{cs2}(p'/pa)^{{cs3}}
\]

where \(pa\) is atmospheric pressure for normalization (typically 101 kPa in SI units and 14.65 psi in English units). All four constants are optional

c (float)
Numerical constant (default value = 0.3 kPa)
---
See also notes

**PressureDependMultiYield02**

**nDMaterial** (`'PressureDependMultiYield02', matTag, nd, rho, refShearModul, refBulkModul, frictionAng, peakShearStra, refPress, pressDependCoe, PTAng, contrac[0], contrac[2], dilat[0], dilat[2], noYieldSurf=20.0, *yieldSurf=[], contrac[1]=5.0, dilat[1]=3.0, *liquefac=[1.0,0.0],e=0.6, *params=[0.9, 0.02, 0.7, 101.0], c=0.1)

PressureDependMultiYield02 material is modified from PressureDependMultiYield material, with:

1. additional parameters (contrac[2] and dilat[2]) to account for $K_\sigma$ effect,
2. a parameter to account for the influence of previous dilation history on subsequent contraction phase (contrac[1]), and
3. modified logic related to permanent shear strain accumulation (liquefac[0] and liquefac[1]).

<table>
<thead>
<tr>
<th>matTag (int)</th>
<th>integer tag identifying material</th>
</tr>
</thead>
<tbody>
<tr>
<td>contrac[1] (float)</td>
<td>A non-negative constant reflecting dilation history on contraction tendency.</td>
</tr>
<tr>
<td>liquefac[0] (float)</td>
<td>Damage parameter to define accumulated permanent shear strain as a function of dilation history. (Redefined and different from PressureDependMultiYield material).</td>
</tr>
<tr>
<td>liquefac[1] (float)</td>
<td>Damage parameter to define biased accumulation of permanent shear strain as a function of load reversal history. (Redefined and different from PressureDependMultiYield material).</td>
</tr>
<tr>
<td>c (float)</td>
<td>Numerical constant (default value = 0.1 kPa)</td>
</tr>
</tbody>
</table>

See also notes

**UC San Diego Saturated Undrained soil**

1. **FluidSolidPorousMaterial**

**FluidSolidPorousMaterial**

**nDMaterial** (`'FluidSolidPorousMaterial', matTag, nd, soilMatTag, combinedBulkModul, pa=101.0)

FluidSolidPorousMaterial couples the responses of two phases: fluid and solid. The fluid phase response is only volumetric and linear elastic. The solid phase can be any NDMaterial. This material is developed to simulate the response of saturated porous media under fully undrained condition.
<table>
<thead>
<tr>
<th>matTag (int)</th>
<th>integer tag identifying material</th>
</tr>
</thead>
<tbody>
<tr>
<td>nd (float)</td>
<td>Number of dimensions, 2 for plane-strain, and 3 for 3D analysis.</td>
</tr>
<tr>
<td>soilMatTag (int)</td>
<td>The material number for the solid phase material (previously defined).</td>
</tr>
<tr>
<td>combinBulkModul (float)</td>
<td>Computed undrained bulk modulus $B_c$ relating changes in pore pressure and volumetric strain, may be approximated by: $B_c \approx B_f/n$ where $B_f$ is the bulk modulus of fluid phase (2.2x106 kPa (or 3.191x105 psi) for water), and $n$ the initial porosity.</td>
</tr>
<tr>
<td>pa (float)</td>
<td>Optional atmospheric pressure for normalization (typically 101 kPa in SI units, or 14.65 psi in English units)</td>
</tr>
</tbody>
</table>

See also notes

### 1.4.15 section commands

**section** *(secType, secTag, *secArgs)*

This command is used to construct a SectionForceDeformation object, referred to as Section, which represents force-deformation (or resultant stress-strain) relationships at beam-column and plate sample points.

<table>
<thead>
<tr>
<th>secType (str)</th>
<th>section type</th>
</tr>
</thead>
<tbody>
<tr>
<td>secTag (int)</td>
<td>section tag.</td>
</tr>
<tr>
<td>secArgs (list)</td>
<td>a list of section arguments, must be preceded with *.</td>
</tr>
</tbody>
</table>

For example,

```python
secType = 'Elastic'
secTag = 1
secArgs = [E, A, Iz]
section(secType, secTag, *secArgs)
```

The following contain information about available secType:

1. **Elastic Section**
2. **Fiber Section**
3. **NDFiber Section**
4. **Wide Flange Section**
5. **RC Section**
6. **RCCircular Section**
7. **Parallel Section**
8. **Section Aggregator**
9. **Uniaxial Section**
10. **Elastic Membrane Plate Section**
11. **Plate Fiber Section**
12. **Bidirectional Section**
13. *Isolator2spring Section*
14. *LayeredShell*

**Elastic Section**

```python
section ('Elastic', secTag, E, A, Iz, G=0.0, alphaY=0.0)
```

This command allows the user to construct an ElasticSection. The inclusion of shear deformations is optional. The dofs for 2D elastic section are \([P, M_z]\), for 3D are \([P, M_z, M_y, T]\).

<table>
<thead>
<tr>
<th>secTag (int)</th>
<th>unique section tag</th>
</tr>
</thead>
<tbody>
<tr>
<td>E (float)</td>
<td>Young's Modulus</td>
</tr>
<tr>
<td>A (float)</td>
<td>cross-sectional area of section</td>
</tr>
<tr>
<td>Iz (float)</td>
<td>second moment of area about the local z-axis</td>
</tr>
<tr>
<td>G (float)</td>
<td>Shear Modulus (optional for 2D analysis, required for 3D analysis)</td>
</tr>
<tr>
<td>J (float)</td>
<td>torsional moment of inertia of section (required for 3D analysis)</td>
</tr>
<tr>
<td>alphaY (float)</td>
<td>shear shape factor along the local y-axis (optional)</td>
</tr>
<tr>
<td>alphaZ (float)</td>
<td>shear shape factor along the local z-axis (optional)</td>
</tr>
</tbody>
</table>

**Note:** The elastic section can be used in the nonlinear beam column elements, which is useful in the initial stages of developing a complex model.

**Fiber Section**

```python
section ('Fiber', secTag, '-GJ', GJ=0.0)
```

This command allows the user to construct a FiberSection object. Each FiberSection object is composed of Fibers, with each fiber containing a UniaxialMaterial, an area and a location \((y,z)\). The dofs for 2D section are \([P, M_z]\), for 3D are \([P, M_z, M_y, T]\).

<table>
<thead>
<tr>
<th>secTag (int)</th>
<th>unique section tag</th>
</tr>
</thead>
<tbody>
<tr>
<td>GJ (float)</td>
<td>linear-elastic torsional stiffness assigned to the section (optional)</td>
</tr>
</tbody>
</table>

**FiberThermal**

```python
section ('FiberThermal', secTag, '-GJ', GJ=0.0)
```

This command creates a FiberSectionThermal object. The dofs for 2D section are \([P, M_z]\), for 3D are \([P, M_z, M_y]\).

**Note:**
1. The commands below should be called after the section command to generate all the fibers in the section.
2. The patch and layer commands can be used to generate multiple fibers in a single command.

**Fiber Command**

```python
fiber (yloc, zloc, A, matTag)
```

This command allows the user to construct a single fiber and add it to the enclosing FiberSection or NDFiberSection.
### Patch Command

**patch (type, *args)**

The patch command is used to generate a number of fibers over a cross-sectional area. Currently there are three types of cross-section that fibers can be generated: quadrilateral, rectangular and circular.


This is the command to generate a quadrilateral shaped patch (the geometry of the patch is defined by four vertices: I J K L. The coordinates of each of the four vertices is specified in COUNTER CLOCKWISE sequence)

| matTag (int) | material tag associated with this fiber (UniaxialMaterial tag for a FiberSection and NDMaterial tag for use in an NDFiberSection). |
| numSubdivIJ (int) | number of subdivisions (fibers) in the IJ direction. |
| numSubdivJK (int) | number of subdivisions (fibers) in the JK direction. |
| crdsI (list (float)) | y & z-coordinates of vertex I (local coordinate system) |
| crdsJ (list (float)) | y & z-coordinates of vertex J (local coordinate system) |
| crdsK (list (float)) | y & z-coordinates of vertex K (local coordinate system) |
| crdsL (list (float)) | y & z-coordinates of vertex L (local coordinate system) |


This is the command to generate a rectangular patch. The geometry of the patch is defined by coordinates of vertices: I and J. The first vertex, I, is the bottom-left point and the second vertex, J, is the top-right point, having as a reference the local y-z plane.

| matTag (int) | material tag associated with this fiber (UniaxialMaterial tag for a FiberSection and NDMaterial tag for use in an NDFiberSection). |
| numSubdivY (int) | number of subdivisions (fibers) in local y direction. |
| numSubdivZ (int) | number of subdivisions (fibers) in local z direction. |
| crdsI (list (float)) | y & z-coordinates of vertex I (local coordinate system) |
| crdsJ (list (float)) | y & z-coordinates of vertex J (local coordinate system) |


This is the command to generate a circular shaped patch.

| matTag (int) | material tag associated with this fiber (UniaxialMaterial tag for a FiberSection and NDMaterial tag for use in an NDFiberSection). |
| numSubdivCirc | number of subdivisions (fibers) in the circumference. |
| numSubdivRad | number of subdivisions (fibers) in the radial direction. |
| crdsI (list (float)) | y & z-coordinates of vertex I (local coordinate system) |
| crdsJ (list (float)) | y & z-coordinates of vertex J (local coordinate system) |
Layer Command

layer (type, *args)
The layer command is used to generate a number of fibers along a line or a circular arc.

layer ('straight', matTag, numFiber, areaFiber, *start, *end)
This command is used to construct a straight line of fibers

layer ('circ', matTag,numFiber,areaFiber,*center,radius,*ang=[0.0,360.0-360/numFiber])
This command is used to construct a line of fibers along a circular arc

NDFiber Section

section ('NDFiber', secTag)
This command allows the user to construct an NDFiberSection object. Each NDFiberSection object is composed of NDFibers, with each fiber containing an NDMaterial, an area and a location (y,z). The NDFiberSection
works for 2D and 3D frame elements and it queries the NDMaterial of each fiber for its axial and shear stresses. In 2D, stress components 11 and 12 are obtained from each fiber in order to provide stress resultants for axial force, bending moment, and shear \([P, M_z, V_y]\). Stress components 11, 12, and 13 lead to all six stress resultants in 3D \([P, M_z, V_y, M_y, V_z, T]\).

The NDFiberSection works with any NDMaterial via wrapper classes that perform static condensation of the stress vector down to the 11, 12, and 13 components, or via concrete NDMaterial subclasses that implement the appropriate fiber stress conditions.

| `secTag` (int) | unique section tag |

Note:
1. The commands below should be called after the section command to generate all the fibers in the section.
2. The patch and layer commands can be used to generate multiple fibers in a single command.

1. `fiber()`
2. `patch()`
3. `layer()`

**Wide Flange Section**

`section` (`'WFSection2d'`, `secTag`, `matTag`, `d`, `tw`, `bf`, `tf`, `Nfw`, `Nff`)

This command allows the user to construct a WFSection2d object, which is an encapsulated fiber representation of a wide flange steel section appropriate for plane frame analysis.

| `secTag` (int) | unique section tag |
| `matTag` (int) | tag of uniaxialMaterial assigned to each fiber |
| `d` (float) | section depth |
| `tw` (float) | web thickness |
| `bf` (float) | flange width |
| `tf` (float) | flange thickness |
| `Nfw` (float) | number of fibers in the web |
| `Nff` (float) | number of fibers in each flange |

Note: The section dimensions `d`, `tw`, `bf`, and `tf` can be found in the AISC steel manual.

**RC Section**

`section` (`'RCSection2d'`, `secTag`, `coreTag`, `coverTag`, `steelTag`, `d`, `b`, `cover`, `Atop`, `Abot`, `Aside`, `Nfcore`, `Nfcover`, `Nfs`)

This command allows the user to construct an RCSection2d object, which is an encapsulated fiber representation of a rectangular reinforced concrete section with core and confined regions of concrete and single top and bottom layers of reinforcement appropriate for plane frame analysis.
secTag (int) | unique section tag
---|---
coreTag (int) | tag of uniaxialMaterial assigned to each fiber in the core region
coverTag (int) | tag of uniaxialMaterial assigned to each fiber in the cover region
steelTag (int) | tag of uniaxialMaterial assigned to each reinforcing bar
d (float) | section depth
b (float) | section width
cover (float) | cover depth (assumed uniform around perimeter)
Atop (float) | area of reinforcing bars in top layer
Abot (float) | area of reinforcing bars in bottom layer
Aside (float) | area of reinforcing bars on intermediate layers
Nfcore (float) | number of fibers through the core depth
Nfcover (float) | number of fibers through the cover depth
Nfs (float) | number of bars on the top and bottom rows of reinforcement (Nfs-2 bars will be placed on the side rows)

Note: For more general reinforced concrete section definitions, use the Fiber Section command.

**RCCircular Section**

`section (’RCCircularSection’, secTag, coreTag, coverTag, steelTag, d, cover, As, NringsCore, NringsCover, Newedges, Nsteel)`

This command allows the user to construct an RCCircularSection object, which is an encapsulated fiber representation of a circular reinforced concrete section with core and confined regions of concrete.

secTag (int) | unique section tag
coreTag (int) | tag of uniaxialMaterial assigned to each fiber in the core region
coverTag (int) | tag of uniaxialMaterial assigned to each fiber in the cover region
steelTag (int) | tag of uniaxialMaterial assigned to each reinforcing bar
d (float) | section radius
cover (float) | cover depth (assumed uniform around perimeter)
As (float) | area of reinforcing bars
NringsCore (int) | number of fibers through the core depth
NringsCover (int) | number of fibers through the cover depth
Newedges (int) | number of fibers through the edges
Nsteel (int) | number of fibers through the steels

Note: For more general reinforced concrete section definitions, use the Fiber Section command.

**Parallel Section**

`section (’Parallel’, secTag, *tags)`

Connect sections in parallel.
Section Aggregator

`section ('Aggregator', secTag, *mats, '-section', sectionTag)`

This command is used to construct a SectionAggregator object which aggregates groups previously-defined UniaxialMaterial objects into a single section force-deformation model. Each UniaxialMaterial object represents the section force-deformation response for a particular section degree-of-freedom (dof). There is no interaction between responses in different dof directions. The aggregation can include one previously defined section.

<table>
<thead>
<tr>
<th>secTag (int)</th>
<th>unique section tag</th>
</tr>
</thead>
<tbody>
<tr>
<td>tags (list (int))</td>
<td>tags of predefined sections.</td>
</tr>
</tbody>
</table>

<table>
<thead>
<tr>
<th>secTag (int)</th>
<th>unique section tag</th>
</tr>
</thead>
<tbody>
<tr>
<td>mats (list)</td>
<td>list of tags and dofs of previously-defined UniaxialMaterial objects, mats = [matTag1,dof1, matTag2,dof2,...] the force-deformation quantity to be modeled by this section object. One of the following section dof may be used:</td>
</tr>
<tr>
<td>sectionTag (int)</td>
<td>tag of previously-defined Section object to which the UniaxialMaterial objects are aggregated as additional force-deformation relationships (optional)</td>
</tr>
</tbody>
</table>

- 'P' Axial force-deformation
- 'Mz' Moment-curvature about section local z-axis
- 'Vy' Shear force-deformation along section local y-axis
- 'My' Moment-curvature about section local y-axis
- 'Vz' Shear force-deformation along section local z-axis
- 'T' Torsion Force-Deformation

Uniaxial Section

`section ('Uniaxial', secTag, matTag, quantity)`

This command is used to construct a UniaxialSection object which uses a previously-defined UniaxialMaterial object to represent a single section force-deformation response quantity.
Elastic Membrane Plate Section

section ('ElasticMembranePlateSection', secTag, E, nu, h, rho)
This command allows the user to construct an ElasticMembranePlateSection object, which is an isotropic section appropriate for plate and shell analysis.

<table>
<thead>
<tr>
<th>secTag (int)</th>
<th>unique section tag</th>
</tr>
</thead>
<tbody>
<tr>
<td>E (float)</td>
<td>Young’s Modulus</td>
</tr>
<tr>
<td>nu (float)</td>
<td>Poisson’s Ratio</td>
</tr>
<tr>
<td>h (float)</td>
<td>depth of section</td>
</tr>
<tr>
<td>rho (float)</td>
<td>mass density</td>
</tr>
</tbody>
</table>

Plate Fiber Section

section ('PlateFiber', secTag, matTag, h)
This command allows the user to construct a MembranePlateFiberSection object, which is a section that numerically integrates through the plate thickness with “fibers” and is appropriate for plate and shell analysis.

<table>
<thead>
<tr>
<th>secTag (int)</th>
<th>unique section tag</th>
</tr>
</thead>
<tbody>
<tr>
<td>matTag (int)</td>
<td>nDMaterial tag to be assigned to each fiber</td>
</tr>
<tr>
<td>h (float)</td>
<td>plate thickness</td>
</tr>
</tbody>
</table>

Bidirectional Section

section ('Bidirectional', secTag, E, Fy, Hiso, Hkin, code1='Vy', code2='P')
This command allows the user to construct a Bidirectional section, which is a stress-resultant plasticity model of two coupled forces. The yield surface is circular and there is combined isotropic and kinematic hardening.
secTag (int) unique section tag
E (float) elastic modulus
Fy (float) yield force
Hiso (float) isotropic hardening modulus
Hkin (float) kinematic hardening modulus
code1 (str) section force code for direction 1 (optional)
code2 (str) section force code for direction 2 (optional)

One of the following section code may be used:
- 'P' Axial force-deformation
- 'Mz' Moment-curvature about section local z-axis
- 'Vy' Shear force-deformation along section local y-axis
- 'My' Moment-curvature about section local y-axis
- 'Vz' Shear force-deformation along section local z-axis
- 'T' Torsion Force-Deformation

Isolator2spring Section

section ('Iso2spring', matTag, tol, k1, Fyo, k2o, kvo, hb, PE, Po=0.0)

This command is used to construct an Isolator2spring section object, which represents the buckling behavior of an elastomeric bearing for two-dimensional analysis in the lateral and vertical plane. An Isolator2spring section represents the resultant force-deformation behavior of the bearing, and should be used with a zeroLengthSection element. The bearing should be constrained against rotation.

| secTag (int) | unique section tag |
| tol (float) | tolerance for convergence of the element state. Suggested value: E-12 to E-10. OpenSees will warn if convergence is not achieved, however this usually does not prevent global convergence. |
| k1 (float) | initial stiffness for lateral force-deformation |
| Fyo (float) | nominal yield strength for lateral force-deformation |
| k2o (float) | nominal postyield stiffness for lateral force-deformation |
| kvo (float) | nominal stiffness in the vertical direction |
| hb (float) | total height of elastomeric bearing |
| PE (float) | Euler Buckling load for the bearing |
| Po (float) | axial load at which nominal yield strength is achieved (optional) |

LayeredShell

nDMaterial ('LayeredShell', sectionTag, nLayers *mats)

This command will create the section of the multi-layer shell element, including the multi-dimensional concrete.

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reinforcement material and the corresponding thickness.

<table>
<thead>
<tr>
<th>sectionTag (int)</th>
<th>unique tag among sections</th>
</tr>
</thead>
<tbody>
<tr>
<td>nLayers (int)</td>
<td>total numbers of layers</td>
</tr>
<tr>
<td>mats (list)</td>
<td>a list of material tags and thicknesses.</td>
</tr>
</tbody>
</table>

### 1.4.16 frictionModel commands

**frictionModel**(frnType, frnTag, *frnArgs)

The frictionModel command is used to construct a friction model object, which specifies the behavior of the coefficient of friction in terms of the absolute sliding velocity and the pressure on the contact area. The command has at least one argument, the friction model type.

**frnType** (str) frictionModel type
**frnTag** (int) frictionModel tag.
**frnArgs** (list) a list of frictionModel arguments, must be preceded with *.

For example,

```python
defination = 'Coulomb'
definationTag = 1
definationArgs = [mu]
frictionModel(frnType, frnTag, *frnArgs)
```

The following contain information about available **frnType**:

1. **Coulomb**
2. **Velocity Dependent Friction**
3. **Velocity and Normal Force Dependent Friction**
4. **Velocity and Pressure Dependent Friction**
5. **Multi-Linear Velocity Dependent Friction**

**Coulomb**

**frictionModel**(‘Coulomb’, frnTag, mu)

This command is used to construct a Coulomb friction model object. Coulomb’s Law of Friction states that kinetic friction is independent of the sliding velocity.

<table>
<thead>
<tr>
<th>frnTag (int)</th>
<th>unique friction model tag</th>
</tr>
</thead>
<tbody>
<tr>
<td>mu (float)</td>
<td>coefficient of friction</td>
</tr>
</tbody>
</table>

**Velocity Dependent Friction**

**frictionModel**(‘VelDependent’, frnTag, muSlow, muFast, transRate)

This command is used to construct a VelDependent friction model object. It is useful for modeling the behavior of PTFE or PTFE-like materials sliding on a stainless steel surface. For a detailed presentation on the velocity dependence of such interfaces please refer to Constantinou et al. (1999).
**Velocity and Normal Force Dependent Friction**

**frictionModel** (‘VelNormalFrcDep’, frnTag, aSlow, nSlow, aFast, nFast, alpha0, alpha1, alpha2, maxMuFact)

This command is used to construct a VelNormalFrcDep friction model object.

<table>
<thead>
<tr>
<th>Parameter</th>
<th>Description</th>
</tr>
</thead>
<tbody>
<tr>
<td><strong>frnTag (int)</strong></td>
<td>unique friction model tag</td>
</tr>
<tr>
<td><strong>aSlow (float)</strong></td>
<td>constant for coefficient of friction at low velocity</td>
</tr>
<tr>
<td><strong>nSlow (float)</strong></td>
<td>exponent for coefficient of friction at low velocity</td>
</tr>
<tr>
<td><strong>aFast (float)</strong></td>
<td>constant for coefficient of friction at high velocity</td>
</tr>
<tr>
<td><strong>nFast (float)</strong></td>
<td>exponent for coefficient of friction at high velocity</td>
</tr>
<tr>
<td><strong>alpha0 (float)</strong></td>
<td>constant rate parameter coefficient</td>
</tr>
<tr>
<td><strong>alpha1 (float)</strong></td>
<td>linear rate parameter coefficient</td>
</tr>
<tr>
<td><strong>alpha2 (float)</strong></td>
<td>quadratic rate parameter coefficient</td>
</tr>
<tr>
<td><strong>maxMuFact (float)</strong></td>
<td>factor for determining the maximum coefficient of friction. This value prevents the friction coefficient from exceeding an unrealistic maximum value when the normal force becomes very small. The maximum friction coefficient is determined from $\mu_{Fast}$, for example $\mu \leq maxMuFac \cdot Fast$.</td>
</tr>
</tbody>
</table>

\[ \mu = \mu_{fast} - (\mu_{fast} - \mu_{slow}) \cdot e^{-transRate \cdot |v|} \]

**REFERENCE:**


**Velocity and Pressure Dependent Friction**

**frictionModel** (‘VelPressureDep’, frnTag, muSlow, muFast0, A, deltaMu, alpha, transRate)

This command is used to construct a VelPressureDep friction model object.

<table>
<thead>
<tr>
<th>Parameter</th>
<th>Description</th>
</tr>
</thead>
<tbody>
<tr>
<td><strong>frnTag (int)</strong></td>
<td>unique friction model tag</td>
</tr>
<tr>
<td><strong>muSlow (float)</strong></td>
<td>coefficient of friction at low velocity</td>
</tr>
<tr>
<td><strong>muFast0 (float)</strong></td>
<td>coefficient of friction at high velocity</td>
</tr>
<tr>
<td><strong>A (float)</strong></td>
<td>factor for determining the maximum coefficient of friction. This value prevents the friction coefficient from exceeding an unrealistic maximum value when the normal force becomes very small. The maximum friction coefficient is determined from $\mu_{Fast}$, for example $\mu \leq maxMuFac \cdot Fast$.</td>
</tr>
<tr>
<td><strong>deltaMu (float)</strong></td>
<td>linear rate parameter coefficient</td>
</tr>
<tr>
<td><strong>alpha (float)</strong></td>
<td>quadratic rate parameter coefficient</td>
</tr>
<tr>
<td><strong>transRate (float)</strong></td>
<td>transition rate from low to high velocity</td>
</tr>
</tbody>
</table>
Multi-Linear Velocity Dependent Friction

**frictionModel** (`'VelDepMultiLinear', frnTag, '-vel', *velocityPoints, '-frn', *frictionPoints)

This command is used to construct a VelDepMultiLinear friction model object. The friction-velocity relationship is given by a multi-linear curve that is defined by a set of points. The slope given by the last two specified points on the positive velocity axis is extrapolated to infinite positive velocities. Velocity and friction points need to be equal or larger than zero (no negative values should be defined). The number of provided velocity points needs to be equal to the number of provided friction points.

<table>
<thead>
<tr>
<th>Parameter</th>
<th>Description</th>
</tr>
</thead>
<tbody>
<tr>
<td>frnTag</td>
<td>unique friction model tag</td>
</tr>
<tr>
<td>velocityPoints</td>
<td>list of velocity points along friction-velocity curve</td>
</tr>
<tr>
<td>frictionPoints</td>
<td>list of friction points along friction-velocity curve</td>
</tr>
</tbody>
</table>

1.4.17 geomTransf commands

**geomTransf** (`transfType, transfTag, *transfArgs`)

The geometric-transformation command is used to construct a coordinate-transformation (CrdTransf) object, which transforms beam element stiffness and resisting force from the basic system to the global-coordinate system. The command has at least one argument, the transformation type.

<table>
<thead>
<tr>
<th>Parameter</th>
<th>Description</th>
</tr>
</thead>
<tbody>
<tr>
<td>transfType</td>
<td>geomTransf type</td>
</tr>
<tr>
<td>transfTag</td>
<td>geomTransf tag</td>
</tr>
<tr>
<td>transfArgs</td>
<td>a list of geomTransf arguments, must be preceded with *</td>
</tr>
</tbody>
</table>

For example,

```python
transfType = 'Linear'
transfTag = 1
transfArgs = []
geomTransf(transfType, transfTag, *transfArgs)
```

The following contain information about available **transfType**:

1. **Linear Transformation**
2. **PDelta Transformation**
3. **Corotational Transformation**

**Linear Transformation**

**geomTransf** (`'Linear', transfTag, '-jntOffset', *dI, *dJ`)

This command is used to construct a linear coordinate transformation (LinearCrdTransf) object, which performs a linear geometric transformation of beam stiffness and resisting force from the basic system to the global-coordinate system.

- **transfTag** (int): integer tag identifying transformation.
- **vecxz** (list(float)): X, Y, and Z components of vecxz, the vector used to define the local x-z plane of the local-coordinate system. The local y-axis is defined by taking the cross product of the vecxz vector and the x-axis. These components are specified in the global-coordinate system X,Y,Z and define a vector that is in a plane parallel to the x-z plane of the local-coordinate system. These items need to be specified for the three-dimensional problem.
- **dI** (list(float)): joint offset values – offsets specified with respect to the global coordinate system for element-end node i (the number of arguments depends on the dimensions of the current model).
- **dJ** (list(float)): joint offset values – offsets specified with respect to the global coordinate system for element-end node j (the number of arguments depends on the dimensions of the current model).

PDelta Transformation

geomTransf (‘PDelta’, transfTag, ‘-jntOffset’, *dI, *dJ)


This command is used to construct the P-Delta Coordinate Transformation (PDeltaCrdTransf) object, which performs a linear geometric transformation of beam stiffness and resisting force from the basic system to the global coordinate system, considering second-order P-Delta effects.

- **transfTag** (int): integer tag identifying transformation.
- **vecxz** (list(float)): X, Y, and Z components of vecxz, the vector used to define the local x-z plane of the local-coordinate system. The local y-axis is defined by taking the cross product of the vecxz vector and the x-axis. These components are specified in the global-coordinate system X,Y,Z and define a vector that is in a plane parallel to the x-z plane of the local-coordinate system. These items need to be specified for the three-dimensional problem.
- **dI** (list(float)): joint offset values – offsets specified with respect to the global coordinate system for element-end node i (the number of arguments depends on the dimensions of the current model).
- **dJ** (list(float)): joint offset values – offsets specified with respect to the global coordinate system for element-end node j (the number of arguments depends on the dimensions of the current model).

Note: P LARGE Delta effects do not include P small delta effects.

Corotational Transformation

geomTransf (‘Corotational’, transfTag, ‘-jntOffset’, *dI, *dJ)

geomTransf (‘Corotational’, transfTag, *vecxz)

This command is used to construct the Corotational Coordinate Transformation (CorotCrdTransf) object. Corotational transformation can be used in large displacement-small strain problems.
transTag (int)

Integer tag identifying transformation

vecxz (list(float))

X, Y, and Z components of vecxz, the vector used to define the local x-z plane of the local-coordinate system. The local y-axis is defined by taking the cross product of the vecxz vector and the x-axis. These components are specified in the global-coordinate system X,Y,Z and define a vector that is in a plane parallel to the x-z plane of the local-coordinate system. These items need to be specified for the three-dimensional problem.

dI (list(float))

Joint offset values – offsets specified with respect to the global coordinate system for element-end node i (the number of arguments depends on the dimensions of the current model).

dJ (list(float))

Joint offset values – offsets specified with respect to the global coordinate system for element-end node j (the number of arguments depends on the dimensions of the current model).

---

**Note:** Currently the transformation does not deal with element loads and will ignore any that are applied to the element.

### 1.5 Analysis Commands

In OpenSees, an analysis is an object which is composed by the aggregation of component objects. It is the component objects which define the type of analysis that is performed on the model. The component classes, as shown in the figure below, consist of the following:

1. ConstraintHandler – determines how the constraint equations are enforced in the analysis – how it handles the boundary conditions/imposed displacements
2. DOF_Numberer – determines the mapping between equation numbers and degrees-of-freedom
3. Integrator – determines the predictive step for time t+dt
4. SolutionAlgorithm – determines the sequence of steps taken to solve the non-linear equation at the current time step
5. SystemOfEqn/Solver – within the solution algorithm, it specifies how to store and solve the system of equations in the analysis
6. Convergence Test – determines when convergence has been achieved.

Analysis commands

1. *constraints commands*
2. *numberer commands*
3. *system commands*
4. *test commands*
5. *algorithm commands*
6. *integrator commands*
7. *analysis command*
8. *eigen command*
9. *analyze command*
1.5.1 constraints commands

**constraints** *(constraintType, *constraintArgs)*

This command is used to construct the ConstraintHandler object. The ConstraintHandler object determines how the constraint equations are enforced in the analysis. Constraint equations enforce a specified value for a DOF, or a relationship between DOFs.

<table>
<thead>
<tr>
<th>constraintType (str)</th>
<th>constraints type</th>
</tr>
</thead>
<tbody>
<tr>
<td>constraintArgs (list)</td>
<td>a list of constraints arguments</td>
</tr>
</tbody>
</table>

The following contain information about available `constraintType`:

1. **Plain Constraints**
2. **Lagrange Multipliers**
3. **Penalty Method**
4. **Transformation Method**

**Plain Constraints**

**constraints** *(‘Plain’)*

This command is used to construct a Plain constraint handler. A plain constraint handler can only enforce homogeneous single point constraints (fix command) and multi-point constraints constructed where the constraint matrix is equal to the identity (equalDOF command). The following is the command to construct a plain constraint handler:

**Note:** As mentioned, this constraint handler can only enforce homogeneous single point constraints (fix command) and multi-point constraints where the constraint matrix is equal to the identity (equalDOF command).

**Lagrange Multipliers**

**constraints** *(‘Lagrange’, alphaS=1.0, alphaM=1.0)*

This command is used to construct a LagrangeMultiplier constraint handler, which enforces the constraints by introducing Lagrange multiplies to the system of equation. The following is the command to construct a plain constraint handler:

<table>
<thead>
<tr>
<th>alphaS (float)</th>
<th>( \alpha_S ) factor on single points.</th>
</tr>
</thead>
<tbody>
<tr>
<td>alphaM (float)</td>
<td>( \alpha_M ) factor on multi-points.</td>
</tr>
</tbody>
</table>

**Note:** The Lagrange multiplier method introduces new unknowns to the system of equations. The diagonal part of the system corresponding to these new unknowns is 0.0. This ensure that the system IS NOT symmetric positive definite.

**Penalty Method**

**constraints** *(‘Penalty’, alphaS=1.0, alphaM=1.0)*

This command is used to construct a Penalty constraint handler, which enforces the constraints using the penalty method. The following is the command to construct a penalty constraint handler:
alphaS (float) $\alpha_S$ factor on single points.
alphaM (float) $\alpha_M$ factor on multi-points.

**Note:** The degree to which the constraints are enforced is dependent on the penalty values chosen. Problems can arise if these values are too small (constraint not enforced strongly enough) or too large (problems associated with conditioning of the system of equations).

**Transformation Method**

`constraints ('Transformation')`

This command is used to construct a transformation constraint handler, which enforces the constraints using the transformation method. The following is the command to construct a transformation constraint handler.

**Note:**

- The single-point constraints when using the transformation method are done directly. The matrix equation is not manipulated to enforce them, rather the trial displacements are set directly at the nodes at the start of each analysis step.
- Great care must be taken when multiple constraints are being enforced as the transformation method does not follow constraints:
  1. If a node is fixed, constrain it with the fix command and not equalDOF or other type of constraint.
  2. If multiple nodes are constrained, make sure that the retained node is not constrained in any other constraint.

And remember if a node is constrained to multiple nodes in your model it probably means you have messed up.

**1.5.2 numberer commands**

`numberer (numbererType, *numbererArgs)`

This command is used to construct the DOF_Numberer object. The DOF_Numberer object determines the mapping between equation numbers and degrees-of-freedom – how degrees-of-freedom are numbered.

<table>
<thead>
<tr>
<th>numbererType (str)</th>
<th>numberer type</th>
</tr>
</thead>
<tbody>
<tr>
<td>numbererArgs (list)</td>
<td>a list of numberer arguments</td>
</tr>
</tbody>
</table>

The following contain information about available `numbererType`:

1. Plain Numberer
2. RCM Numberer
3. AMD Numberer
4. Parallel Plain Numberer
5. Parallel RCM Numberer

1.5. Analysis Commands
Plain Numberer

`numberer ('Plain')`

This command is used to construct a Plain degree-of-freedom numbering object to provide the mapping between the degrees-of-freedom at the nodes and the equation numbers. A Plain numberer just takes whatever order the domain gives it nodes and numbers them, this ordering is both dependent on node numbering and size of the model.

**Note:** For very small problems and for the sparse matrix solvers which provide their own numbering scheme, order is not really important so plain numberer is just fine. For large models and analysis using solver types other than the sparse solvers, the order will have a major impact on performance of the solver and the plain handler is a poor choice.

RCM Numberer

`numberer ('RCM')`

This command is used to construct an RCM degree-of-freedom numbering object to provide the mapping between the degrees-of-freedom at the nodes and the equation numbers. An RCM numberer uses the reverse Cuthill-McKee scheme to order the matrix equations.

AMD Numberer

`numberer ('AMD')`

This command is used to construct an AMD degree-of-freedom numbering object to provide the mapping between the degrees-of-freedom at the nodes and the equation numbers. An AMD numberer uses the approximate minimum degree scheme to order the matrix equations.

Parallel Plain Numberer

`numberer ('ParallelPlain')`

This command is used to construct a parallel version of Plain degree-of-freedom numbering object to provide the mapping between the degrees-of-freedom at the nodes and the equation numbers. A Plain numberer just takes whatever order the domain gives it nodes and numbers them, this ordering is both dependent on node numbering and size of the model.

Use this command only for parallel model.

**Warning:** Don’t use this command if model is not parallel, for example, parametric study.

Parallel RCM Numberer

`numberer ('ParallelRCM')`

This command is used to construct a parallel version of RCM degree-of-freedom numbering object to provide the mapping between the degrees-of-freedom at the nodes and the equation numbers. A Plain numberer just takes whatever order the domain gives it nodes and numbers them, this ordering is both dependent on node numbering and size of the model.

Use this command only for parallel model.
Warning: Don’t use this command if model is not parallel, for example, parametric study.

1.5.3 system commands

system(systemType, *systemArgs)

This command is used to construct the LinearSOE and LinearSolver objects to store and solve the system of equations in the analysis.

<table>
<thead>
<tr>
<th>systemType (str)</th>
<th>system type</th>
</tr>
</thead>
<tbody>
<tr>
<td>systemArgs (list)</td>
<td>a list of system arguments</td>
</tr>
</tbody>
</table>

The following contain information about available systemType:

1. BandGeneral SOE
2. BandSPD SOE
3. ProfileSPD SOE
4. SuperLU SOE
5. UmfPack SOE
6. FullGeneral SOE
7. SparseSYM SOE
8. PFEM SOE
9. MUMPS Solver

**BandGeneral SOE**

system('BandGen')

This command is used to construct a BandGeneralSOE linear system of equation object. As the name implies, this class is used for matrix systems which have a banded profile. The matrix is stored as shown below in a 1 dimensional array of size equal to the bandwidth times the number of unknowns. When a solution is required, the Lapack routines DGBSV and SGBTRS are used.

**BandSPD SOE**

system('BandSPD')

This command is used to construct a BandSPDSOE linear system of equation object. As the name implies, this class is used for symmetric positive definite matrix systems which have a banded profile. The matrix is stored as shown below in a 1 dimensional array of size equal to the (bandwidth/2) times the number of unknowns. When a solution is required, the Lapack routines DPBSV and DPBTRS are used.

**ProfileSPD SOE**

system('ProfileSPD')

This command is used to construct a profileSPDSOE linear system of equation object. As the name implies, this class is used for symmetric positive definite matrix systems. The matrix is stored as shown below in a 1 dimensional array with only those values below the first non-zero row in any column being stored. This is sometimes also referred to as a skyline storage scheme.
SuperLU SOE

`system('SuperLU')`

This command is used to construct a SparseGEN linear system of equation object. As the name implies, this class is used for sparse matrix systems. The solution of the sparse matrix is carried out using SuperLU.

UmfPack SOE

`system('UmfPack')`

This command is used to construct a sparse system of equations which uses the UmfPack solver.

FullGeneral SOE

`system('FullGeneral')`

This command is used to construct a Full General linear system of equation object. As the name implies, the class utilizes NO space saving techniques to cut down on the amount of memory used. If the matrix is of size, n×n, then storage for an n×n array is sought from memory when the program runs. When a solution is required, the Lapack routines DGESV and DGETRS are used.

**Note:** This type of system should almost never be used! This is because it requires a lot more memory than every other solver and takes more time in the actual solving operation than any other solver. It is required if the user is interested in looking at the global system matrix.

SparseSYM SOE

`system('SparseSYM')`

This command is used to construct a sparse symmetric system of equations which uses a row-oriented solution method in the solution phase.

MUMPS Solver

`system('Mumps', '-ICNTL14', icntl14=20.0, '-ICNTL7', icntl7=7)`

Create a system of equations using the Mumps solver

<table>
<thead>
<tr>
<th>icntl14</th>
<th>controls the percentage increase in the estimated working space (optional)</th>
</tr>
</thead>
<tbody>
<tr>
<td>icntl7</td>
<td>computes a symmetric permutation (ordering) to determine the pivot order to be used for the factorization in case of sequential analysis (optional)</td>
</tr>
<tr>
<td></td>
<td>• 0: AMD</td>
</tr>
<tr>
<td></td>
<td>• 1: set by user</td>
</tr>
<tr>
<td></td>
<td>• 2: AMF</td>
</tr>
<tr>
<td></td>
<td>• 3: SCOTCH</td>
</tr>
<tr>
<td></td>
<td>• 4: PORD</td>
</tr>
<tr>
<td></td>
<td>• 5: Metis</td>
</tr>
<tr>
<td></td>
<td>• 6: AMD with QADM</td>
</tr>
<tr>
<td></td>
<td>• 7: automatic</td>
</tr>
</tbody>
</table>
Use this command only for parallel model.

**Warning:** Don’t use this command if model is not parallel, for example, parametric study.

### 1.5.4 test commands

**test** (*testType*, *testArgs*)

This command is used to construct the LinearSOE and LinearSolver objects to store and solve the test of equations in the analysis.

<table>
<thead>
<tr>
<th>testType (str)</th>
<th>test type</th>
</tr>
</thead>
<tbody>
<tr>
<td>testArgs (list)</td>
<td>a list of test arguments</td>
</tr>
</tbody>
</table>

The following contain information about available *testType*:

1. *NormUnbalance*
2. *NormDispIncr*
3. *energyIncr*
4. *RelativeNormUnbalance*
5. *RelativeNormDispIncr*
6. *RelativeTotalNormDispIncr*
7. *RelativeEnergyIncr*
8. *FixedNumIter*
9. *NormDispAndUnbalance*
10. *NormDispOrUnbalance*
11. *PFEM test*

**NormUnbalance**

**test** (*'NormUnbalance'*’, *tol*, *iter*, *pFlag*=0, *nType*=2, *maxincr*=-1)

Create a NormUnbalance test, which uses the norm of the right hand side of the matrix equation to determine if convergence has been reached.
tol (float)
Tolerance criteria used to check for convergence.

iter (int)
Max number of iterations to check

pFlag (int)
Print flag (optional):
• 0 print nothing.
• 1 print information on norms each time test () is invoked.
• 2 print information on norms and number of iterations at end of successful test.
• 4 at each step it will print the norms and also the δU and R(U) vectors.
• 5 if it fails to converge at end of numIter it will print an error message but return a successful test.

nType (int)
Type of norm, (0 = max-norm, 1 = 1-norm, 2 = 2-norm). (optional)

maxincr (int)
Maximum times of error increasing. (optional)

When using the Penalty method additional large forces to enforce the penalty functions exist on the right hand side, making convergence using this test usually impossible (even though solution might have converged).

NormDispIncr
test (‘NormDispIncr’, tol, iter, pFlag=0, nType=2)
Create a NormUnbalance test, which uses the norm of the left hand side solution vector of the matrix equation to determine if convergence has been reached.

energyIncr
test (‘EnergyIncr’, tol, iter, pFlag=0, nType=2)
Create a EnergyIncr test, which uses the dot product of the solution vector and norm of the right hand side of
the matrix equation to determine if convergence has been reached.

<table>
<thead>
<tr>
<th><strong>tol</strong> (float)</th>
<th>Tolerance criteria used to check for convergence.</th>
</tr>
</thead>
<tbody>
<tr>
<td><strong>iter</strong> (int)</td>
<td>Max number of iterations to check</td>
</tr>
<tr>
<td><strong>pFlag</strong> (int)</td>
<td>Print flag (optional):</td>
</tr>
<tr>
<td></td>
<td>• 0 print nothing.</td>
</tr>
<tr>
<td></td>
<td>• 1 print information on norms each time</td>
</tr>
<tr>
<td></td>
<td><code>test()</code> is invoked.</td>
</tr>
<tr>
<td></td>
<td>• 2 print information on norms and number of it-</td>
</tr>
<tr>
<td></td>
<td>erations at end of successful test.</td>
</tr>
<tr>
<td></td>
<td>• 4 at each step it will print the norms and al-</td>
</tr>
<tr>
<td></td>
<td>so the ( \Delta U ) and ( R(U) ) vectors.</td>
</tr>
<tr>
<td></td>
<td>• 5 if it fails to converge at end of <code>numIter</code> it</td>
</tr>
<tr>
<td></td>
<td>will print an error message **but return a suc-</td>
</tr>
<tr>
<td></td>
<td>cessful test.</td>
</tr>
</tbody>
</table>

| **nType** (int) | Type of norm, (0 = max-norm, 1 = 1-norm, 2 = 2-|
|                | norm). (optional)                              |

- When using the Penalty method additional large forces to enforce the penalty functions exist on the right hand side, making convergence using this test usually impossible (even though solution might have converged).

- When using the Lagrange method to enforce the constraints, the Lagrange multipliers appear in the solution vector.

**RelativeNormUnbalance**

`test` (`RelativeNormUnbalance`, `tol`, `iter`, `pFlag=0`, `nType=2`)

Create a RelativeNormUnbalance test, which uses the relative norm of the right hand side of the matrix equation to determine if convergence has been reached.

<table>
<thead>
<tr>
<th><strong>tol</strong> (float)</th>
<th>Tolerance criteria used to check for convergence.</th>
</tr>
</thead>
<tbody>
<tr>
<td><strong>iter</strong> (int)</td>
<td>Max number of iterations to check</td>
</tr>
<tr>
<td><strong>pFlag</strong> (int)</td>
<td>Print flag (optional):</td>
</tr>
<tr>
<td></td>
<td>• 0 print nothing.</td>
</tr>
<tr>
<td></td>
<td>• 1 print information on norms each time</td>
</tr>
<tr>
<td></td>
<td><code>test()</code> is invoked.</td>
</tr>
<tr>
<td></td>
<td>• 2 print information on norms and number of it-</td>
</tr>
<tr>
<td></td>
<td>erations at end of successful test.</td>
</tr>
<tr>
<td></td>
<td>• 4 at each step it will print the norms and al-</td>
</tr>
<tr>
<td></td>
<td>so the ( \Delta U ) and ( R(U) ) vectors.</td>
</tr>
<tr>
<td></td>
<td>• 5 if it fails to converge at end of <code>numIter</code> it</td>
</tr>
<tr>
<td></td>
<td>will print an error message **but return a suc-</td>
</tr>
<tr>
<td></td>
<td>cessful test.</td>
</tr>
</tbody>
</table>

| **nType** (int) | Type of norm, (0 = max-norm, 1 = 1-norm, 2 = 2-|
|                | norm). (optional)                              |

- When using the Penalty method additional large forces to enforce the penalty functions exist on the right hand side, making convergence using this test usually impossible (even though solution might have converged).
RelativeNormDispIncr

test ('RelativeNormDispIncr', tol, iter, pFlag=0, nType=2)

Create a RelativeNormDispIncr test, which uses the relative of the solution vector of the matrix equation to determine if convergence has been reached.

| tol (float) | Tolerance criteria used to check for convergence. |
| iter (int)  | Max number of iterations to check                |
| pFlag (int) | Print flag (optional):                          |
|             | • 0 print nothing.                              |
|             | • 1 print information on norms each time test () is invoked. |
|             | • 2 print information on norms and number of iterations at end of successful test. |
|             | • 4 at each step it will print the norms and also the ΔU and R(U) vectors. |
|             | • 5 if it fails to converge at end of numIter it will print an error message but return a successful test. |
| nType (int) | Type of norm, (0 = max-norm, 1 = 1-norm, 2 = 2-norm). (optional) |

RelativeTotalNormDispIncr

test ('relativeTotalNormDispIncr', tol, iter, pFlag=0, nType=2)

Create a RelativeTotalNormDispIncr test, which uses the ratio of the current norm to the total norm (the sum of all the norms since last convergence) of the solution vector.

| tol (float) | Tolerance criteria used to check for convergence. |
| iter (int)  | Max number of iterations to check                |
| pFlag (int) | Print flag (optional):                          |
|             | • 0 print nothing.                              |
|             | • 1 print information on norms each time test () is invoked. |
|             | • 2 print information on norms and number of iterations at end of successful test. |
|             | • 4 at each step it will print the norms and also the ΔU and R(U) vectors. |
|             | • 5 if it fails to converge at end of numIter it will print an error message but return a successful test. |
| nType (int) | Type of norm, (0 = max-norm, 1 = 1-norm, 2 = 2-norm). (optional) |

RelativeEnergyIncr

test ('RelativeEnergyIncr', tol, iter, pFlag=0, nType=2)

Create a RelativeEnergyIncr test, which uses the relative dot product of the solution vector and norm of the right hand side of the matrix equation to determine if convergence has been reached.
<table>
<thead>
<tr>
<th>tol (float)</th>
<th>Tolerance criteria used to check for convergence.</th>
</tr>
</thead>
<tbody>
<tr>
<td>iter (int)</td>
<td>Max number of iterations to check</td>
</tr>
<tr>
<td>pFlag (int)</td>
<td>Print flag (optional):</td>
</tr>
<tr>
<td></td>
<td>• 0 print nothing.</td>
</tr>
<tr>
<td></td>
<td>• 1 print information on norms each time</td>
</tr>
<tr>
<td></td>
<td>test() is invoked.</td>
</tr>
<tr>
<td></td>
<td>• 2 print information on norms and number of</td>
</tr>
<tr>
<td></td>
<td>iterations at end of successful test.</td>
</tr>
<tr>
<td></td>
<td>• 4 at each step it will print the norms and also</td>
</tr>
<tr>
<td></td>
<td>the $\Delta U$ and $R(U)$ vectors.</td>
</tr>
<tr>
<td></td>
<td>• 5 if it fails to converge at end of numIter it</td>
</tr>
<tr>
<td></td>
<td>will print an error message but return a</td>
</tr>
<tr>
<td></td>
<td>successful test.</td>
</tr>
<tr>
<td>nType (int)</td>
<td>Type of norm, $(0 = \text{max-norm}, 1 = \text{1-norm}, 2 = \text{2-norm})$. (optional)</td>
</tr>
</tbody>
</table>

**FixedNumIter**

**test ('FixedNumIter', iter, pFlag=0, nType=2)**

Create a FixedNumIter test, that performs a fixed number of iterations without testing for convergence.

<table>
<thead>
<tr>
<th>tol (float)</th>
<th>Tolerance criteria used to check for convergence.</th>
</tr>
</thead>
<tbody>
<tr>
<td>iter (int)</td>
<td>Max number of iterations to check</td>
</tr>
<tr>
<td>pFlag (int)</td>
<td>Print flag (optional):</td>
</tr>
<tr>
<td></td>
<td>• 0 print nothing.</td>
</tr>
<tr>
<td></td>
<td>• 1 print information on norms each time</td>
</tr>
<tr>
<td></td>
<td>test() is invoked.</td>
</tr>
<tr>
<td></td>
<td>• 2 print information on norms and number of</td>
</tr>
<tr>
<td></td>
<td>iterations at end of successful test.</td>
</tr>
<tr>
<td></td>
<td>• 4 at each step it will print the norms and also</td>
</tr>
<tr>
<td></td>
<td>the $\Delta U$ and $R(U)$ vectors.</td>
</tr>
<tr>
<td></td>
<td>• 5 if it fails to converge at end of numIter it</td>
</tr>
<tr>
<td></td>
<td>will print an error message but return a</td>
</tr>
<tr>
<td></td>
<td>successful test.</td>
</tr>
<tr>
<td>nType (int)</td>
<td>Type of norm, $(0 = \text{max-norm}, 1 = \text{1-norm}, 2 = \text{2-norm})$. (optional)</td>
</tr>
</tbody>
</table>

**NormDispAndUnbalance**

**test ('NormDispAndUnbalance', tolIncr, tolR, iter, pFlag=0, nType=2, maxincr=-1)**

Create a NormDispAndUnbalance test, which check if both 'NormUnbalance' and 'NormDispIncr' are converged.
1.5.5 algorithm commands

**algorithm**(algoType, *algoArgs)

This command is used to construct a SolutionAlgorithm object, which determines the sequence of steps taken to solve the non-linear equation.

```python
algorithm(algoType, *algoArgs)
```

- **algoType**: The algorithm type to be used.
- **algoArgs**: Additional arguments for the algorithm.

**NormDispOrUnbalance**

Create a NormDispOrUnbalance test, which check if both 'NormUnbalance' and 'normDispIncr' are converged.

```python
test('NormDispOrUnbalance', tolIncr, tolR, iter, pFlag=0, nType=2, maxincr=-1)
```

- **'NormDispOrUnbalance'**: The test type.
- **tolIncr** (float): Tolerance for left hand solution increments.
- **tolR** (float): Tolerance for right hand residual.
- **iter** (int): Max number of iterations to check.
- **pFlag** (int): Print flag (optional):
  - 0 print nothing.
  - 1 print information on norms each time test() is invoked.
  - 2 print information on norms and number of iterations at end of successful test.
  - 4 at each step it will print the norms and also the ΔU and R(U) vectors.
  - 5 if it fails to converge at end of numIter it will print an error message but return a successful test.
- **nType** (int): Type of norm, (0 = max-norm, 1 = 1-norm, 2 = 2-norm).
- **maxincr** (int): Maximum times of error increasing.

### Table: NormDispOrUnbalance Test Parameters

<table>
<thead>
<tr>
<th>Parameter</th>
<th>Description</th>
</tr>
</thead>
<tbody>
<tr>
<td>tolIncr (float)</td>
<td>Tolerance for left hand solution increments</td>
</tr>
<tr>
<td>tolIncr (float)</td>
<td>Tolerance for right hand residual</td>
</tr>
<tr>
<td>iter (int)</td>
<td>Max number of iterations to check</td>
</tr>
<tr>
<td>pFlag (int)</td>
<td>Print flag (optional):</td>
</tr>
<tr>
<td>nType (int)</td>
<td>Type of norm, (0 = max-norm, 1 = 1-norm, 2 = 2-norm). (optional)</td>
</tr>
<tr>
<td>maxincr (int)</td>
<td>Maximum times of error increasing. (optional)</td>
</tr>
</tbody>
</table>
The following contain information about available \texttt{algoType}:

1. Linear Algorithm
2. Newton Algorithm
3. Newton with Line Search
4. Modified Newton Algorithm
5. Krylov-Newton Algorithm
6. SecantNewton Algorithm
7. RaphsonNewton Algorithm
8. PeriodicNewton Algorithm
9. BFGS Algorithm
10. Broyden Algorithm

**Linear Algorithm**

\texttt{algorithm ('Linear', secant=False, initial=False, factorOnce=False)}

Create a Linear algorithm which takes one iteration to solve the system of equations.

| \texttt{secant} (bool) | Flag to indicate to use secant stiffness. (optional) |
| \texttt{initial} (bool) | Flag to indicate to use initial stiffness. (optional) |
| \texttt{factorOnce} (bool) | Flag to indicate to only set up and factor matrix once. (optional) |

**Note:** As the tangent matrix typically will not change during the analysis in case of an elastic system it is highly advantageous to use the \texttt{-factorOnce} option. Do not use this option if you have a nonlinear system and you want the tangent used to be actual tangent at time of the analysis step.

**Newton Algorithm**

\texttt{algorithm (‘Newton’, secant=False, initial=False, initialThenCurrent=False)}

Create a Newton-Raphson algorithm. The Newton-Raphson method is the most widely used and most robust method for solving nonlinear algebraic equations.

| \texttt{secant} (bool) | Flag to indicate to use secant stiffness. (optional) |
| \texttt{initial} (bool) | Flag to indicate to use initial stiffness. (optional) |
| \texttt{initialThenCurrent} (bool) | Flag to indicate to use initial stiffness on first step, then use current stiffness for subsequent steps. (optional) |
Newton with Line Search

**algorithm** (\texttt{NewtonLineSearch}, Bisection=False, Secant=False, RegulaFalsi=False, InitialInterpolated=False, tol=0.8, maxIter=10, minEta=0.1, maxEta=10.0)

Create a NewtonLineSearch algorithm. Introduces line search to the Newton algorithm to solve the nonlinear residual equation.

<table>
<thead>
<tr>
<th>Bisection (bool)</th>
<th>Flag to use Bisection line search. (optional)</th>
</tr>
</thead>
<tbody>
<tr>
<td>Secant (bool)</td>
<td>Flag to use Secant line search. (optional)</td>
</tr>
<tr>
<td>RegulaFalsi (bool)</td>
<td>Flag to use RegulaFalsi line search. (optional)</td>
</tr>
<tr>
<td>InitialInterpolated (bool)</td>
<td>Flag to use InitialInterpolated line search. (optional)</td>
</tr>
<tr>
<td>tol (float)</td>
<td>Tolerance for search. (optional)</td>
</tr>
<tr>
<td>maxIter (float)</td>
<td>Max num of iterations to try. (optional)</td>
</tr>
<tr>
<td>minEta (float)</td>
<td>Min $\eta$ value. (optional)</td>
</tr>
<tr>
<td>maxEta (float)</td>
<td>Max $\eta$ value. (optional)</td>
</tr>
</tbody>
</table>

Modified Newton Algorithm

**algorithm** (\texttt{ModifiedNewton}, secant=False, initial=False)

Create a ModifiedNewton algorithm. The difference to Newton is that the tangent at the initial guess is used in the iterations, instead of the current tangent.

<table>
<thead>
<tr>
<th>secant (bool)</th>
<th>Flag to indicate to use secant stiffness. (optional)</th>
</tr>
</thead>
<tbody>
<tr>
<td>initial (bool)</td>
<td>Flag to indicate to use initial stiffness. (optional)</td>
</tr>
</tbody>
</table>

Krylov-Newton Algorithm

**algorithm** (\texttt{KrylovNewton}, iterate='current', increment='current', maxDim=3)

Create a KrylovNewton algorithm which uses a Krylov subspace accelerator to accelerate the convergence of the ModifiedNewton.

<table>
<thead>
<tr>
<th>iterate (str)</th>
<th>Tangent to iterate on, 'current', 'initial', 'noTangent'. (optional)</th>
</tr>
</thead>
<tbody>
<tr>
<td>increment (str)</td>
<td>Tangent to increment on, 'current', 'initial', 'noTangent'. (optional)</td>
</tr>
<tr>
<td>maxDim (int)</td>
<td>Max number of iterations until the tangent is reformed and the acceleration restarts. (optional)</td>
</tr>
</tbody>
</table>

SecantNewton Algorithm

**algorithm** (\texttt{SecantNewton}, iterate='current', increment='current', maxDim=3)

Create a SecantNewton algorithm which uses the two-term update to accelerate the convergence of the ModifiedNewton.

The default “cut-out” values recommended by Crisfield (R1=3.5, R2=0.3) are used.

<table>
<thead>
<tr>
<th>iterate (str)</th>
<th>Tangent to iterate on, 'current', 'initial', 'noTangent'. (optional)</th>
</tr>
</thead>
<tbody>
<tr>
<td>increment (str)</td>
<td>Tangent to increment on, 'current', 'initial', 'noTangent'. (optional)</td>
</tr>
<tr>
<td>maxDim (int)</td>
<td>Max number of iterations until the tangent is reformed and the acceleration restarts. (optional)</td>
</tr>
</tbody>
</table>
RaphsonNewton Algorithm

```python
algorithm('RaphsonNewton', iterate='current', increment='current')
```

Create a RaphsonNewton algorithm which uses Raphson accelerator.

<table>
<thead>
<tr>
<th>iterate (str)</th>
<th>Tangent to iterate on, 'current', 'initial', 'noTangent' (optional)</th>
</tr>
</thead>
<tbody>
<tr>
<td>increment (str)</td>
<td>Tangent to increment on, 'current', 'initial', 'noTangent' (optional)</td>
</tr>
</tbody>
</table>

PeriodicNewton Algorithm

```python
algorithm('PeriodicNewton', iterate='current', increment='current', maxDim=3)
```

Create a PeriodicNewton algorithm using periodic accelerator.

<table>
<thead>
<tr>
<th>iterate (str)</th>
<th>Tangent to iterate on, 'current', 'initial', 'noTangent' (optional)</th>
</tr>
</thead>
<tbody>
<tr>
<td>increment (str)</td>
<td>Tangent to increment on, 'current', 'initial', 'noTangent' (optional)</td>
</tr>
<tr>
<td>maxDim (int)</td>
<td>Max number of iterations until the tangent is reformed and the acceleration restarts. (optional)</td>
</tr>
</tbody>
</table>

BFGS Algorithm

```python
algorithm('BFGS', secant=False, initial=False, count=10)
```

Create a BFGS algorithm. The BFGS method is one of the most effective matrix-update or quasi Newton methods for iteration on a nonlinear system of equations. The method computes new search directions at each iteration step based on the initial jacobian, and subsequent trial solutions. Unlike regular Newton does not require the tangent matrix be reformulated and refactored at every iteration, however unlike ModifiedNewton it does not rely on the tangent matrix from a previous iteration.

<table>
<thead>
<tr>
<th>secant (bool)</th>
<th>Flag to indicate to use secant stiffness. (optional)</th>
</tr>
</thead>
<tbody>
<tr>
<td>initial (bool)</td>
<td>Flag to indicate to use initial stiffness. (optional)</td>
</tr>
<tr>
<td>count (int)</td>
<td>Number of iterations. (optional)</td>
</tr>
</tbody>
</table>

Broyden Algorithm

```python
algorithm('Broyden', secant=False, initial=False, count=10)
```

Create a Broyden algorithm for general unsymmetric systems which performs successive rank-one updates of the tangent at the first iteration of the current time step.

<table>
<thead>
<tr>
<th>secant (bool)</th>
<th>Flag to indicate to use secant stiffness. (optional)</th>
</tr>
</thead>
<tbody>
<tr>
<td>initial (bool)</td>
<td>Flag to indicate to use initial stiffness. (optional)</td>
</tr>
<tr>
<td>count (int)</td>
<td>Number of iterations. (optional)</td>
</tr>
</tbody>
</table>

1.5.6 integrator commands

```python
integrator(intType, *intArgs)
```

This command is used to construct the Integrator object. The Integrator object determines the meaning of the terms in the system of equation object Ax=B.

The Integrator object is used for the following:
• determine the predictive step for time \( t+dt \)
• specify the tangent matrix and residual vector at any iteration
• determine the corrective step based on the displacement increment \( dU \)

<table>
<thead>
<tr>
<th>intType (str)</th>
<th>integrator type</th>
</tr>
</thead>
<tbody>
<tr>
<td>intArgs (list)</td>
<td>a list of integrator arguments</td>
</tr>
</tbody>
</table>

The following contain information about available intType:

**Static integrator objects**

1. LoadControl
2. DisplacementControl
3. Parallel DisplacementControl
4. Minimum Unbalanced Displacement Norm
5. Arc-Length Control

**LoadControl**

integrator ('LoadControl', incr, numIter=1, minincr=incr, maxincr=incr)

Create a OpenSees LoadControl integrator object.

<table>
<thead>
<tr>
<th>incr (float)</th>
<th>Load factor increment ( \lambda ).</th>
</tr>
</thead>
<tbody>
<tr>
<td>numIter (int)</td>
<td>Number of iterations the user would like to occur in the solution algorithm. (optional)</td>
</tr>
<tr>
<td>minIncr (float)</td>
<td>Min stepsize the user will allow ( \lambda_{\text{min}} ). (optional)</td>
</tr>
<tr>
<td>maxIncr (float)</td>
<td>Max stepsize the user will allow ( \lambda_{\text{max}} ). (optional)</td>
</tr>
</tbody>
</table>

1. The change in applied loads that this causes depends on the active load pattern (those load pattern not set constant) and the loads in the load pattern. If the only active load acting on the Domain are in load pattern with a Linear time series with a factor of 1.0, this integrator is the same as the classical load control method.

2. The optional arguments are supplied to speed up the step size in cases where convergence is too fast and slow down the step size in cases where convergence is too slow.

**DisplacementControl**

integrator ('DisplacementControl', nd, dof, incr, numIter=1, dUmin=incr, dUmax=incr)

Create a DisplacementControl integrator. In an analysis step with Displacement Control we seek to determine the time step that will result in a displacement increment for a particular degree-of-freedom at a node to be a prescribed value.

<table>
<thead>
<tr>
<th>nd (int)</th>
<th>tag of node whose response controls solution</th>
</tr>
</thead>
<tbody>
<tr>
<td>dof (int)</td>
<td>Degree of freedom at the node, 1 through ndf.</td>
</tr>
<tr>
<td>incr (float)</td>
<td>First displacement increment ( \Delta U_{\text{dof}} ).</td>
</tr>
<tr>
<td>numIter (int)</td>
<td>Number of iterations the user would like to occur in the solution algorithm. (optional)</td>
</tr>
<tr>
<td>minIncr (float)</td>
<td>Min stepsize the user will allow ( \Delta U_{\text{min}} ). (optional)</td>
</tr>
<tr>
<td>maxIncr (float)</td>
<td>Max stepsize the user will allow ( \Delta U_{\text{max}} ). (optional)</td>
</tr>
</tbody>
</table>
Parallel DisplacementControl

**integrator** ('ParallelDisplacementControl', nd, dof, incr, numIter=1, dUmin=incr, dUmax=incr)

Create a Parallel version of DisplacementControl integrator. In an analysis step with Displacement Control we seek to determine the time step that will result in a displacement increment for a particular degree-of-freedom at a node to be a prescribed value.

<table>
<thead>
<tr>
<th>Parameter</th>
<th>Description</th>
</tr>
</thead>
<tbody>
<tr>
<td>nd (int)</td>
<td>tag of node whose response controls solution</td>
</tr>
<tr>
<td>dof (int)</td>
<td>Degree of freedom at the node, 1 through ndf.</td>
</tr>
<tr>
<td>incr (float)</td>
<td>First displacement increment $\Delta U_{dof}$.</td>
</tr>
<tr>
<td>numIter (int)</td>
<td>Number of iterations the user would like to occur in the solution algorithm. (optional)</td>
</tr>
<tr>
<td>minIncr (float)</td>
<td>Min stepsize the user will allow $\Delta U_{\text{min}}$. (optional)</td>
</tr>
<tr>
<td>maxIncr (float)</td>
<td>Max stepsize the user will allow $\Delta U_{\text{max}}$. (optional)</td>
</tr>
</tbody>
</table>

Use this command only for parallel model.

**Warning:** Don’t use this command if model is not parallel, for example, parametric study.

Minimum Unbalanced Displacement Norm

**integrator** ('MinUnbalDispNorm', dlambda1, Jd=1, minLambda=dlambda1, maxLambda=dlambda1, det=False)

Create a MinUnbalDispNorm integrator.

<table>
<thead>
<tr>
<th>Parameter</th>
<th>Description</th>
</tr>
</thead>
<tbody>
<tr>
<td>dlambda1 (float)</td>
<td>First load increment (pseudo-time step) at the first iteration in the next invocation of the analysis command.</td>
</tr>
<tr>
<td>Jd (int)</td>
<td>Factor relating first load increment at subsequent time steps. (optional)</td>
</tr>
<tr>
<td>minLambda (float)</td>
<td>Min load increment. (optional)</td>
</tr>
<tr>
<td>maxLambda (float)</td>
<td>Max load increment. (optional)</td>
</tr>
</tbody>
</table>

Arc-Length Control

**integrator** ('ArcLength', s, alpha)

Create a ArcLength integrator. In an analysis step with ArcLength we seek to determine the time step that will result in our constraint equation being satisfied.

<table>
<thead>
<tr>
<th>Parameter</th>
<th>Description</th>
</tr>
</thead>
<tbody>
<tr>
<td>s (float)</td>
<td>The arcLength.</td>
</tr>
<tr>
<td>alpha (float)</td>
<td>$\alpha$ a scaling factor on the reference loads.</td>
</tr>
</tbody>
</table>

Transient integrator objects

1. Central Difference
2. Explicit Difference
3. Newmark Method
4. *Hilber-Hughes-Taylor Method*

5. *Generalized Alpha Method*

6. *TRBDF2*

7. *Explicit Difference*

8. *PFEM integrator*

**Central Difference**

integrator ('CentralDifference')

Create a centralDifference integrator.

1. The calculation of $U_t + \Delta t$, is based on using the equilibrium equation at time $t$. For this reason the method is called an explicit integration method.

2. If there is no rayleigh damping and the $C$ matrix is 0, for a diagonal mass matrix a diagonal solver may and should be used.

3. For stability, $\frac{\Delta t}{T_n} < \frac{1}{\pi}$

**Explicit Difference**

integrator ('ExplicitDifference')

Create a ExplicitDifference integrator.

**Newmark Method**

integrator ('Newmark', gamma, beta, '-formD', form)

Create a Newmark integrator.

<table>
<thead>
<tr>
<th>gamma (float)</th>
<th>$\gamma$ factor.</th>
</tr>
</thead>
<tbody>
<tr>
<td>beta (float)</td>
<td>$\beta$ factor.</td>
</tr>
<tr>
<td>form (str)</td>
<td>Flag to indicate which variable to be used as primary variable (optional)</td>
</tr>
<tr>
<td></td>
<td>• 'D' – displacement (default)</td>
</tr>
<tr>
<td></td>
<td>• 'V' – velocity</td>
</tr>
<tr>
<td></td>
<td>• 'A' – acceleration</td>
</tr>
</tbody>
</table>

1. If the accelerations are chosen as the unknowns and $\beta$ is chosen as 0, the formulation results in the fast but conditionally stable explicit Central Difference method. Otherwise the method is implicit and requires an iterative solution process.

2. Two common sets of choices are
   1. Average Acceleration Method ($\gamma = \frac{1}{2}, \beta = \frac{1}{4}$)
   2. Linear Acceleration Method ($\gamma = \frac{1}{2}, \beta = \frac{1}{6}$)

3. $\gamma > \frac{1}{2}$ results in numerical damping proportional to $\gamma - \frac{1}{2}$

4. The method is second order accurate if and only if $\gamma = \frac{1}{2}$

5. The method is unconditionally stable for $\beta >= \frac{7}{2} >= \frac{1}{4}$
Hilber-Hughes-Taylor Method

integrator ('HHT', alpha, gamma=1.5-alpha, beta=(2-alpha)^2/4)

Create a Hilber-Hughes-Taylor (HHT) integrator. This is an implicit method that allows for energy dissipation and second order accuracy (which is not possible with the regular Newmark object). Depending on choices of input parameters, the method can be unconditionally stable.

- **alpha (float)**: \( \alpha \) factor.
- **gamma (float)**: \( \gamma \) factor. (optional)
- **beta (float)**: \( \beta \) factor. (optional)

1. Like Newmark and all the implicit schemes, the unconditional stability of this method applies to linear problems. There are no results showing stability of this method over the wide range of nonlinear problems that potentially exist. Experience indicates that the time step for implicit schemes in nonlinear situations can be much greater than those for explicit schemes.

2. \( \alpha = 1.0 \) corresponds to the Newmark method.

3. \( \alpha \) should be between 0.67 and 1.0. The smaller the \( \alpha \) the greater the numerical damping.

4. \( \gamma \) and \( \beta \) are optional. The default values ensure the method is second order accurate and unconditionally stable when \( \alpha \) is \( \frac{2}{3} \leq \alpha \leq 1.0 \). The defaults are:
   \[
   \begin{align*}
   \beta &= \frac{(2-\alpha)^2}{4} \\
   \gamma &= \frac{3}{2} - \alpha
   \end{align*}
   
   and

Generalized Alpha Method

integrator ('GeneralizedAlpha', alphaM, alphaF, gamma=0.5+alphaM-alphaF, beta=(1+alphaM-alphaF)^2/4)

Create a GeneralizedAlpha integrator. This is an implicit method that like the HHT method allows for high frequency energy dissipation and second order accuracy, i.e. \( \Delta t^2 \). Depending on choices of input parameters, the method can be unconditionally stable.

- **alphaM (float)**: \( \alpha_M \) factor.
- **alphaF (float)**: \( \alpha_F \) factor.
- **gamma (float)**: \( \gamma \) factor. (optional)
- **beta (float)**: \( \beta \) factor. (optional)

1. Like Newmark and all the implicit schemes, the unconditional stability of this method applies to linear problems. There are no results showing stability of this method over the wide range of nonlinear problems that potentially exist. Experience indicates that the time step for implicit schemes in nonlinear situations can be much greater than those for explicit schemes.

2. \( \alpha_M = 1.0, \alpha_F = 1.0 \) produces the Newmark Method.

3. \( \alpha_M = 1.0 \) corresponds to the integrator.HHT() method.

4. The method is second-order accurate provided \( \gamma = \frac{1}{2} + \alpha_M - \alpha_F \)

5. The method is unconditionally stable provided \( \alpha_M \geq \alpha_F \geq \frac{1}{2}, \beta \geq \frac{3}{4} + \frac{1}{3} (\gamma_M - \gamma_F) \)
6. $\gamma$ and $\beta$ are optional. The default values ensure the method is unconditionally stable, second order accurate and high frequency dissipation is maximized.

The defaults are:

$$\gamma = \frac{1}{2} + \alpha_M - \alpha_F$$

and

$$\beta = \frac{1}{4}(1 + \alpha_M - \alpha_F)^2$$

**TRBDF2**

`integrator('TRBDF2')`

Create a TRBDF2 integrator. The TRBDF2 integrator is a composite scheme that alternates between the Trapezoidal scheme and a 3 point backward Euler scheme. It does this in an attempt to conserve energy and momentum, something Newmark does not always do.

As opposed to dividing the time-step in 2 as outlined in the Bathe2007, we just switch alternate between the 2 integration strategies, i.e. the time step in our implementation is double that described in the Bathe2007.

**Explicit Difference**

`integrator('ExplicitDifference')`

Create a ExplicitDifference integrator.

1. When using Rayleigh damping, the damping ratio of high vibration modes is overrated, and the critical time step size will be much smaller. Hence Modal damping is more suitable for this method.

2. There should be no zero element on the diagonal of the mass matrix when using this method.

3. Diagonal solver should be used when lumped mass matrix is used because the equations are uncoupled.

4. For stability, $\Delta t \leq \left(\sqrt{\zeta^2 + 1 - \zeta}\right) \frac{2}{\omega}$

**1.5.7 analysis command**

`analysis(analysisType)`

This command is used to construct the Analysis object, which defines what type of analysis is to be performed.

- determine the predictive step for time $t+dt$
- specify the tangent matrix and residual vector at any iteration
- determine the corrective step based on the displacement increment $dU$

<table>
<thead>
<tr>
<th>analysisType (str)</th>
<th>char string identifying type of analysis object to be constructed. Currently 3 valid options:</th>
</tr>
</thead>
<tbody>
<tr>
<td></td>
<td>1. 'Static' - for static analysis</td>
</tr>
<tr>
<td></td>
<td>2. 'Transient' - for transient analysis constant time step</td>
</tr>
<tr>
<td></td>
<td>3. 'VariableTransient' - for transient analysis with variable time step</td>
</tr>
<tr>
<td></td>
<td>4. 'PFEM' - for PFEM analysis.</td>
</tr>
</tbody>
</table>
Note: If the component objects are not defined beforehand, the command automatically creates default component objects and issues warning messages to this effect. The number of warning messages depends on the number of component objects that are undefined.

1.5.8 eigen command

eigen(solver='genBandArpack', numEigenvalues)

Eigen value analysis. Return a list of eigen values.

<table>
<thead>
<tr>
<th>numEigenvalues (int)</th>
<th>number of eigenvalues required</th>
</tr>
</thead>
<tbody>
<tr>
<td>solver (str)</td>
<td>optional string detailing type of solver: 'genBandArpack', 'symmBandLapack', 'fullGenLapack', (optional)</td>
</tr>
</tbody>
</table>

Note:

1. The eigenvectors are stored at the nodes and can be printed out using a Node Recorder, the nodeEigenvector command, or the Print command.

2. The default eigensolver is able to solve only for N-1 eigenvalues, where N is the number of inertial DOFs. When running into this limitation the -fullGenLapack solver can be used instead of the default Arpack solver.

1.5.9 analyze command

analyze(numIncr=1, dt=0.0, dtMin=0.0, dtMax=0.0, Jd=0)

Perform the analysis. Return 0 if successful, <0 if NOT successful

<table>
<thead>
<tr>
<th>numIncr (int)</th>
<th>Number of analysis steps to perform. (required except for PFEM analysis)</th>
</tr>
</thead>
<tbody>
<tr>
<td>dt (float)</td>
<td>Time-step increment. (required for Transient analysis and VariableTransient analysis.)</td>
</tr>
<tr>
<td>dtMin (float)</td>
<td>Minimum time steps. (required for VariableTransient analysis)</td>
</tr>
<tr>
<td>dtMax (float)</td>
<td>Maximum time steps (required for VariableTransient analysis)</td>
</tr>
<tr>
<td>Jd (float)</td>
<td>Number of iterations user would like performed at each step. The variable transient analysis will change current time step if last analysis step took more or less iterations than this to converge (required for VariableTransient analysis)</td>
</tr>
</tbody>
</table>

1.6 Output Commands

Get outputs from OpenSees. These commands don’t change internal states of OpenSees.

1. basicDeformation command
2. basicForce command
3. basicStiffness command
4. eleDynamicalForce command
5. eleForce command
6. eleNodes command
7. eleResponse command
8. getEleTags command
9. getLoadFactor command
10. getNodeTags command
11. getTime command
12. nodeAccel command
13. nodeBounds command
14. nodeCoord command
15. nodeDisp command
16. nodeEigenvector command
17. nodeDOFs command
18. nodeMass command
19. nodePressure command
20. nodeReaction command
21. nodeResponse command
22. nodeVel command
23. nodeUnbalance command
24. numFact command
25. numIter command
26. printA command
27. printB command
28. printGID command
29. printModel command
30. record command
31. recorder command
32. sectionForce command
33. sectionDeformation command
34. sectionStiffness command
35. sectionFlexibility command
36. sectionLocation command
37. sectionWeight command
38. systemSize command
39. testIter command
40. *testNorm command*
41. *version command*
42. *logFile command*

### 1.6.1 basicDeformation command

**basicDeformation** *(eleTag)*  
Returns the deformation of the basic system for a beam-column element.

<table>
<thead>
<tr>
<th>eleTag</th>
<th>element tag</th>
</tr>
</thead>
</table>

### 1.6.2 basicForce command

**basicForce** *(eleTag)*  
Returns the forces of the basic system for a beam-column element.

<table>
<thead>
<tr>
<th>eleTag</th>
<th>element tag</th>
</tr>
</thead>
</table>

### 1.6.3 basicStiffness command

**basicStiffness** *(eleTag)*  
Returns the stiffness of the basic system for a beam-column element. A list of values in row order will be returned.

<table>
<thead>
<tr>
<th>eleTag</th>
<th>element tag</th>
</tr>
</thead>
</table>

### 1.6.4 eleDynamicalForce command

**eleDynamicalForce** *(eleTag, dof=-1)*  
Returns the elemental dynamic force.

<table>
<thead>
<tr>
<th>eleTag</th>
<th>element tag</th>
</tr>
</thead>
<tbody>
<tr>
<td>dof</td>
<td>specific dof at the element, (optional), if no dof is provided, a list of values for all dofs is returned.</td>
</tr>
</tbody>
</table>

### 1.6.5 eleForce command

**eleForce** *(eleTag, dof=-1)*  
Returns the elemental resisting force.

<table>
<thead>
<tr>
<th>eleTag</th>
<th>element tag</th>
</tr>
</thead>
<tbody>
<tr>
<td>dof</td>
<td>specific dof at the element, (optional), if no dof is provided, a list of values for all dofs is returned.</td>
</tr>
</tbody>
</table>

1.6. Output Commands
1.6.6 eleNodes command

eleNodes (eleTag)
Get nodes in an element

\[
\text{eleTag}(\text{int}) \quad \text{element tag.}
\]

1.6.7 eleResponse command

eleResponse (eleTag, *args)
This command is used to obtain the same element quantities as those obtained from the element recorder at a particular time step.

<table>
<thead>
<tr>
<th>eletag (int)</th>
<th>element tag.</th>
</tr>
</thead>
<tbody>
<tr>
<td>args (list)</td>
<td>same arguments as those specified in element recorder. These arguments are specific to the type of element being used.</td>
</tr>
</tbody>
</table>

1.6.8 getEleTags command

geteleTags ('-mesh', mtag)
Get all elements in the domain or in a mesh.

\[
\text{mtag}(\text{int}) \quad \text{mesh tag. (optional)}
\]

1.6.9 getLoadFactor command

getLoadFactor (patternTag)
Returns the load factor $\lambda$ for the pattern

\[
\text{patternTag}(\text{int}) \quad \text{pattern tag.}
\]

1.6.10 getNodeTags command

getNodeTags ('-mesh', mtag)
Get all nodes in the domain or in a mesh.

\[
\text{mtag}(\text{int}) \quad \text{mesh tag. (optional)}
\]

1.6.11 getTime command

getime ()
Returns the current time in the domain.
1.6.12 nodeAccel command

**nodeAccel** *(nodeTag, dof=-1)*

Returns the current acceleration at a specified node.

<table>
<thead>
<tr>
<th>nodeTag (int)</th>
<th>node tag.</th>
</tr>
</thead>
<tbody>
<tr>
<td>dof (int)</td>
<td>specific dof at the node (1 through ndf), (optional), if no dof is provided, a list of values for all dofs is returned.</td>
</tr>
</tbody>
</table>

1.6.13 nodeBounds command

**nodeBounds** *

Get the boundary of all nodes. Return a list of boundary values.

1.6.14 nodeCoord command

**nodeCoord** *(nodeTag, dim=-1)*

Returns the coordinates of a specified node.

<table>
<thead>
<tr>
<th>nodeTag (int)</th>
<th>node tag.</th>
</tr>
</thead>
<tbody>
<tr>
<td>dof (int)</td>
<td>specific dimension at the node (1 through ndf), (optional), if no dim is provided, a list of values for all dimensions is returned.</td>
</tr>
</tbody>
</table>

1.6.15 nodeDisp command

**nodeDisp** *(nodeTag, dof=-1)*

Returns the current displacement at a specified node.

<table>
<thead>
<tr>
<th>nodeTag (int)</th>
<th>node tag.</th>
</tr>
</thead>
<tbody>
<tr>
<td>dof (int)</td>
<td>specific dof at the node (1 through ndf), (optional), if no dof is provided, a list of values for all dofs is returned.</td>
</tr>
</tbody>
</table>

1.6.16 nodeEigenvector command

**nodeEigenvector** *(eigenvector, dof=-1)*

Returns the eigenvector at a specified node.

<table>
<thead>
<tr>
<th>nodeTag (int)</th>
<th>node tag.</th>
</tr>
</thead>
<tbody>
<tr>
<td>eigenvector (int)</td>
<td>mode number of eigenvector to be returned</td>
</tr>
<tr>
<td>dof (int)</td>
<td>specific dof at the node (1 through ndf), (optional), if no dof is provided, a list of values for all dofs is returned.</td>
</tr>
</tbody>
</table>
1.6.17 nodeDOFs command

nodeDOFs (nodeTag)

Returns the DOF numbering of a node.

```
nodeTag (int)  node tag.
```

1.6.18 nodeMass command

nodeMass (nodeTag, dof=-1)

Returns the mass at a specified node.

```
nodeTag (int)  node tag.
dof (int)       specific dof at the node (1 through ndf), (optional), if no dof is provided, a list of values for all dofs is returned.
```

1.6.19 nodePressure command

nodePressure (nodeTag)

Returns the fluid pressures at a specified node if this is a fluid node.

```
nodeTag (int)  node tag.
```

1.6.20 nodeReaction command

nodeReaction (nodeTag, dof=-1)

Returns the reactions at a specified node. Must call reactions() command before this command.

```
nodeTag (int)  node tag.
dof (int)       specific dof at the node (1 through ndf), (optional), if no dof is provided, a list of values for all dofs is returned.
```

1.6.21 nodeResponse command

nodeResponse (nodeTag, dof, responseID)

Returns the responses at a specified node. Must call responses command before this command.
1.6.22 nodeVel command

**nodeVel** \((nodeTag, dof=-1)\)

Returns the current velocity at a specified node.

<table>
<thead>
<tr>
<th>nodeTag (int)</th>
<th>node tag.</th>
</tr>
</thead>
<tbody>
<tr>
<td>dof (int)</td>
<td>specific dof at the node (1 through ndf), (optional), if no dof is provided, a list of values for all dofs is returned.</td>
</tr>
</tbody>
</table>

1.6.23 nodeUnbalance command

**nodeUnbalance** \((nodeTag, dof=-1)\)

Returns the unbalanced force at a specified node.

<table>
<thead>
<tr>
<th>nodeTag (int)</th>
<th>node tag.</th>
</tr>
</thead>
<tbody>
<tr>
<td>dof (int)</td>
<td>specific dof at the node (1 through ndf), (optional), if no dof is provided, a list of values for all dofs is returned.</td>
</tr>
</tbody>
</table>

1.6.24 numFact command

**numFact**

Return the number of factorizations.

1.6.25 numIter command

**numIter**

Return the number of iterations.

1.6.26 printA command

**printA**\(\text{('-file', filename, '-ret')}\)

Print the contents of a FullGeneral system that the integrator creates to the screen or a file if the '-file' parameter is provided.
option is used. If using a static integrator, the resulting matrix is the stiffness matrix. If a transient integrator, it will be some combination of mass and stiffness matrices.

<table>
<thead>
<tr>
<th>filename (str)</th>
<th>name of file to which output is sent, by default, print to the screen. (optional)</th>
</tr>
</thead>
<tbody>
<tr>
<td>'-ret' (str)</td>
<td>return the A matrix as a list. (optional)</td>
</tr>
</tbody>
</table>

1.6.27 printB command

printB (’-file’, filename, ’-ret’)
print the right hand side of a FullGeneral system that the integrator creates to the screen or a file if the ’-file’ option is used.

<table>
<thead>
<tr>
<th>filename (str)</th>
<th>name of file to which output is sent, by default, print to the screen. (optional)</th>
</tr>
</thead>
<tbody>
<tr>
<td>'-ret' (str)</td>
<td>return the B vector as a list. (optional)</td>
</tr>
</tbody>
</table>

1.6.28 printGID command

printGID (filename, ’-append’, ’-eleRange’, startEle, endEle)
Print in GID format.

<table>
<thead>
<tr>
<th>filename (str)</th>
<th>output file name.</th>
</tr>
</thead>
<tbody>
<tr>
<td>'-append' (str)</td>
<td>append to existing file. (optional)</td>
</tr>
<tr>
<td>startEle (int)</td>
<td>start element tag. (optional)</td>
</tr>
<tr>
<td>endEle (int)</td>
<td>end element tag. (optional)</td>
</tr>
</tbody>
</table>

1.6.29 printModel command

printModel (’-file’, filename, ’-JSON’, ’-node’, ’-flag’, *nodes=[], *eles=[]) This command is used to print output to screen or file.

<table>
<thead>
<tr>
<th>filename (str)</th>
<th>name of file to which output is sent, by default, print to the screen. (optional)</th>
</tr>
</thead>
<tbody>
<tr>
<td>'-JSON' (str)</td>
<td>print to a JSON file. (optional)</td>
</tr>
<tr>
<td>'-node' (str)</td>
<td>print node information. (optional)</td>
</tr>
<tr>
<td>flag (int)</td>
<td>integer flag to be sent to the print() method, depending on the node and element type (optional)</td>
</tr>
<tr>
<td>nodes (list (int))</td>
<td>a list of nodes tags to be printed, default is to print all, (optional)</td>
</tr>
<tr>
<td>eles (list (int))</td>
<td>a list of element tags to be printed, default is to print all, (optional)</td>
</tr>
</tbody>
</table>

Note: This command was called print in Tcl. Since print is a built-in function in Python, it is renamed to printModel.
1.6.30 record command

`record()`

This command is used to cause all the recorders to do a record on the current state of the model.

**Note:** A record is issued after every successful static or transient analysis step. Sometimes the user may need the record to be issued on more occasions than this, for example if the user is just looking to record the eigenvectors after an eigen command or for example the user wishes to include the state of the model at time 0.0 before any analysis has been completed.

1.6.31 recorder command

`recorder(recorderType, *recorderArgs)`

This command is used to generate a recorder object which is to monitor what is happening during the analysis and generate output for the user.

Return:

- >0 an integer tag that can be used as a handle on the recorder for the remove recorder command.
- -1 recorder command failed if integer -1 returned.

<table>
<thead>
<tr>
<th>recorderType (str)</th>
<th>recorder type</th>
</tr>
</thead>
<tbody>
<tr>
<td>recorderArgs (list)</td>
<td>a list of recorder arguments</td>
</tr>
</tbody>
</table>

The following contain information about available `recorderType`:

**node recorder command**

`recorder('Node', '-file', filename, '-xml', filename, '-binary', filename, '-tcp', inetAddress, port, '-precision', nSD=6, '-timeSeries', tsTag, '-time', '-dT', deltaT=0.0, '-closeOnWrite', '-node', *nodeTags=[], '-nodeRange', startNode, endNode, '-region', regionTag, '-dof', *dofs=[], respType)`

The Node recorder type records the response of a number of nodes at every converged step.
<table>
<thead>
<tr>
<th>Parameter</th>
<th>Description</th>
</tr>
</thead>
<tbody>
<tr>
<td>filename (str)</td>
<td>name of file to which output is sent. File output is either in xml format (’-xml’ option), textual (’-file’ option) or binary (’-binary’ option) which must pre-exist.</td>
</tr>
<tr>
<td>inetAddr (str)</td>
<td>ip address, “xx.xx.xx.xx”, of remote machine to which data is sent. (optional)</td>
</tr>
<tr>
<td>port (int)</td>
<td>port on remote machine awaiting tcp. (optional)</td>
</tr>
<tr>
<td>nSD (int)</td>
<td>number of significant digits (optional)</td>
</tr>
<tr>
<td>’-time’ (str)</td>
<td>using this option places domain time in first entry of each data line, default is to have time omitted, (optional)</td>
</tr>
<tr>
<td>’-closeOnWrite’ (str)</td>
<td>using this option will instruct the recorder to invoke a close on the data handler after every timestep. If this is a file it will close the file on every step and then re-open it for the next step. Note, this greatly slows the execution time, but is useful if you need to monitor the data during the analysis. (optional)</td>
</tr>
<tr>
<td>deltaT (float)</td>
<td>time interval for recording. will record when next step is deltaT greater than last recorder step. (optional, default: records at every time step)</td>
</tr>
<tr>
<td>tsTag (int)</td>
<td>the tag of a previously constructed TimeSeries, results from node at each time step are added to load factor from series (optional)</td>
</tr>
<tr>
<td>nodeTags (list (int))</td>
<td>list of tags of nodes whose response is being recorded (optional)</td>
</tr>
<tr>
<td>startNode (int)</td>
<td>tag for start node whose response is being recorded (optional)</td>
</tr>
<tr>
<td>endNode (int)</td>
<td>tag for end node whose response is being recorded (optional)</td>
</tr>
<tr>
<td>regionTag (int)</td>
<td>a region tag; to specify all nodes in the previously defined region. (optional)</td>
</tr>
<tr>
<td>dofs (list (int))</td>
<td>the specified dof at the nodes whose response is requested.</td>
</tr>
<tr>
<td>resType (list (str))</td>
<td>a string indicating response required. Response types are given in table below</td>
</tr>
<tr>
<td></td>
<td>• ’disp’ displacement</td>
</tr>
<tr>
<td></td>
<td>• ’vel’ velocity</td>
</tr>
<tr>
<td></td>
<td>• ’accel’ acceleration</td>
</tr>
<tr>
<td></td>
<td>• ’incrDisp’ incremental displacement</td>
</tr>
<tr>
<td></td>
<td>• ’reaction’ nodal reaction</td>
</tr>
<tr>
<td></td>
<td>• ’eigen i’ eigenvector for mode i</td>
</tr>
<tr>
<td></td>
<td>• ’rayleighForces’ damping forces</td>
</tr>
</tbody>
</table>

**Note:** Only one of ’-file’, ’-xml’, ’-binary’, ’-tcp’ will be used. If multiple specified last option is used.
The EnvelopeNode recorder type records the min, max and absolute max of a number of nodal response quantities.

- **filename (str)**: name of file to which output is sent. File output is either in xml format (`'-xml'` option), or textual (`'-file'` option) which must pre-exist.
- **nSD (int)**: number of significant digits (optional)
- **'-time' (str)**: using this option places domain time in first entry of each data line, default is to have time omitted, (optional)
- **'-closeOnWrite' (str)**: using this option will instruct the recorder to invoke a close on the data handler after every timestep. If this is a file it will close the file on every step and then re-open it for the next step. Note, this greatly slows the execution time, but is useful if you need to monitor the data during the analysis. (optional)
- **deltaT (float)**: time interval for recording. will record when next step is `deltaT` greater than last recorder step. (optional, default: records at every time step)
- **tsTag (int)**: the tag of a previously constructed TimeSeries, results from node at each time step are added to load factor from series (optional)
- **nodeTags (list (int))**: list of tags of nodes whose response is being recorded (optional)
- **startNode (int)**: tag for start node whose response is being recorded (optional)
- **endNode (int)**: tag for end node whose response is being recorded (optional)
- **regionTag (int)**: a region tag; to specify all nodes in the previously defined region. (optional)
- **dofs (list (int))**: the specified dof at the nodes whose response is requested.
- **resType (list (str))**: a string indicating response required. Response types are given in table below
  - 'disp' displacement
  - 'vel' velocity
  - 'accel' acceleration
  - 'incrDisp' incremental displacement
  - 'reaction' nodal reaction
  - 'eigen i' eigenvector for mode i

The Element recorder type records the response of a number of elements at every converged step. The response
recorded is element-dependent and also depends on the arguments which are passed to the \texttt{setResponse()} element method.

| filename  | name of file to which output is sent. file output is either in xml format ('-xml' option), textual ('-file' option) or binary ('-binary' option) which must pre-exist. |
| nSD       | number of significant digits (optional) |
| '-time'   | using this option places domain time in first entry of each data line, default is to have time omitted, (optional) |
| '-close'  | using this option will instruct the recorder to invoke a close on the data handler after every timestep. If this is a file it will close the file on every step and then re-open it for the next step. Note, this greatly slows the execution time, but is useful if you need to monitor the data during the analysis. (optional) |
| deltaT    | time interval for recording. will record when next step is \texttt{deltaT} greater than last recorder step. (optional, default: records at every time step) |
| tsTag     | the tag of a previously constructed TimeSeries, results from node at each time step are added to load factor from series (optional) |
| eleTags   | list of tags of elements whose response is being recorded (optional) |
| startEle  | tag for start node whose response is being recorded (optional) |
| endEle    | tag for end node whose response is being recorded (optional) |
| regionTag | a region tag; to specify all nodes in the previously defined region. (optional) |
| args      | arguments which are passed to the \texttt{setResponse()} element method, all arguments must be in string format even for double and integer numbers because internally the \texttt{setResponse()} element method only accepts strings. |

\textbf{Note:} The \texttt{setResponse()} element method is dependent on the element type, and is described with the \texttt{element()} Command.

\textbf{element envelope recorder command}

\texttt{recorder ('EnvelopeElement', '-file', filename, '-xml', filename, '-binary', filename, '-precision', nSD=6, '-timeSeries', tsTag, '-time', '-dT', deltaT=0.0, '-closeOnWrite', '-ele', *eleTags=[], '-eleRange', startEle, endEle, '-region', regionTag, *args)}

The Envelope Element recorder type records the response of a number of elements at every converged step. The response recorded is element-dependent and also depends on the arguments which are passed to the \texttt{setResponse()} element method. When the object is terminated, through the use of a wipe, exit, or remove the object will output the min, max and absolute max values on 3 separate lines of the output file for each quantity.
filename (str) | name of file to which output is sent. File output is either in xml format ('-xml' option), textual ('-file' option) or binary ('-binary' option) which must pre-exist.
---|---
nSD (int) | number of significant digits (optional)
'-time' (str) | using this option places domain time in first entry of each data line, default is to have time omitted, (optional)
'-closeOnWrite' (str) | using this option will instruct the recorder to invoke a close on the data handler after every timestep. If this is a file it will close the file on every step and then re-open it for the next step. Note, this greatly slows the execution time, but is useful if you need to monitor the data during the analysis. (optional)
deltaT (float) | time interval for recording. Will record when next step is deltaT greater than last recorder step. (optional, default: records at every time step)
tsTag (int) | the tag of a previously constructed TimeSeries, results from node at each time step are added to load factor from series (optional)
eleTags (list (int)) | list of tags of elements whose response is being recorded (optional)
startEle (int) | tag for start node whose response is being recorded (optional)
endEle (int) | tag for end node whose response is being recorded (optional)
regionTag (int) | a region tag; to specify all nodes in the previously defined region. (optional)
args (list) | arguments which are passed to the setResponse() element method

**Note:** The setResponse() element method is dependent on the element type, and is described with the `element()` Command.

### pvd recorder command

**recorder** (`'PVD', filename, '-precision', precision=10, '-dT', dT=0.0, *res)

Create a PVD recorder.
background recorder command

 recorder ('BgPVD', filename, '-precision', precision=10, '-dT', dT=0.0, *res)

Create a PVD recorder for background mesh. This recorder is same as the PVD recorder, but will be automatically called in background mesh and is able to record wave height and velocity.

<table>
<thead>
<tr>
<th>filename (str)</th>
<th>the name for filename.pvd and filename/ directory, which must pre-exist.</th>
</tr>
</thead>
<tbody>
<tr>
<td>precision (int)</td>
<td>the precision of data. (optional)</td>
</tr>
<tr>
<td>dT (float)</td>
<td>the time interval for recording. (optional)</td>
</tr>
<tr>
<td>res (list (str))</td>
<td>a list of (str) of responses to be recorded, (optional)</td>
</tr>
</tbody>
</table>

  • 'disp'
  • 'vel'
  • 'accel'
  • 'incrDisp'
  • 'reaction'
  • 'pressure'
  • 'unbalancedLoad'
  • 'mass'
  • 'eigen'

1.6.32 sectionForce command

 sectionForce (eleTag, secNum, dof)

Returns the section force for a beam-column element. The dof of the section depends on the section type. Please check with the section manual.

<table>
<thead>
<tr>
<th>eleTag (int)</th>
<th>element tag.</th>
</tr>
</thead>
<tbody>
<tr>
<td>secNum (int)</td>
<td>section number, i.e. the Gauss integratio number</td>
</tr>
<tr>
<td>dof (int)</td>
<td>the dof of the section</td>
</tr>
</tbody>
</table>

1.6.33 sectionDeformation command

 sectionDeformation (eleTag, secNum, dof)

Returns the section deformation for a beam-column element. The dof of the section depends on the section type. Please check with the section manual.

<table>
<thead>
<tr>
<th>eleTag (int)</th>
<th>element tag.</th>
</tr>
</thead>
<tbody>
<tr>
<td>secNum (int)</td>
<td>section number, i.e. the Gauss integratio number</td>
</tr>
<tr>
<td>dof (int)</td>
<td>the dof of the section</td>
</tr>
</tbody>
</table>

1.6.34 sectionStiffness command

 sectionStiffness (eleTag, secNum, dof)

Returns the section stiffness matrix for a beam-column element. A list of values in the row order will be returned.
### 1.6.35 sectionFlexibility command

**sectionFlexibility** *(eleTag, secNum, dof)*

Returns the section flexibility matrix for a beam-column element. A list of values in the row order will be returned.

| eleTag (int) | element tag. |
| secNum (int) | section number, i.e. the Gauss integration number |
| dof (int)    | the dof of the section |

### 1.6.36 sectionLocation command

**sectionLocation** *(eleTag, secNum)*

Returns the locations of integration points of a section for a beam-column element.

| eleTag (int) | element tag. |
| secNum (int) | section number, i.e. the Gauss integration number |

### 1.6.37 sectionWeight command

**sectionWeight** *(eleTag, secNum)*

Returns the weights of integration points of a section for a beam-column element.

| eleTag (int) | element tag. |
| secNum (int) | section number, i.e. the Gauss integration number |

### 1.6.38 systemSize command

**systemSize()**

Return the size of the system.

### 1.6.39 testIter command

**testIter()**

Returns the number of iterations the convergence test took in the last analysis step

### 1.6.40 testNorm command

**testNorm()**

Returns the norms from the convergence test for the last analysis step.
Note: The size of norms will be equal to the max number of iterations specified. The first `testIter` of these will be non-zero, the remaining ones will be zero.

1.6.41 version command

`version()`
Return the current OpenSees version.

1.6.42 logFile command

`logFile(filename, '-append', '-noEcho')`
Log all messages and errors in a file. By default, all messages and errors print to terminal or Jupyter Notebook depending on how Python was run.

<table>
<thead>
<tr>
<th>filename (str)</th>
<th>name of the log file</th>
</tr>
</thead>
<tbody>
<tr>
<td>'-append' (str)</td>
<td>append to the file</td>
</tr>
<tr>
<td>'-noEcho' (str)</td>
<td>do not print to terminal or Jupyter Notebook</td>
</tr>
</tbody>
</table>

1.7 Utility Commands

These commands are used to monitor and change the state of the model.

1. `convertBinaryToText command`
2. `convertTextToBinary command`
3. `database command`
4. `InitialStateAnalysis command`
5. `loadConst command`
6. `modalDamping command`
7. `reactions command`
8. `remove command`
9. `reset command`
10. `restore command`
11. `save command`
12. `sdfResponse command`
13. `setTime command`
14. `setNodeCoord command`
15. `setNodeDisp command`
16. `setNodeVel command`
17. `setNodeAccel command`
18. `setPrecision command`
19. `setElementRayleighDampingFactors command`
20. `start command`
21. `stop command`
22. `stripXML command`
23. `updateElementDomain command`
24. `updateMaterialStage`
25. `wipe command`
26. `wipeAnalysis command`
27. `setNumthread command`
28. `getNumthread command`

1.7.1 `convertBinaryToText command`

`convertBinaryToText (inputfile, outputfile)`

Convert binary file to text file

<table>
<thead>
<tr>
<th>inputfile (str)</th>
<th>input file name.</th>
</tr>
</thead>
<tbody>
<tr>
<td>outputfile (str)</td>
<td>output file name.</td>
</tr>
</tbody>
</table>

1.7.2 `convertTextToBinary command`

`convertTextToBinary (inputfile, outputfile)`

Convert text file to binary file

<table>
<thead>
<tr>
<th>inputfile (str)</th>
<th>input file name.</th>
</tr>
</thead>
<tbody>
<tr>
<td>outputfile (str)</td>
<td>output file name.</td>
</tr>
</tbody>
</table>

1.7.3 `database command`

`database (type, dbName)`

Create a database.

<table>
<thead>
<tr>
<th>type (str)</th>
<th>database type:</th>
</tr>
</thead>
<tbody>
<tr>
<td></td>
<td>- 'File' - outputs database into a file</td>
</tr>
<tr>
<td></td>
<td>- 'MySQL' - creates a SQL database</td>
</tr>
<tr>
<td></td>
<td>- 'BerkeleyDB' - creates a BerkeleyDB database</td>
</tr>
</tbody>
</table>

<table>
<thead>
<tr>
<th>dbName (str)</th>
<th>database name.</th>
</tr>
</thead>
</table>

1.7.4 `InitialStateAnalysis command`

`InitialStateAnalysis (flag)`

Set the initial state analysis to 'on' or 'off'
1.7.5 loadConst command

loadConst ('-time', pseudoTime)
This command is used to set the loads constant in the domain and to also set the time in the domain. When setting the loads constant, the procedure will invoke setLoadConst() on all LoadPattern objects which exist in the domain at the time the command is called.

pseudoTime (float)  Time domain is to be set to (optional)

Note: Load Patterns added after this command is invoked are not set to constant.

1.7.6 modalDamping command

modalDamping (factor)
Set modal damping factor. The eigen() must be called before.

factor (float)  damping factor.

1.7.7 reactions command

reactions ('-dynamic', '-rayleigh')
Calculate the reactions. Call this command before the nodeReaction().

'-dynamic' (str)  Include dynamic effects.
'-rayleigh' (str)  Include rayleigh damping.

1.7.8 remove command

remove (type, tag)
This command is used to remove components from the model.

type (str)  type of the object, 'ele', 'loadPattern', 'parameter', 'node', 'timeSeries', 'sp', 'mp'.
tag (int)  tag of the object

remove ('recorders')
Remove all recorder objects.
remove ('sp', nodeTag, dofTag, patternTag)
Remove a sp object based on node

nodeTag (int)  node tag
dof (int)  dof the sp constrains
patternTag (int)  pattern tag, (optional)
1.7.9 reset command

reset()

This command is used to set the state of the domain to its original state.

Note: It iterates over all components of the domain telling them to set their state back to the initial state. This is not always the same as going back to the state of the model after initial model generation, e.g. if elements have been removed.

1.7.10 restore command

restore(commitTag)

Restore data from database, which should be created through database().

commitTag(int) a tag identify the commit

1.7.11 save command

save(commitTag)

Save current state to database, which should be created through database().

commitTag(int) a tag identify the commit

1.7.12 sdfResponse command

sdfResponse(m, zeta, k, Fy, alpha, dtF, filename, dt[, uresidual, umaxprev])

It is a command that computes bilinear single degree of freedom response in C++, and is much quicker than using the OpenSees model builder. The command implements Newmark’s method with an inner Newton loop.

<table>
<thead>
<tr>
<th>m (float)</th>
<th>mass</th>
</tr>
</thead>
<tbody>
<tr>
<td>zeta (float)</td>
<td>damping ratio</td>
</tr>
<tr>
<td>k (float)</td>
<td>stiffness</td>
</tr>
<tr>
<td>Fy (float)</td>
<td>yielding strength</td>
</tr>
<tr>
<td>alpha (float)</td>
<td>strain-hardening ratio</td>
</tr>
<tr>
<td>dtF (float)</td>
<td>time step for input data</td>
</tr>
<tr>
<td>filename (str)</td>
<td>input data file, one force per line</td>
</tr>
<tr>
<td>dt (float)</td>
<td>time step for analysis</td>
</tr>
<tr>
<td>uresidual (float)</td>
<td>residual displacement at the end of previous analysis (optional)</td>
</tr>
<tr>
<td>umaxprev (float)</td>
<td>previous displacement (optional)</td>
</tr>
</tbody>
</table>

1.7.13 setTime command

setTime(pseudoTime)

This command is used to set the time in the Domain.

pseudoTime(float) Time domain to be set
1.7.14 setNodeCoord command

**setNodeCoord** *(nodeTag, dim, value)*

set the nodal coordinate at the specified dimension.

<table>
<thead>
<tr>
<th>nodeTag (int)</th>
<th>node tag.</th>
</tr>
</thead>
<tbody>
<tr>
<td>dim (int)</td>
<td>the dimension of the coordinate to be set.</td>
</tr>
<tr>
<td>value (float)</td>
<td>coordinate value</td>
</tr>
</tbody>
</table>

1.7.15 setNodeDisp command

**setNodeDisp** *(nodeTag, dim, value, '-commit')*

set the nodal displacement at the specified dimension.

<table>
<thead>
<tr>
<th>nodeTag (int)</th>
<th>node tag.</th>
</tr>
</thead>
<tbody>
<tr>
<td>dim (int)</td>
<td>the dimension of the dispinate to be set.</td>
</tr>
<tr>
<td>value (float)</td>
<td>displacement value</td>
</tr>
<tr>
<td>'-commit' (str)</td>
<td>commit nodal state. (optional)</td>
</tr>
</tbody>
</table>

1.7.16 setNodeVel command

**setNodeVel** *(nodeTag, dim, value, '-commit')*

set the nodal velocity at the specified dimension.

<table>
<thead>
<tr>
<th>nodeTag (int)</th>
<th>node tag.</th>
</tr>
</thead>
<tbody>
<tr>
<td>dim (int)</td>
<td>the dimension of the velinate to be set.</td>
</tr>
<tr>
<td>value (float)</td>
<td>velocity value</td>
</tr>
<tr>
<td>'-commit' (str)</td>
<td>commit nodal state. (optional)</td>
</tr>
</tbody>
</table>

1.7.17 setNodeAccel command

**setNodeAccel** *(nodeTag, dim, value, '-commit')*

set the nodal acceleration at the specified dimension.

<table>
<thead>
<tr>
<th>nodeTag (int)</th>
<th>node tag.</th>
</tr>
</thead>
<tbody>
<tr>
<td>dim (int)</td>
<td>the dimension of the accelinate to be set.</td>
</tr>
<tr>
<td>value (float)</td>
<td>acceleration value</td>
</tr>
<tr>
<td>'-commit' (str)</td>
<td>commit nodal state. (optional)</td>
</tr>
</tbody>
</table>

1.7.18 setPrecision command

**setPrecision** *(precision)*

Set the precision for screen output.

| precision (int) | the precision number. |
1.7.19 setElementRayleighDampingFactors command

`setElementRayleighDampingFactors(eleTag, alphaM, betaK, betaK0, betaKc)`

Set the `rayleigh()` damping for an element.

<table>
<thead>
<tr>
<th>eleTag (int)</th>
<th>element tag</th>
</tr>
</thead>
<tbody>
<tr>
<td>alphaM (float)</td>
<td>factor applied to elements or nodes mass matrix</td>
</tr>
<tr>
<td>betaK (float)</td>
<td>factor applied to elements current stiffness matrix.</td>
</tr>
<tr>
<td>betaK0 (float)</td>
<td>factor applied to elements initial stiffness matrix.</td>
</tr>
<tr>
<td>betaKc (float)</td>
<td>factor applied to elements committed stiffness matrix.</td>
</tr>
</tbody>
</table>

1.7.20 start command

`start()`

Start the timer.

1.7.21 stop command

`stop()`

Stop the timer and print timing information.

1.7.22 stripXML command

`stripXML(inputxml, outputdata, outputxml)`

Strip a xml file to a data file and a descriptive file.

<table>
<thead>
<tr>
<th>inputxml (str)</th>
<th>input xml file name.</th>
</tr>
</thead>
<tbody>
<tr>
<td>outputdata (str)</td>
<td>output data file name.</td>
</tr>
<tr>
<td>outputxml (str)</td>
<td>output xml file name.</td>
</tr>
</tbody>
</table>

1.7.23 updateElementDomain command

`updateElementDomain()`

Update elements in the domain.

1.7.24 updateMaterialStage

`updateMaterialStage('-material', matTag, '-stage', value, '-parameter', paramTag)`

This function is used in geotechnical modeling to maintain elastic nDMaterial response during the application of gravity loads. The material is then updated to allow for plastic strains during additional static loads or earthquakes.

<table>
<thead>
<tr>
<th>matTag (int)</th>
<th>tag of nDMaterial</th>
</tr>
</thead>
<tbody>
<tr>
<td>value (int)</td>
<td>stage value</td>
</tr>
<tr>
<td>paramTag (int)</td>
<td>tag of parameter (optional)</td>
</tr>
</tbody>
</table>
1.7.25 wipe command

\texttt{wipe()}

This command is used to destroy all constructed objects, i.e. all components of the model, all components of the analysis and all recorders.

This command is used to start over without having to exit and restart the interpreter. It causes all elements, nodes, constraints, loads to be removed from the domain. In addition it deletes all recorders, analysis objects and all material objects created by the model builder.

1.7.26 wipeAnalysis command

\texttt{wipeAnalysis()}

This command is used to destroy all components of the Analysis object, i.e. any objects created with system, numberer, constraints, integrator, algorithm, and analysis commands.

1.7.27 setNumthread command

\texttt{setNumThread(num)}

set the number of threads to be used in the multi-threaded environment.

\begin{verbatim}
num (int) number of threads
\end{verbatim}

1.7.28 getNumthread command

\texttt{getNumThread(num)}

return the total number of threads available

1.8 FSI Commands

These commands are related to the Fluid-Structure Interaction analysis in OpenSees.

1. mesh command
2. remesh command
3. PFEM integrator
4. PFEM SOE
5. PFEM test
6. PFEM analysis

1.8.1 mesh command

\texttt{mesh(type, tag, *args)}

Create a mesh object. See below for available mesh types.
**line mesh**

`mesh ('line', tag, numnodes, *ndtags, id, ndf, meshsize, eleType="", *eleArgs=[])`  
Create a line mesh object.

<table>
<thead>
<tr>
<th>Parameter</th>
<th>Description</th>
</tr>
</thead>
<tbody>
<tr>
<td>tag (int)</td>
<td>mesh tag.</td>
</tr>
<tr>
<td>numnodes (int)</td>
<td>number of nodes for defining consecutive lines.</td>
</tr>
<tr>
<td>ndtags (list(int))</td>
<td>the node tags</td>
</tr>
<tr>
<td>id (int)</td>
<td>mesh id. Meshes with same id are considered as</td>
</tr>
<tr>
<td></td>
<td>same structure of fluid identity.</td>
</tr>
<tr>
<td></td>
<td>• id = 0 : not in FSI</td>
</tr>
<tr>
<td></td>
<td>• id &gt; 0 : structure</td>
</tr>
<tr>
<td></td>
<td>• id &lt; 0 : fluid</td>
</tr>
<tr>
<td>ndf (int)</td>
<td>ndf for nodes to be created.</td>
</tr>
<tr>
<td>meshsize (float)</td>
<td>mesh size.</td>
</tr>
<tr>
<td>eleType (str)</td>
<td>the type of the element, (optional)</td>
</tr>
<tr>
<td></td>
<td>• Elastic Beam Column Element</td>
</tr>
<tr>
<td></td>
<td>• forceBeamColumn</td>
</tr>
<tr>
<td></td>
<td>• dispBeamColumn</td>
</tr>
<tr>
<td></td>
<td>if no type is given, only nodes are created</td>
</tr>
<tr>
<td>eleArgs (list)</td>
<td>a list of element arguments. The arguments are</td>
</tr>
<tr>
<td></td>
<td>same as in the element commands, but without</td>
</tr>
<tr>
<td></td>
<td>element tag, and node tags. (optional)</td>
</tr>
<tr>
<td></td>
<td>For example,</td>
</tr>
<tr>
<td></td>
<td>eleArgs = ['elasticBeamColumn', A, E, Iz, transfTag]</td>
</tr>
</tbody>
</table>

**triangular mesh**

`mesh ('tri', tag, numlines, *ltags, id, ndf, meshsize, eleType="", *eleArgs=[])`  
Create a triangular mesh object.
tag (int) mesh tag.
numlines (int) number of lines (line mesh) for defining a polygon.
ltags (list (int)) the line mesh tags
id (int) mesh id. Meshes with same id are considered as same structure of fluid identity.
  • id = 0: not in FSI
  • id > 0: structure
  • id < 0: fluid
ndf (int) ndf for nodes to be created.
meshsize (float) mesh size.
eleType (str) the element type, (optional)
  • PFEMElementBubble
  • PFEMElementCompressible
  • Tri31 Element
  if no type is given, only nodes are created
eleArgs (list) a list of element arguments. The arguments are same as in the element commands, but without element tag, and node tags. (optional)
  For example,
  eleArgs = ['PFEMElementBubble', rho, mu, b1, b2, thickness, kappa]

**particle mesh**

mesh ('part', tag, type, *pArgs, eleType=', *eleArgs=[], '-vel', *vel0, '-pressure', p0)
Create a particle mesh which is used for background mesh.
### OpenSeesPy Documentation, Release 0.4.2019.7

**background mesh**


Create a background mesh. Structural mesh must be created before the background mesh in order to be included.

<table>
<thead>
<tr>
<th>basicsize (float)</th>
<th>basic mesh size</th>
</tr>
</thead>
<tbody>
<tr>
<td>lower (list (float))</td>
<td>a list of coordinates of the lower point of the background region.</td>
</tr>
<tr>
<td>upper (list (float))</td>
<td>a list of coordinates of the upper point of the background region.</td>
</tr>
<tr>
<td>tol (float)</td>
<td>tolerance for intri check. (optional, default 1e-10)</td>
</tr>
<tr>
<td>meshtol (float)</td>
<td>tolerance for cell boundary check. (optional, default 0.1)</td>
</tr>
<tr>
<td>wavefilename (str)</td>
<td>a filename to record wave heights and velocities (optional)</td>
</tr>
<tr>
<td>numl (int)</td>
<td>number of locations to record wave (optional)</td>
</tr>
<tr>
<td>locations (list (float))</td>
<td>coordinates of the locations (optional)</td>
</tr>
<tr>
<td>wlower (list (float))</td>
<td>a list of coordinates of the lower point of the wall (optional)</td>
</tr>
<tr>
<td>wupper (list (float))</td>
<td>a list of coordinates of the upper point of the wall (optional)</td>
</tr>
<tr>
<td>numsnodes (int)</td>
<td>number of structural nodes (optional)</td>
</tr>
<tr>
<td>sNodes (list (int))</td>
<td>a list of structural nodes (optional)</td>
</tr>
</tbody>
</table>
1.8.2 remesh command

\texttt{remesh(alpha=-1.0)}

- \( \alpha \geq 0 \) for updating moving mesh.
- \( \alpha < 0 \) for updating background mesh.

<table>
<thead>
<tr>
<th>alpha (float)</th>
<th>Parameter for the ( \alpha ) method to construct a mesh from the node cloud of moving meshes. (optional)</th>
</tr>
</thead>
<tbody>
<tr>
<td>( \alpha = 0 )</td>
<td>no elements are created</td>
</tr>
<tr>
<td>large ( \alpha )</td>
<td>all elements in the convex hull are created</td>
</tr>
<tr>
<td>( 1.0 &lt; \alpha &lt; 2.0 )</td>
<td>usually gives a good shape</td>
</tr>
</tbody>
</table>

1.8.3 PFEM integrator

\texttt{integrator('PFEM')}  
Create a PFEM Integrator.

1.8.4 PFEM SOE

\texttt{system('PFEM', '-compressible', '-mumps')}  
Create a incompressible PFEM system of equations using the Umfpack solver

<table>
<thead>
<tr>
<th>-compressible</th>
<th>Solve using a quasi-incompressible formulation. (optional)</th>
</tr>
</thead>
<tbody>
<tr>
<td>-mumps</td>
<td>Solve using the MUMPS solver. (optional, not supported on Windows)</td>
</tr>
</tbody>
</table>

1.8.5 PFEM test

\texttt{test('PFEM', tolv, tolp, tolrv, tolrlp, tolrelv, tolrelp, iter, maxincr, pFlag=0, nType=2)}  
Create a PFEM test, which check both increments and residual for velocities and pressures.
tolv (float)  Tolerance for velocity increments
tolp (float)  Tolerance for pressure increments
tolv (float)  Tolerance for velocity residual
tolrp (float) Tolerance for pressure residual
tolv (float)  Tolerance for relative velocity increments
tolrp (float) Tolerance for relative pressure increments
iter (int)    Max number of iterations to check
maxincr (int) Max times for error increasing
pFlag (int)   Print flag (optional):
              • 0 print nothing.
              • 1 print information on norms each time test() is invoked.
              • 2 print information on norms and number of iterations at end of successful test.
              • 4 at each step it will print the norms and also the $\Delta U$ and $R(U)$ vectors.
              • 5 if it fails to converge at end of numIter it will print an error message but return a successful test.

nType (int)   Type of norm, (0 = max-norm, 1 = 1-norm, 2 = 2-norm). (optional)

1.8.6 PFEM analysis

analysis ('PFEM', dtmax, dtmin, gravity, ratio=0.5)
Create a OpenSees PFEMAnalysis object.

<table>
<thead>
<tr>
<th>dtmax (float)</th>
<th>Maximum time steps.</th>
</tr>
</thead>
<tbody>
<tr>
<td>dtmin (float)</td>
<td>Minimum time steps.</td>
</tr>
<tr>
<td>gravity (float)</td>
<td>Gravity acceleration used to move isolated particles.</td>
</tr>
<tr>
<td>ratio (float)</td>
<td>The ratio to reduce time steps if it was not converged. (optional)</td>
</tr>
</tbody>
</table>

1.9 Sensitivity Commands

These commands are for sensitivity analysis in OpenSees.

1. parameter command
2. addToParameter command
3. updateParameter command
4. setParameter command
5. getParamTags command
6. getParamValue command
7. computeGradients command
8. sensitivityAlgorithm command
9. sensNodeDisp command
10. `sensNodeVel` command
11. `sensNodeAccel` command
12. `sensLambda` command
13. `sensSectionForce` command
14. `sensNodePressure` command

### 1.9.1 parameter command

```plaintext
parameter (tag, <specific parameter args>)
```

In DDM-based FE response sensitivity analysis, the sensitivity parameters can be material, geometry or discrete loading parameters.

<table>
<thead>
<tr>
<th>tag (int)</th>
<th>integer tag identifying the parameter.</th>
</tr>
</thead>
<tbody>
<tr>
<td>&lt;specific parameter args&gt;</td>
<td>depend on the object in the FE model encapsulating the desired parameters.</td>
</tr>
</tbody>
</table>

**Note:** Each parameter must be unique in the FE domain, and all parameter tags must be numbered sequentially starting from 1.

#### Examples

1. To identify the elastic modulus, E, of the material 1 at section 3 of element 4, the `<specific object arguments>` string becomes:

   ```plaintext
   parameter 1 element 4 section 3 material 1 E
   ```

2. To identify the elastic modulus, E, of elastic section 3 of element 4 (for elastic section, no specific material need to be defined), the `<specific object arguments>` string becomes:

   ```plaintext
   parameter 1 element 4 section 3 E
   ```

3. To parameterize E for element 4 with material 1 (no section need to be defined), the `<specific object arguments>` string simplifies as:

   ```plaintext
   parameter 1 element 4 material 1 E
   ```

**Note:** Notice that the format of the `<specific object arguments>` is different for each considered element/section/material. The specific set of parameters and the relative `<specific object arguments>` format will be added in the future.

### 1.9.2 addToParameter command

```plaintext
addToParameter (tag, <specific parameter args>)
```

In case that more objects (e.g., element, section) are mapped to an existing parameter, the command can be used to relate these additional objects to the specific parameter.
**1.9.3 updateParameter command**

updateParameter *(tag, newValue)*

Once the parameters in FE model are defined, their value can be updated.

<table>
<thead>
<tr>
<th>tag (int)</th>
<th>integer tag identifying the parameter.</th>
</tr>
</thead>
<tbody>
<tr>
<td>newValue (float)</td>
<td>the updated value to which the parameter needs to be set.</td>
</tr>
</tbody>
</table>


**1.9.4 setParameter command**

setParameter *('-val', newValue', '-ele', *eleTags [', '-eleRange', start, end [', *args ]*)

set value for an element parameter

<table>
<thead>
<tr>
<th>newValue (float)</th>
<th>the updated value to which the parameter needs to be set.</th>
</tr>
</thead>
<tbody>
<tr>
<td>eleTags (list (int))</td>
<td>a list of element tags</td>
</tr>
<tr>
<td>start (int)</td>
<td>start element tag</td>
</tr>
<tr>
<td>end (int)</td>
<td>end element tag</td>
</tr>
<tr>
<td>args (list (str))</td>
<td>a list of strings for the element parameter</td>
</tr>
</tbody>
</table>

**1.9.5 getParamTags command**

ggetParamTags ()

Return a list of tags for all parameters

**1.9.6 getParamValue command**

ggetParamValue *(tag)*

Return the value of a parameter

<table>
<thead>
<tr>
<th>tag (int)</th>
<th>integer tag identifying the parameter.</th>
</tr>
</thead>
</table>

**1.9.7 computeGradients command**

computeGradients ()

This command is used to perform a sensitivity analysis. If the user wants to call this command, then the *'-computeByCommand'* should be set in the sensitivityAlgorithm command.

### 1.9. Sensitivity Commands

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1.9.8 sensitivityAlgorithm command

`sensitivityAlgorithm(type)`

This command is used to create a sensitivity algorithm.

<table>
<thead>
<tr>
<th>type (str)</th>
<th>the type of the sensitivity algorithm,</th>
</tr>
</thead>
<tbody>
<tr>
<td></td>
<td>• '-computeAtEachStep' automatically compute at the end of each step</td>
</tr>
<tr>
<td></td>
<td>• '-computeByCommand' compute by calling <code>computeGradients</code></td>
</tr>
</tbody>
</table>

1.9.9 sensNodeDisp command

`sensNodeDisp(nodeTag, dof, paramTag)`

Returns the current displacement sensitivity to a parameter at a specified node.

<table>
<thead>
<tr>
<th>nodeTag (int)</th>
<th>node tag</th>
</tr>
</thead>
<tbody>
<tr>
<td>dof (int)</td>
<td>specific dof at the node (1 through ndf)</td>
</tr>
<tr>
<td>paramTag (int)</td>
<td>parameter tag</td>
</tr>
</tbody>
</table>

1.9.10 sensNodeVel command

`sensNodeVel(nodeTag, dof, paramTag)`

Returns the current velocity sensitivity to a parameter at a specified node.

<table>
<thead>
<tr>
<th>nodeTag (int)</th>
<th>node tag</th>
</tr>
</thead>
<tbody>
<tr>
<td>dof (int)</td>
<td>specific dof at the node (1 through ndf)</td>
</tr>
<tr>
<td>paramTag (int)</td>
<td>parameter tag</td>
</tr>
</tbody>
</table>

1.9.11 sensNodeAccel command

`sensNodeAccel(nodeTag, dof, paramTag)`

Returns the current acceleration sensitivity to a parameter at a specified node.

<table>
<thead>
<tr>
<th>nodeTag (int)</th>
<th>node tag</th>
</tr>
</thead>
<tbody>
<tr>
<td>dof (int)</td>
<td>specific dof at the node (1 through ndf)</td>
</tr>
<tr>
<td>paramTag (int)</td>
<td>parameter tag</td>
</tr>
</tbody>
</table>

1.9.12 sensLambda command

`sensLambda(patternTag, paramTag)`

Returns the current load factor sensitivity to a parameter in a load pattern.

<table>
<thead>
<tr>
<th>patternTag (int)</th>
<th>load pattern tag</th>
</tr>
</thead>
<tbody>
<tr>
<td>paramTag (int)</td>
<td>parameter tag</td>
</tr>
</tbody>
</table>
1.9.13 sensSectionForce command

`sensSectionForce(eleTag[, secNum ], dof, paramTag)`

Returns the current section force sensitivity to a parameter at a specified element and section.

<table>
<thead>
<tr>
<th>Parameter</th>
<th>Description</th>
</tr>
</thead>
<tbody>
<tr>
<td>eleTag (int)</td>
<td>element tag</td>
</tr>
<tr>
<td>secNum (int)</td>
<td>section number (optional)</td>
</tr>
<tr>
<td>dof (int)</td>
<td>specific dof at the element (1 through element force ndf)</td>
</tr>
<tr>
<td>paramTag (int)</td>
<td>parameter tag</td>
</tr>
</tbody>
</table>

1.9.14 sensNodePressure command

`sensNodePressure(nodeTag, paramTag)`

Returns the current pressure sensitivity to a parameter at a specified node.

<table>
<thead>
<tr>
<th>Parameter</th>
<th>Description</th>
</tr>
</thead>
<tbody>
<tr>
<td>nodeTag (int)</td>
<td>node tag</td>
</tr>
<tr>
<td>paramTag (int)</td>
<td>parameter tag</td>
</tr>
</tbody>
</table>

1.10 Reliability Commands

These commands are for reliability analysis in OpenSees.

1. `randomVariable command`

1.10.1 randomVariable command

`randomVariable(tag, dist, '-mean', mean, '-stdv', stdv, '-startPoint', startPoint, '-parameters', *params)`

Create a random variable with user specified distribution
<table>
<thead>
<tr>
<th>tag (int)</th>
<th>random variable tag</th>
</tr>
</thead>
<tbody>
<tr>
<td>dist (str)</td>
<td>random variable distribution</td>
</tr>
<tr>
<td></td>
<td>• 'normal'</td>
</tr>
<tr>
<td></td>
<td>• 'lognormal'</td>
</tr>
<tr>
<td></td>
<td>• 'gamma'</td>
</tr>
<tr>
<td></td>
<td>• 'shiftedExponential'</td>
</tr>
<tr>
<td></td>
<td>• 'shiftedRayleigh'</td>
</tr>
<tr>
<td></td>
<td>• 'exponential'</td>
</tr>
<tr>
<td></td>
<td>• 'rayleigh'</td>
</tr>
<tr>
<td></td>
<td>• 'uniform'</td>
</tr>
<tr>
<td></td>
<td>• 'beta'</td>
</tr>
<tr>
<td></td>
<td>• 'type1LargestValue'</td>
</tr>
<tr>
<td></td>
<td>• 'type1SmallestValue'</td>
</tr>
<tr>
<td></td>
<td>• 'type2LargestValue'</td>
</tr>
<tr>
<td></td>
<td>• 'type3SmallestValue'</td>
</tr>
<tr>
<td></td>
<td>• 'chiSquare'</td>
</tr>
<tr>
<td></td>
<td>• 'gumbel'</td>
</tr>
<tr>
<td></td>
<td>• 'weibull'</td>
</tr>
<tr>
<td></td>
<td>• 'laplace'</td>
</tr>
<tr>
<td></td>
<td>• 'pareto'</td>
</tr>
</tbody>
</table>

| mean (float) | mean value |
| stdv (float) | standard deviation |
| startPoint (float) | starting point of the distribution |
| params (list (int)) | a list of parameter tags |

### 1.11 Parallel Commands

The parallel commands are currently only working in the Linux version. The parallel OpenSeesPy is similar to OpenSeesMP, which requires users to divide the model to distributed processors.

You can still run the single-processor version as before. To run the parallel version, you have to install a MPI implementation, such as mpich. Then call your python scripts in the command line

```
mpiexec -np np python filename.py
```

where `np` is the number of processors to be used, `python` is the python interpreter, and `filename.py` is the script name.

Inside the script, OpenSeesPy is still imported as

```
import openseespy.opensees as ops
```

Common problems:

1. Unmatch send/recv will cause deadlock.
2. Writing to the same files at the same from different processors will cause race conditions.
3. Poor model decomposition will cause load imbalance problem.

Following are commands related to parallel computing:

1. `getPID` command
2. `getNP` command
3. **barrier command**
4. **send command**
5. **recv command**
6. **Bcast command**
7. **setStartNodeTag command**
8. **domainChange command**
9. **Parallel Plain Numberer**
10. **Parallel RCM Numberer**
11. **MUMPS Solver**
12. **Parallel DisplacementControl**

### 1.11.1 getPID command

**getPID()**

Get the processor ID of the calling processor.

### 1.11.2 getNP command

**getNP()**

Get total number of processors.

### 1.11.3 barrier command

**barrier()**

Set a barrier for all processors, i.e., faster processors will pause here to wait for all processors to reach to this point.

### 1.11.4 send command

**send ('-pid', pid, *data)**

Send information to another processor.

<table>
<thead>
<tr>
<th>pid (int)</th>
<th>ID of processor where data is sent to</th>
</tr>
</thead>
<tbody>
<tr>
<td>data (list (int))</td>
<td>can be a list of integers</td>
</tr>
<tr>
<td>data (list (float))</td>
<td>can be a list of floats</td>
</tr>
<tr>
<td>data (str)</td>
<td>can be a string</td>
</tr>
</tbody>
</table>

**Note:** send command and recv command must match and the order of calling both commands matters.
1.11.5 recv command

recv (\'pid\', pid)
Receive information from another processor.

<table>
<thead>
<tr>
<th>pid (int)</th>
<th>ID of processor where data is received from</th>
</tr>
</thead>
<tbody>
<tr>
<td>pid (str)</td>
<td>if pid is 'ANY', the processor can receive data from any processor.</td>
</tr>
</tbody>
</table>

Note: send command and recv command must match and the order of calling both commands matters.

1.11.6 Bcast command

Bcast (data)
Broadcast information from processor 0 to all processors.

<table>
<thead>
<tr>
<th>data (list (int))</th>
<th>can be a list of integers</th>
</tr>
</thead>
<tbody>
<tr>
<td>data (list (float))</td>
<td>can be a list of floats</td>
</tr>
<tr>
<td>data (str)</td>
<td>can be a string</td>
</tr>
</tbody>
</table>

1.11.7 setStartNodeTag command

setStartNodeTag (ndtag)
Set the starting node tag for the mesh command. The purpose of this command is to control the node tags generated by the mesh command. Some nodes are shared by processors, which must have same tags. Nodes which are unique to a processor must have uniques tags across all processors.

| ndtag (int) | starting node tag for the next call of mesh command |

1.11.8 domainChange command

domainChange()
Mark the domain has changed manually. This is used to notify processors whose domain is not changed, but the domain in other processors have changed.

1.12 Examples

1. Structural Examples
2. Earthquake Examples
3. Tsunami Examples
4. GeoTechnical Examples
5. Thermal Examples
6. Parallel Examples
1.12.1 Structural Examples

1. *Elastic Truss Analysis*
2. *Nonlinear Truss Analysis*
3. *Portal Frame 2d Analysis*
4. *Moment Curvature Analysis*
5. *Reinforced Concrete Frame Gravity Analysis*
6. *Reinforced Concrete Frame Pushover Analysis*
7. *Three story steel building with rigid beam-column connections and W-section*

**Elastic Truss Analysis**

1. The source code is shown below, which can be downloaded [here](#).
2. Run the source code in your favorite Python program and should see `Passed!` in the results.

```python
from openseespy.opensees import *
import numpy as np
import matplotlib.pyplot as plt

# ------------------------------
# Start of model generation
# -----------------------------

# remove existing model
wipe()

# set modelbuilder
model('basic', '-ndm', 2, '-ndf', 2)

# create nodes
node(1, 0.0, 0.0)
node(2, 144.0, 0.0)
node(3, 168.0, 0.0)
node(4, 72.0, 96.0)

# set boundary condition
fix(1, 1, 1)
fix(2, 1, 1)
fix(3, 1, 1)

# define materials
uniaxialMaterial("Elastic", 1, 3000.0)

# define elements
element("Truss",1,1,4,10.0,1)
element("Truss",2,2,4,5.0,1)
element("Truss",3,3,4,5.0,1)

# create TimeSeries
timeSeries("Linear", 1)
```

(continues on next page)
Nonlinear Truss Analysis

1. The source code is shown below, which can be downloaded here.

2. Make sure the numpy and matplotlib packages are installed in your Python distribution.

3. Run the source code in your favorite Python program and should see
```python
from openseespy.opensees import *
import numpy as np
import matplotlib.pyplot as plt

# ------------------------------
# Start of model generation
# -----------------------------

# set modelbuilder
wipe()
model('basic', '-ndm', 2, '-ndf', 2)

# variables
A = 4.0
E = 29000.0
alpha = 0.05
sY = 36.0
udisp = 2.5
Nsteps = 1000
Px = 160.0
Py = 0.0

# create nodes
node(1, 0.0, 0.0)
```

(continues on next page)
26 node(2, 72.0, 0.0)  
27 node(3, 168.0, 0.0)  
28 node(4, 48.0, 144.0)  
29  
30 # set boundary condition  
31 fix(1, 1, 1)  
32 fix(2, 1, 1)  
33 fix(3, 1, 1)  
34  
35 # define materials  
36 uniaxialMaterial("Hardening", 1, E, sY, 0.0, alpha/(1-alpha)*E)  
37  
38 # define elements  
39 element("Truss",1,1,4,A,1)  
40 element("Truss",2,2,4,A,1)  
41 element("Truss",3,3,4,A,1)  
42  
43 # create TimeSeries  
44 timeSeries("Linear", 1)  
45  
46 # create a plain load pattern  
47 pattern("Plain", 1, 1)  
48  
49 # Create the nodal load  
50 load(4, Px, Py)  
51  
52 # ----------------------------------  
53 # Start of analysis generation  
54 # ----------------------------------  
55  
56 # create SOE  
57 system("ProfileSPD")  
58  
59 # create DOF number  
60 numberer("Plain")  
61  
62 # create constraint handler  
63 constraints("Plain")  
64  
65 # create integrator  
66 integrator("LoadControl", 1.0/Nsteps)  
67  
68 # create algorithm  
69 algorithm("Newton")  
70  
71 # create test  
72 test('NormUnbalance',1e-8, 10)  
73  
74 # create analysis object  
75 analysis("Static")  
76  
77 # ------------------------------  
78 # Finally perform the analysis  
79 # ------------------------------  
80  
81 # perform the analysis  
82 data = np.zeros((Nsteps+1,2))
for j in range(Nsteps):
    analyze(1)
    data[j+1,0] = nodeDisp(4,1)
    data[j+1,1] = getLoadFactor(1)*Px
plt.plot(data[:,0], data[:,1])
plt.xlabel('Horizontal Displacement')
plt.ylabel('Horizontal Load')
plt.show()

Portal Frame 2d Analysis

1. The source code is shown below, which can be downloaded here.

2. Run the source code in your favorite Python program and should see results below

<table>
<thead>
<tr>
<th>Period</th>
<th>OpenSees</th>
<th>SAP2000</th>
<th>SeismoStruct</th>
</tr>
</thead>
<tbody>
<tr>
<td>1</td>
<td>1.27321</td>
<td>1.2732</td>
<td>1.2732</td>
</tr>
<tr>
<td>2</td>
<td>0.43128</td>
<td>0.4313</td>
<td>0.4313</td>
</tr>
<tr>
<td>3</td>
<td>0.24204</td>
<td>0.2420</td>
<td>0.2420</td>
</tr>
<tr>
<td>4</td>
<td>0.16018</td>
<td>0.1602</td>
<td>0.1602</td>
</tr>
<tr>
<td>5</td>
<td>0.11899</td>
<td>0.1190</td>
<td>0.1190</td>
</tr>
<tr>
<td>6</td>
<td>0.09506</td>
<td>0.0951</td>
<td>0.0951</td>
</tr>
<tr>
<td>7</td>
<td>0.07951</td>
<td>0.0795</td>
<td>0.0795</td>
</tr>
</tbody>
</table>

<table>
<thead>
<tr>
<th>Parameter</th>
<th>OpenSees</th>
<th>SAP2000</th>
<th>SeismoStruct</th>
</tr>
</thead>
<tbody>
<tr>
<td>Axial Force Bottom Left</td>
<td>69.987</td>
<td>69.99</td>
<td>70.01</td>
</tr>
<tr>
<td>Moment Bottom Left</td>
<td>2324.677</td>
<td>2324.68</td>
<td>2324.71</td>
</tr>
</tbody>
</table>

PASSED Verification Test PortalFrame2d.py

from openseespy.opensees import *
from math import asin, sqrt

# Two dimensional Frame: Eigenvalue & Static Loads

# REFERENCES:
# used in verification by SAP2000:
# SAP2000 Integrated Finite Element Analysis and Design of Structures, Verification Manual,
# and seismo-struct (Example 10)

# set some properties
wipe()
model('Basic', '-ndm', 2)
# properties
# units kip, ft

numBay = 2
numFloor = 7
bayWidth = 360.0
storyHeights = [162.0, 162.0, 156.0, 156.0, 156.0, 156.0, 156.0]
E = 29500.0
massX = 0.49
M = 0.
coordTransf = "Linear"  # Linear, PDelta, Corotational
massType = "-lMass"  # -lMass, -cMass

eColumn = ['W14X246', 'W14X246', 'W14X246', 'W14X211', 'W14X211', 'W14X176', 'W14X176']
iColumn = ['W14X287', 'W14X287', 'W14X287', 'W14X246', 'W14X246', 'W14X211', 'W14X211']
columns = [eColumn, iColumn, eColumn]

WSection = {
    'W14X176': [51.7, 2150.],
    'W14X211': [62.1, 2670.],
    'W14X246': [72.3, 3230.],
    'W14X287': [84.4, 3910.],
    'W24X110': [32.5, 3330.],
    'W24X130': [38.3, 4020.],
    'W24X160': [47.1, 5120.]
}

nodeTag = 1

# procedure to read
def ElasticBeamColumn(eleTag, iNode, jNode, sectType, E, transfTag, M, massType):
    found = 0
    prop = WSection[sectType]
    A = prop[0]
    I = prop[1]
    element('elasticBeamColumn', eleTag, iNode, jNode, A, E, I, transfTag, '-mass', M, massType)

# add the nodes
# - floor at a time
yLoc = 0.
for j in range(0, numFloor + 1):
    xLoc = 0.
    for i in range(0, numBay + 1):
        node(nodeTag, xLoc, yLoc)
xLoc += bayWidth
nodeTag += 1

if j < numFloor:
    storyHeight = storyHeights[j]
yLoc += storyHeight

# fix first floor
fix(1, 1, 1, 1)
fix(2, 1, 1, 1)
fix(3, 1, 1, 1)

# rigid floor constraint & masses
nodeTagR = 5
nodeTag = 4
for j in range(1, numFloor + 1):
    for i in range(0, numBay + 1):
        if nodeTag != nodeTagR:
            equalDOF(nodeTagR, nodeTag, 1)
        else:
            mass(nodeTagR, massX, 1.0e-10, 1.0e-10)

nodeTag += 1
nodeTagR += numBay + 1

# add the columns
# add column element
geomTransf(coordTransf, 1)
eleTag = 1
for j in range(0, numBay + 1):
    end1 = j + 1
    end2 = end1 + numBay + 1
    thisColumn = columns[j]
    for i in range(0, numFloor):
        secType = thisColumn[i]
        ElasticBeamColumn(eleTag, end1, end2, secType, E, 1, M, massType)
        end1 = end2
        end2 += numBay + 1
        eleTag += 1

# add beam elements
for j in range(1, numFloor + 1):
    end1 = (numBay + 1) * j + 1
    end2 = end1 + 1
    secType = beams[j - 1]
    for i in range(0, numBay):
        ElasticBeamColumn(eleTag, end1, end2, secType, E, 1, M, massType)
        end1 = end2
        end2 = end1 + 1
        eleTag += 1

# calculate eigenvalues & print results

1.12. Examples 243
numEigen = 7
eigenValues = eigen(numEigen)
PI = 2 * asin(1.0)

# apply loads for static analysis & perform analysis
#
timeSeries('Linear', 1)
pattern('Plain', 1, 1)
load(22, 20.0, 0., 0.)
load(19, 15.0, 0., 0.)
load(16, 12.5, 0., 0.)
load(13, 10.0, 0., 0.)
load(10, 7.5, 0., 0.)
load(7, 5.0, 0., 0.)
load(4, 2.5, 0., 0.)

integrator('LoadControl', 1.0)
algorith('Linear')
analysis('Static')
analyze(1)

# determine PASS/FAILURE of test
ok = 0

# print pretty output of comparisons
#
SAP2000   SeismoStruct
comparisonResults = [[1.2732, 0.4313, 0.2420, 0.1602, 0.1190, 0.0951, 0.0795],
                      [1.2732, 0.4313, 0.2420, 0.1602, 0.1190, 0.0951, 0.0795]]

print("\n\nPeriod Comparisons:")
print('  {:>10}{:>15.5f}{:>15.4f}{:>15.4f}{:>15.4f}{:>15.4f}{:>15.4f}
            '.format('Period','OpenSees','SAP2000','SeismoStruct'))

for i in range(0, numEigen):
    lamb = eigenValues[i]
    period = 2 * PI / sqrt(lamb)
    print('  {:>10}{:>15.5f}{:>15.4f}{:>15.4f}{:>15.4f}{:>15.4f}{:>15.4f}'.format(i + 1, period,
            comparisonResults[0][i], comparisonResults[1][i],
            resultOther = comparisonResults[0][i]
            if abs(period - resultOther) > 9.99e-5:
                ok - 1

# print table of comparision
# Parameter   SAP2000   SeismoStruct
comparisonResults = [['Disp Top', 'Axial Force Bottom Left', 'Moment Bottom Left'],
                      [1.45076, 69.99, 2324.68],
                      [1.451, 70.01, 2324.71]]

print("\n\nStatic Analysis Result Comparisons:\")
print('  {:>30}{:>15}{:>15}{:>15}'.format('Parameter','OpenSees','SAP2000','SeismoStruct'))
for i in range(3):
    response = eleResponse(1, 'forces')
    if i == 0:
        result = nodeDisp(22, 1)
    elif i == 1:
        result = abs(response[1])
    else:
        result = response[2]
    print('{:>30}{:>15.3f}{:>15.2f}{:>15.2f}'.format(comparisonResults[0][i],
                                                    result,
                                                    comparisonResults[1][i],
                                                    comparisonResults[2][i]))

resultOther = comparisonResults[1][i]
tol = tolerances[i]
if abs(result - resultOther) > tol:
    ok = 1
    print("failed-> ", i, abs(result - resultOther), tol)
else:
    if ok == 0:
        print("PASSED Verification Test PortalFrame2d.py \n\n")
    else:
        print("FAILED Verification Test PortalFrame2d.py \n\n")

Moment Curvature Analysis

1. The source code is shown below, which can be downloaded here.
2. Run the source code in your favorite Python program and should see results below

Start MomentCurvature.py example
Estimated yield curvature: 0.000126984126984127
Passed!

from openseespy.opensees import *

def MomentCurvature(secTag, axialLoad, maxK, numIncr=100):
    node(1, 0.0, 0.0)
    node(2, 0.0, 0.0)
    fix(1, 1, 1, 1)
    fix(2, 0, 1, 0)

    element('zeroLengthSection', 1, 1, 2, secTag)

    timeSeries('Constant', 1)
    pattern('Plain', 1, 1)
# Define analysis parameters
integrator('LoadControl', 0.0)
system('SparseGeneral', '-piv')
test('NormUnbalance', 1e-9, 10)
numberer('Plain')
constraints('Plain')
algorithm('Plain')
analysis('Static')

# Do one analysis for constant axial load
analyze(1)

# Define reference moment
timeSeries('Linear', 2)
pattern('Plain', 2)
load(2, 0.0, 0.0, 1.0)

# Compute curvature increment
dK = maxK / numIncr

# Use displacement control at node 2 for section analysis
integrator('DisplacementControl', 2, 3, dK, 1, dK, dK)

# Do the section analysis
analyze(numIncr)

wipe()
print("Start MomentCurvature.py example")

# Define model builder
# ---------------------
model('basic', '-ndm', 2, '-ndf', 3)

# Define materials for nonlinear columns
# --------------------------------------
# CONCRETE tag f'c ec0 f'cu ecu
# Core concrete (confined)
uniaxialMaterial('Concrete01', 1, -6.0, -0.004, -5.0, -0.014)

# Cover concrete (unconfined)
uniaxialMaterial('Concrete01', 2, -5.0, -0.002, 0.0, -0.006)

# STEEL
# Reinforcing steel
fy = 60.0  # Yield stress
E = 30000.0  # Young's modulus

# tag fy E0 b
uniaxialMaterial('Steel01', 3, fy, E, 0.01)

# Define cross-section for nonlinear columns
# -----------------------------------------
# set some parameters
colWidth = 15
colDepth = 24

cover = 1.5
As = 0.60;  # area of no. 7 bars

# some variables derived from the parameters
y1 = colDepth/2.0
z1 = colWidth/2.0

section('Fiber', 1)

# Create the concrete core fibers
patch('rect',1,10,1 ,cover-y1, cover-z1, y1-cover, z1-cover)

# Create the concrete cover fibers (top, bottom, left, right)
patch('rect',2,10,1 ,-y1, z1-cover, y1, z1)
patch('rect',2,10,1 ,-y1, -z1, y1, cover-z1)
patch('rect',2,2,1 ,-y1, cover-z1, cover-y1, z1-cover)
patch('rect',2,2,1 ,y1-cover, cover-z1, y1, z1-cover)

# Create the reinforcing fibers (left, middle, right)
layer('straight', 3, 3, As, y1-cover, z1-cover, y1-cover, cover-z1)
layer('straight', 3, 2, As, 0.0 , z1-cover, 0.0 , cover-z1)
layer('straight', 3, 3, As, cover-y1, z1-cover, cover-y1, cover-z1)

# Estimate yield curvature
# (Assuming no axial load and only top and bottom steel)
# d -- from cover to rebar
d = colDepth-cover
epsy = fy/E
Ky = epsy/(0.7*d)

# Print estimate to standard output
print("Estimated yield curvature: ", Ky)

# Set axial load
P = -180.0

# Target ductility for analysis
mu = 15.0

# Number of analysis increments
numIncr = 100

# Call the section analysis procedure
MomentCurvature(1, P, Ky*mu, numIncr)

results = open('results.out','a+')
u = nodeDisp(2,3)
if abs(u-0.00190476190476190541)<1e-12:
    results.write('PASSED : MomentCurvature.py\n');
    print("Passed!")
else:
134 results.write('FAILED : MomentCurvature.py
');
135 print("Failed!")
136 results.close()
137 print("=================================")

Reinforced Concrete Frame Gravity Analysis

1. The source code is shown below, which can be downloaded here.
2. Run the source code in your favorite Python program and should see Passed! in the results.

```python
from openseespy.opensees import *

print("Starting RCFrameGravity example")

# Create ModelBuilder (with two-dimensions and 3 DOF/node)
model('basic', '-ndm', 2, '-ndf', 3)

# Create nodes
# ------------
node(1, 0.0, 0.0)
node(2, width, 0.0)
node(3, 0.0, height)
node(4, width, height)

# Fix supports at base of columns
# tag, DX, DY, RZ
fix(1, 1, 1, 1)
fix(2, 1, 1, 1)

# Define materials for nonlinear columns
# ------------------------------------------
# CONCRETE tag f’c ec0 f’cu ecu
# Core concrete (confined)
uniaxialMaterial('Concrete01', 1, -6.0, -0.004, -5.0, -0.014)

# Cover concrete (unconfined)
uniaxialMaterial('Concrete01', 2, -5.0, -0.002, 0.0, -0.006)

# STEEL
# Reinforcing steel
fy = 60.0;  # Yield stress
E = 30000.0;  # Young’s modulus
# tag fy E0 b
```

(continues on next page)
uniaxialMaterial('Steel01', 3, fy, E, 0.01)

# Define cross-section for nonlinear columns
# -----------------------------

# some parameters
colWidth = 15
colDepth = 24
cover = 1.5
As = 0.60  # area of no. 7 bars

# some variables derived from the parameters
y1 = colDepth / 2.0
z1 = colWidth / 2.0
section('Fiber', 1)

# Create the concrete core fibers
patch('rect', 1, 10, 1, cover - y1, cover - z1, y1 - cover, z1 - cover)

# Create the concrete cover fibers (top, bottom, left, right)
patch('rect', 2, 10, 1, -y1, z1 - cover, y1, z1)
patch('rect', 2, 10, 1, -y1, -z1, y1, cover - z1)
patch('rect', 2, 2, 1, -y1, cover - z1, cover - y1, z1 - cover)
patch('rect', 2, 2, 1, y1 - cover, cover - z1, y1, z1 - cover)

# Create the reinforcing fibers (left, middle, right)
layer('straight', 3, 3, As, y1 - cover, z1 - cover, y1 - cover, z1 - cover)
layer('straight', 3, 2, As, 0.0, z1 - cover, 0.0, cover - z1)
layer('straight', 3, 3, As, cover - y1, z1 - cover, cover - y1, z1 - cover)

# Define column elements
# ----------------------

# Geometry of column elements
# tag
gemTransf('PDelta', 1)

# Number of integration points along length of element
np = 5

# Lobatto integration
beamIntegration('Lobatto', 1, 1, np)

# Create the columns using Beam-column elements
# e tag ndI ndJ transfTag integrationTag
eleType = 'forceBeamColumn'
element(eleType, 1, 1, 3, 1, 1)
element(eleType, 2, 2, 4, 1, 1)

# Define beam element
# ----------------------------

# Geometry of column elements
# tag
geomTransf('Linear', 2)

# Create the beam element
# 
tag, ndI, ndJ, A, E, Iz, transfTag
element('elasticBeamColumn', 3, 3, 4, 360.0, 4030.0, 8640.0, 2)

# Define gravity loads
# ---------------------

# a parameter for the axial load
P = 180.0;  # 10% of axial capacity of columns

# Create a Plain load pattern with a Linear TimeSeries
timeSeries('Linear', 1)
pattern('Plain', 1, 1)

# Create nodal loads at nodes 3 & 4
# nd FX, FY, MZ
load(3, 0.0, -P, 0.0)
load(4, 0.0, -P, 0.0)

# --------------------------------------------
# End of model generation
# --------------------------------------------

# --------------------------------------------
# Start of analysis generation
# --------------------------------------------

# Create the system of equation, a sparse solver with partial pivoting
system('BandGeneral')

# Create the constraint handler, the transformation method
constraints('Transformation')

# Create the DOF numberer, the reverse Cuthill-McKee algorithm
numberer('RCM')

# Create the convergence test, the norm of the residual with a tolerance of
# 1e-12 and a max number of iterations of 10
test('NormDispIncr', 1.0e-12, 10, 3)

# Create the solution algorithm, a Newton-Raphson algorithm
algorithm('Newton')

# Create the integration scheme, the LoadControl scheme using steps of 0.1
integrator('LoadControl', 0.1)

# Create the analysis object
analysis('Static')

# --------------------------------------------
# End of analysis generation
# --------------------------------------------
# Finally perform the analysis

# perform the gravity load analysis, requires 10 steps to reach the load level
analyze(10)

# Print out the state of nodes 3 and 4
# print node 3 4

# Print out the state of element 1
# print ele 1

u3 = nodeDisp(3, 2)
u4 = nodeDisp(4, 2)

results = open('results.out', 'a+')

if abs(u3 + 0.0183736) < 1e-6 and abs(u4 + 0.0183736) < 1e-6:
    results.write('PASSED : RCFrameGravity.py

    Passed!')
else:
    results.write('FAILED : RCFrameGravity.py

    Failed!')

results.close()

"""

Reinforced Concrete Frame Pushover Analysis

1. The source code is shown below, which can be downloaded [here](#).  
2. The file for gravity analysis is also needed: [here](#).  
3. Run the source code in your favorite Python program and should see Passed! in the results.

print("================================================")
print("Start RCFramePushover Example")

# Units: kips, in, sec
#
# Written: GLF/MHS/fmk
# Date: January 2001
from openseespy.opensees import *

wipe()

# Start of Model Generation & Initial Gravity Analysis
#
# Do operations of Example3.1 by sourcing in the tcl file
import RCFrameGravity
print("Gravity Analysis Completed")

# Set the gravity loads to be constant & reset the time in the domain

(continues on next page)
loadConst('-time', 0.0)

# End of Model Generation & Initial Gravity Analysis
# ----------------------------------------------------

# Start of additional modelling for lateral loads
# ----------------------------------------------------

# Define lateral loads
# ----------------------

# Set some parameters
H = 10.0 # Reference lateral load

# Set lateral load pattern with a Linear TimeSeries
pattern('Plain', 2, 1)

# Create nodal loads at nodes 3 & 4
# nd FX FY MZ
load(3, H, 0.0, 0.0)
load(4, H, 0.0, 0.0)

# End of additional modelling for lateral loads
# ----------------------------------------------------

# Start of modifications to analysis for push over
# ----------------------------------------------------

# Set some parameters
dU = 0.1 # Displacement increment

# Change the integration scheme to be displacement control
# node dof init Jd min max
integrator('DisplacementControl', 3, 1, dU, 1, dU, dU)

# End of modifications to analysis for push over
# ----------------------------------------------------

# Start of recorder generation
# ------------------------------

# Stop the old recorders by destroying them
# remove recorders

# Create a recorder to monitor nodal displacements
# recorder Node -file node32.out -time -node 3 4 -dof 1 2 3 disp

# Create a recorder to monitor element forces in columns

(continues on next page)
# recorder EnvelopeElement -file ele32.out -time -ele 1 2 forces
#
# --------------------------------
# End of recorder generation
# --------------------------------
#
# --------------------------------
# Finally perform the analysis
# --------------------------------
#
# Set some parameters
maxU = 15.0  # Max displacement
currentDisp = 0.0
ok = 0

test('NormDispIncr', 1.0e-12, 1000)
algorith('ModifiedNewton', '-initial')

while ok == 0 and currentDisp < maxU:
    ok = analyze(1)
    # if the analysis fails try initial tangent iteration
    if ok != 0:
        print("modified newton failed")
        break
    # print "regular newton failed .. lets try an initail stiffness for this step"
    # test('NormDispIncr', 1.0e-12, 1000)
    # # algorithm('ModifiedNewton', '-initial')
    # ok = analyze(1)
    # if ok == 0:
    #     print "that worked .. back to regular newton"
    currentDisp = nodeDisp(3, 1)

results = open('results.out', 'a+')

if ok == 0:
    results.write("PASSED : RCFramePushover.py")
    print("Passed!")
else:
    results.write("FAILED : RCFramePushover.py")
    print("Failed!")

results.close()

# Print the state at node 3
# print node 3

print("================================")
Three story steel building with rigid beam-column connections and W-section

1. The source code is developed by Anurag Upadhyay from University of Utah.
2. The source code is shown below, which can be downloaded here.
3. Run the source code in your favorite Python program and should see following plot.

```python
# 2D steel frame example.
# 3 story steel building with rigid beam-column connections.
# This script uses W-section command inOpensees to create steel beam-column fiber sections.
#
# By - Anurag Upadhyay, PhD Student, University of Utah.
# Date - 08/06/2018
#
print("=================================")
print("Start 2D Steel Frame Example")
from openseespy.opensees import *
import numpy as np
import matplotlib.pyplot as plt
```
import os

AnalysisType='Pushover' ;  
    # Pushover Gravity

# Start of model generation
# remove existing model
wipe()

# set modelbuilder
model('basic', '-ndm', 2, '-ndf', 3)

import math

############################################
### Units and Constants  ###################
############################################

inch = 1;
kilopound = 1;
second = 1;

# Dependent units
sq_in = inch*inch;
ksi = kilopound/sq_in;
foot = 12*inch;

# Constants
gravity = 386.2*inch/(second*second);
pi = math.acos(-1);

#######################################
##### Dimensions  
#######################################

# Dimensions Input
H_story=10.0*foot;
W_bayX=16.0*foot;
W_bayY_ab=5.0*foot+10.0*inch;
W_bayY_bc=8.0*foot+4.0*inch;
W_bayY_cd=5.0*foot+10.0*inch;

# Calculated dimensions
W_structure=W_bayY_ab+W_bayY_bc+W_bayY_cd;

###########################
### Material  
###########################

# Steel02 Material
matTag=1;
matConnAx=2;
matConnRot=3;

Fy=60.0*ksi;  
    # Yield stress
Es=29000.0*ksi; # Modulus of Elasticity of Steel
v=0.2; # Poisson's ratio
Gs=Es/(1+v); # Shear modulus
b=0.10; # Strain hardening ratio
params=[18.0,0.925,0.15] # R0,cR1,cR2
R0=18.0
cR1=0.925
cR2=0.15
a1=0.05
a2=1.00
a3=0.05
a4=1.0
sigInit=0.0
alpha=0.05

uniaxialMaterial('Steel02', matTag, Fy, Es, b, R0, cR1, cR2, a1, a2, a3, a4, sigInit)

# COMMAND: section('WFSection2d', secTag, matTag, d, tw, bf, tf, Nfw, Nff)
section('WFSection2d', colSecTag1, matTag, 10.5*inch, 0.26*inch, 5.77*inch, 0.44*inch, 15, 16) # outer Column
section('WFSection2d', colSecTag2, matTag, 10.5*inch, 0.26*inch, 5.77*inch, 0.44*inch, 15, 16) # Inner Column
section('WFSection2d', beamSecTag1, matTag, 8.3*inch, 0.44*inch, 8.11*inch, 0.685*inch, 15, 15) # outer Beam
section('WFSection2d', beamSecTag2, matTag, 8.2*inch, 0.40*inch, 8.01*inch, 0.650*inch, 15, 15) # Inner Beam
section('WFSection2d', beamSecTag3, matTag, 8.0*inch, 0.40*inch, 7.89*inch, 0.600*inch, 15, 15) # Inner Beam

# Beam size - W10x26
Abeam=7.61*inch*inch;
IbeamY=144.*(inch**4); # Inertia along horizontal axis
IbeamZ=14.1*(inch**4); # Inertia along vertical axis

# BRB input data
Acore=2.25*inch;
Aend=10.0*inch;
LR_BRB=0.55;

# ###################################
# #### Nodes
# ###################################
# Create All main nodes
node(1, 0.0, 0.0)
node(2, W_bayX, 0.0)
node(3, 2*W_bayX, 0.0)
node(11, 0.0, H_story)
node(12, W_bayX, H_story)
node(13, 2*W_bayX, H_story)
node(21, 0.0, 2*H_story)
node(22, W_bayX, 2*H_story)
node(23, 2*W_bayX, 2*H_story)
node(31, 0.0, 3*H_story)
node(32, W_bayX, 3*H_story)
node(33, 2*W_bayX, 3*H_story)

# Beam Connection nodes
node(1101, 0.0, H_story)
node(1201, W_bayX, H_story)
node(1202, W_bayX, H_story)
node(1301, 2*W_bayX, H_story)
node(2101, 0.0, 2*H_story)
node(2201, W_bayX, 2*H_story)
node(2202, W_bayX, 2*H_story)
node(2301, 2*W_bayX, 2*H_story)
node(3101, 0.0, 3*H_story)
node(3201, W_bayX, 3*H_story)
node(3202, W_bayX, 3*H_story)
node(3301, 2*W_bayX, 3*H_story)

# Constraints
fix(1, 1, 1, 1)
fix(2, 1, 1, 1)
fix(3, 1, 1, 1)

# Elements
ColIntTag1=1;
ColIntTag2=2;
BeamIntTag1=3;
BeamIntTag2=4;
BeamIntTag3=5;
beamIntegration('Lobatto', ColIntTag1, colSecTag1, 4)
beamIntegration('Lobatto', ColIntTag2, colSecTag2, 4)
beamIntegration('Lobatto', BeamIntTag1, beamSecTag1, 4)
beamIntegration('Lobatto', BeamIntTag2, beamSecTag2, 4)
beamIntegration('Lobatto', BeamIntTag3, beamSecTag3, 4)
# Assign geometric transformation

ColTransfTag=1
BeamTranfTag=2

geomTransf('PDelta', ColTransfTag)
geomTransf('Linear', BeamTranfTag)

# Assign Elements ####################

# ## Add non-linear column elements

element('forceBeamColumn', 1, 1, 11, ColTransfTag, ColIntTag1, '-mass', 0.0)
element('forceBeamColumn', 2, 2, 12, ColTransfTag, ColIntTag2, '-mass', 0.0)
element('forceBeamColumn', 3, 3, 13, ColTransfTag, ColIntTag1, '-mass', 0.0)
element('forceBeamColumn', 11, 11, 21, ColTransfTag, ColIntTag1, '-mass', 0.0)
element('forceBeamColumn', 12, 12, 22, ColTransfTag, ColIntTag2, '-mass', 0.0)
element('forceBeamColumn', 13, 13, 23, ColTransfTag, ColIntTag1, '-mass', 0.0)
element('forceBeamColumn', 21, 21, 31, ColTransfTag, ColIntTag1, '-mass', 0.0)
element('forceBeamColumn', 22, 22, 32, ColTransfTag, ColIntTag2, '-mass', 0.0)
element('forceBeamColumn', 23, 23, 33, ColTransfTag, ColIntTag1, '-mass', 0.0)

# ### Add linear main beam elements, along x-axis

element('elasticBeamColumn', 101, 1101, 1201, Abeam, Es, Gs, Jbeam, IbeamY, IbeamZ, beamTranfTag, '-mass', 0.0)

element('forceBeamColumn', 101, 1101, 1201, BeamTranfTag, BeamIntTag1, '-mass', 0.0)
element('forceBeamColumn', 102, 1202, 1301, BeamTranfTag, BeamIntTag1, '-mass', 0.0)
element('forceBeamColumn', 201, 2101, 2201, BeamTranfTag, BeamIntTag2, '-mass', 0.0)
element('forceBeamColumn', 202, 2202, 2301, BeamTranfTag, BeamIntTag2, '-mass', 0.0)
element('forceBeamColumn', 301, 3101, 3201, BeamTranfTag, BeamIntTag3, '-mass', 0.0)
element('forceBeamColumn', 302, 3202, 3301, BeamTranfTag, BeamIntTag3, '-mass', 0.0)

# Assign constraints between beam end nodes and column nodes (RIGID beam column connections)
equalDOF(11, 1101, 1,2,3)
equalDOF(12, 1201, 1,2,3)
equalDOF(12, 1202, 1,2,3)
equalDOF(13, 1301, 1,2,3)
equalDOF(21, 2101, 1,2,3)
equalDOF(22, 2201, 1,2,3)
equalDOF(22, 2202, 1,2,3)
equalDOF(23, 2301, 1,2,3)
equalDOF(31, 3101, 1,2,3)
equalDOF(32, 3201, 1,2,3)
equalDOF(32, 3202, 1,2,3)
equalDOF(33, 3301, 1,2,3)

(continues on next page)
# Gravity Load

# create TimeSeries
timeSeries("Linear", 1)

# create a plain load pattern
pattern("Plain", 1, 1)

# Create the nodal load
load(11, 0.0, -5.0*kip, 0.0)
load(12, 0.0, -6.0*kip, 0.0)
load(13, 0.0, -5.0*kip, 0.0)
load(21, 0., -5.*kip, 0.0)
load(22, 0., -6.*kip, 0.0)
load(23, 0., -5.*kip, 0.0)
load(31, 0., -5.*kip, 0.0)
load(32, 0., -6.*kip, 0.0)
load(33, 0., -5.*kip, 0.0)

# Start of analysis generation

NstepsGrav = 10
system("BandGEN")
numberer("Plain")
constraints("Plain")
integrator("LoadControl", 1.0/NstepsGrav)
algorithm("Newton")
test('NormUnbalance',1e-8, 10)
analysis("Static")

# perform the analysis
data = np.zeros((NstepsGrav+1,2))
for j in range(NstepsGrav):
    analyze(1)
data[j+1,0] = nodeDisp(31,2)
data[j+1,1] = getLoadFactor(1)*5
loadConst('-time', 0.0)
print("Gravity analysis complete")
wipeAnalysis()

1.12. Examples

if(AnalysisType=="Pushover"):
print("<<< Running Pushover Analysis >>>")

# Create load pattern for pushover analysis
# create a plain load pattern
pattern("Plain", 2, 1)

load(11, 1.61, 0.0, 0.0)
load(21, 3.22, 0.0, 0.0)
load(31, 4.83, 0.0, 0.0)

ControlNode=31
ControlDOF=1
MaxDisp=0.15*H_story
DispIncr=0.1
NstepsPush=int(MaxDisp/DispIncr)

system("ProfileSPD")
numberer("Plain")
constraints("Plain")
integrator("DisplacementControl", ControlNode, ControlDOF, DispIncr)
algorithim("Newton")
test('NormUnbalance', 1e-8, 10)
analysis("Static")

PushDataDir = r'PushoverOut'
if not os.path.exists(PushDataDir):
    os.makedirs(PushDataDir)
    recorder('Node', '-file', "PushoverOut/Node2React.out", '-closeOnWrite', '-node', 2, '-dof', 1, 'reaction')
    recorder('Node', '-file', "PushoverOut/Node31Disp.out", '-closeOnWrite', '-node', 31, '-dof', 1, 'disp')
    recorder('Element', '-file', "PushoverOut/BeamStress.out", '-closeOnWrite', '-ele', 102, 'section', '4', 'fiber', '1', 'stressStrain')

    # analyze (NstepsPush)

    # Perform pushover analysis
    dataPush = np.zeros((NstepsPush+1,5))
    for j in range(NstepsPush):
        analyze(1)
        dataPush[j+1,0] = nodeDisp(31,1)
        reactions()
        dataPush[j+1,1] = nodeReaction(1, 1) + nodeReaction(2, 1) + nodeReaction(3, 1)

        plt.plot(dataPush[:,0], -dataPush[:,1])
        plt.xlim(0, MaxDisp)
        plt.xticks(np.linspace(0, MaxDisp, 5, endpoint=True))
        plt.yticks(np.linspace(0, -int(dataPush[NstepsPush,1]), 10, endpoint=True))
        plt.grid(linestyle='dotted')
        plt.xlabel('Top Displacement (inch)')
        plt.ylabel('Base Shear (kip)')
        plt.show()

    print("Pushover analysis complete")
1.12.2 Earthquake Examples

1. Cantilever 2D EQ ground motion with gravity Analysis
2. Reinforced Concrete Frame Earthquake Analysis
3. Example name spaced nonlinear SDOF
4. RotD Spectra of Ground Motion
5. Portal 2D Frame - Dynamic EQ Ground Motion
6. 2D Column - Dynamic EQ Ground Motion
7. Nonlinear Canti Col Uniaxial Inelastic Section- Dyn EQ GM
8. Nonlin Canti Col Inelstc Uniaxial Mat in Fiber Sec - Dyn EQ
9. Cantilever 2D Column with Units- Dynamic EQ Ground Motion
10. Cantilever 2D Column with Units- Static Pushover
11. 2D Portal Frame with Units- Dynamic EQ Ground Motion
12. 2D Portal Frame with Units- Multiple Support Dynamic EQ Ground Motion-acctimeseries
13. 2D Portal Frame with Units- Multiple Support Dynamic EQ Ground Motion-disptimeseries
14. 2D Portal Frame with Units- Uniform Dynamic EQ -bidirectional-acctimeseries
15. ExampleFRP

Cantilever 2D EQ ground motion with gravity Analysis

1. The source code is shown below, which can be downloaded here.
2. The ground motion data file here must be put in the same folder.
3. Run the source code in your favorite Python program and should see results below

```python
print("=========================================================")
print("Start cantilever 2D EQ ground motion with gravity example")
u2 = -0.07441860465116278
Passed!
print("=========================================================")
```

1.12. Examples
# Example 1. cantilever 2D
# EQ ground motion with gravity
# all units are in kip, inch, second
# elasticBeamColumn ELEMENT
#
# ^Y
# |
# 2 __
# | |
# | |
# | 36'
# | |
# | |
# | |
# =1= ---- -------->X
#
#
# SET UP
# clear opensees model
wipe()
model('basic', '-ndm', 2, '-ndf', 3) # 2 dimensions, 3 dof per node
# create data directory
file mkdir data
# define GEOMETRY
node(1, 0., 0.) # node#, X Y
node(2, 0., 432.)
# Single point constraints -- Boundary Conditions
fix(1, 1, 1) # node DX DY RZ
# nodal masses:
mass(2, 5.18, 0., 0.) # node#, Mx My Mz, Mass=Weight/g.
# Define ELEMENTS
# define geometric transformation: performs a linear geometric transformation of beam
# stiffness and resisting force from the basic system to the global-coordinate system
geomTransf('Linear', 1) # associate a tag to transformation
# connectivity:
element('elasticBeamColumn', 1, 1, 2, 3600.0, 3225.0, 1080000.0, 1)
# define GRAVITY
timeSeries('Linear', 1)
pattern('Plain', 1) # node#, FX FY MZ -- superstructure-weight
load(2, 0., -2000., 0.) # how it handles boundary
# numberer('Plain') # renumber dof's to minimize band-width
# system('BandGeneral') # how to store and solve the system of
# equations in the analysis
algorithm('Linear') # use Linear algorithm for linear analysis
integrator('LoadControl', 0.1) # determine the next time step
--- for an analysis, # apply gravity in 10 steps (continues on next page)
### Reinforced Concrete Frame Earthquake Analysis

1. The source code is shown below, which can be downloaded here.
2. The file for gravity analysis is also needed here.
3. The ReadRecord is a useful python function for parsing the PEER strong motion data base files and returning the \( dt, nPnts \) and creating a file containing just data points. The function is kept in a separate file here and is imported in the example.
4. The ground motion data file here must be put in the same folder.
5. Run the source code in your favorite Python program and should see *Passed!* in the results and a plotting of displacement for node 3

```python
print("==========================")
print("Start RCFrameEarthquake Example")

# Units: kips, in, sec
#
# Written: Minjie

from openseespy.opensees import *
import ReadRecord
import numpy as np
import matplotlib.pyplot as plt

wipe()
# ----------------------------------------------------
# Start of Model Generation & Initial Gravity Analysis
# ----------------------------------------------------
import RCFrameGravity
print("Gravity Analysis Completed")

# Set the gravity loads to be constant & reset the time in the domain
```

(continues on next page)
loadConst('-time', 0.0)
# ----------------------------------------------------
# End of Model Generation & Initial Gravity Analysis
# ----------------------------------------------------
# Define nodal mass in terms of axial load on columns
# g = 386.4
# m = RCFrameGravity.P/g
mass(3, m, m, 0.0)
mass(4, m, m, 0.0)
# Set some parameters
record = 'elCentro'
# Perform the conversion from SMD record to OpenSees record
dt, nPts = ReadRecord.ReadRecord(record+'.at2', record+'.dat')
# Set time series to be passed to uniform excitation
timeSeries('Path', 2, '-filePath', record+'.dat', '-dt', dt, '-factor', g)
# Create UniformExcitation load pattern
# tag dir
tag dir
pattern('UniformExcitation', 2, 1, '-accel', 2)
# set the rayleigh damping factors for nodes & elements
rayleigh(0.0, 0.0, 0.0, 0.000625)
# Delete the old analysis and all its component objects
wipeAnalysis()
# Create the system of equation, a banded general storage scheme
system('BandGeneral')
# Create the constraint handler, a plain handler as homogeneous boundary
constraints('Plain')
# Create the convergence test, the norm of the residual with a tolerance of
# 1e-12 and a max number of iterations of 10
test('NormDispIncr', 1.0e-12, 10)
# Create the solution algorithm, a Newton-Raphson algorithm
algorithm('Newton')
# Create the DOF numberer, the reverse Cuthill-McKee algorithm
numberer('RCM')
# Create the integration scheme, the Newmark with alpha =0.5 and beta =.25
integrator('Newmark', 0.5, 0.25)
# Create the analysis object
analysis('Transient')
# Perform an eigenvalue analysis
numEigen = 2
eigenValues = eigen(numEigen)
print("eigen values at start of transient:", eigenValues)

# set some variables
tFinal = nPts*dt
tCurrent = getTime()
ok = 0
time = [tCurrent]

u3 = [0.0]

# Perform the transient analysis
while ok == 0 and tCurrent < tFinal:
    ok = analyze(1, .01)

    # if the analysis fails try initial tangent iteration
    if ok != 0:
        print("regular newton failed .. lets try an initail stiffness for this step")
        test('NormDispIncr', 1.0e-12, 100, 0)
        algorithm('ModifiedNewton', '-initial')
        ok = analyze(1, .01)

        # if analysis fails try initial tangent iteration
        if ok == 0:
            print("that worked .. back to regular newton")
            test('NormDispIncr', 1.0e-12, 10)
            algorithm('Newton')

    tCurrent = getTime()

time.append(tCurrent)
u3.append(nodeDisp(3,1))

# Perform an eigenvalue analysis
eigenValues = eigen(numEigen)

print("eigen values at end of transient:", eigenValues)

results = open('results.out','a+')

if ok == 0:
    results.write('PASSED : RCFrameEarthquake.py

Passed!"
else:
    results.write('FAILED : RCFrameEarthquake.py

Failed!"

results.close()

plt.plot(time, u3)
plt.ylabel('Horizontal Displacement of node 3 (in)')
plt.xlabel('Time (s)')

plt.show()

print("==========================")
Example name spaced nonlinear SDOF

1. The source code is developed by Maxim Millen from University of Porto.
2. The source code is shown below, which can be downloaded here.
3. Also download the constants file here, and the ground motion file.
4. Make sure the numpy, matplotlib and eqsig packages are installed in your Python distribution.
5. Run the source code in your favorite Python program and should see

```python
import eqsig
from eqsig import duhamels
import matplotlib.pyplot as plt
import numpy as np

import openseespy.opensees as op
import opensees_constants as opc
#opensees_constants.py should be close to main file_ or use sys.path... to its directory

def get_inelastic_response(mass, k_spring, f_yield, motion, dt, xi=0.05, r_post=0.0):
    
    Run seismic analysis of a nonlinear SDOF

    :param mass: SDOF mass
```

(continues on next page)
```python
:param k_spring: spring stiffness
:param f_yield: yield strength
:param motion: list, acceleration values
:param dt: float, time step of acceleration values
:param xi: damping ratio
:param r_post: post-yield stiffness
:return:

```

```python
op.wipe()
op.model('basic', '-ndm', 2, '-ndf', 3)  # 2 dimensions, 3 dof per node

# Establish nodes
bot_node = 1
top_node = 2
op.node(bot_node, 0., 0.)
op.node(top_node, 0., 0.)

# Fix bottom node
op.fix(top_node, opc.FREE, opc.FIXED, opc.FIXED)
op.fix(bot_node, opc.FIXED, opc.FIXED, opc.FIXED)
# Set out-of-plane DOFs to be slaved
op.equalDOF(1, 2, *[2, 3])

# nodal mass (weight / g):
op.mass(top_node, mass, 0., 0.)

# Define material
bilinear_mat_tag = 1
mat_type = "Steel01"
mat_props = [f_yield, k_spring, r_post]
op.uniaxialMaterial(mat_type, bilinear_mat_tag, *mat_props)

# Assign zero length element
beam_tag = 1
op.element('zeroLength', beam_tag, bot_node, top_node, "-mat", bilinear_mat_tag, "-dir", 1, "-doRayleigh", 1)

# Define the dynamic analysis
load_tag_dynamic = 1
pattern_tag_dynamic = 1

values = list(-1 * motion)  # should be negative
op.timeSeries('Path', load_tag_dynamic, '-dt', dt, '-values', *values)
op.pattern('UniformExcitation', pattern_tag_dynamic, opc.X, '-accel', load_tag_dynamic)

# set damping based on first eigen mode
angular_freq = op.eigen('-fullGenLapack', 1) ** 0.5
alpha_m = 0.0
beta_k = 2 * xi / angular_freq
beta_k_comm = 0.0
beta_k_init = 0.0
op.rayleigh(alpha_m, beta_k, beta_k_init, beta_k_comm)

# Run the dynamic analysis
```
op.wipeAnalysis()

op.algorithm('Newton')
op.system('SparseGeneral')
op.numberer('RCM')
op.constraints('Transformation')
op.integrator('Newmark', 0.5, 0.25)
op.analysis('Transient')

tol = 1.0e-10
iterations = 10
op.test('EnergyIncr', tol, iterations, 0, 2)
analysis_time = (len(values) - 1) * dt
analysis_dt = 0.001

outputs = {
    "time": [],
    "rel_disp": [],
    "rel_accel": [],
    "rel_vel": [],
    "force": []
}

while op.getTime() < analysis_time:
    curr_time = op.getTime()
    op.analyze(1, analysis_dt)
    outputs["time"].append(curr_time)
    outputs["rel_disp"].append(op.nodeDisp(top_node, 1))
    outputs["rel_accel"].append(op.nodeVel(top_node, 1))
    outputs["rel_vel"].append(op.nodeAccel(top_node, 1))
    op.reactions()
    outputs["force"].append(-op.nodeReaction(bot_node, 1))  # Negative since diff
    op.wipe()
    for item in outputs:
        outputs[item] = np.array(outputs[item])

return outputs

def show_single_comparison():
    """
    Create a plot of an elastic analysis, nonlinear analysis and closed form elastic
    :return:
    """

    record_filename = 'test_motion_dt0p01.txt'
motion_step = 0.01
rec = np.loadtxt(record_filename)
acc_signal = eqsig.AccSignal(rec, motion_step)
period = 1.0
xi = 0.05
mass = 1.0
f_yield = 1.5  # Reduce this to make it nonlinear
r_post = 0.0

(continues on next page)
periods = np.array([period])
resp_u, resp_v, resp_a = duhameels.response_series(motion=rec, dt=motion_step,
periods=periods, xi=xi)

k_spring = 4 * np.pi ** 2 * mass / period ** 2
outputs = get_inelastic_response(mass, k_spring, f_yield, rec, motion_step, xi=xi,
r_post=r_post)
outputs_elastic = get_inelastic_response(mass, k_spring, f_yield * 100, rec,
motion_step, xi=xi, r_post=r_post)
ux_opensees = outputs["rel_disp"]
ux_opensees_elastic = outputs_elastic["rel_disp"]

bf, sps = plt.subplots(nrows=2)
sps[0].plot(acc_signal.time, resp_u[0], label="Eqsig")
sps[0].plot(outputs["time"], ux_opensees, label="Opensees fy=%.3g N " % f_yield, ls="--")
sps[0].plot(outputs["time"], ux_opensees_elastic, label="Opensees fy=%.3g N " % (f_yield * 100), ls="--")
sps[1].plot(acc_signal.time, resp_a[0], label="Eqsig")  # Elastic solution
time = acc_signal.time
acc_opensees_elastic = np.interp(time, outputs_elastic["time"], outputs_elastic["rel_accel"])) - rec
print("diff", sum(acc_opensees_elastic - resp_a[0]))
sps[1].plot(time, acc_opensees_elastic, label="Opensees fy=%.2g N " % (f_yield * 100), ls="--")
sps[0].legend()
sps[1].legend()
plt.show()

if __name__ == '__main__':
    show_single_comparison()
This code develops the RotD50 Sa and RotD100 Sa Spectra of the Bi-Directional
Ground Motion records as '.AT2' files provided in the current directory

The two directions of the ground motion record must be named as 'GM1i' and 'GM2i',
where 'i' is the ground motion number which goes from 1 to 'n', 'n' being the total
number of ground motions for which the Spectra needs to be generated. The extension
of the files must be '.AT2'

For example: If the Spectra of two ground motion records are required, 4 files with
the following names must be provided in the given 'GM' folder:
  'GM11.AT2' - Ground Motion 1 in direction 1 (direction 1 can be either one of the
  bi-directional GM as we are rotating the ground motions it does not matter)
  'GM21.AT2' - Ground Motion 1 in direction 2 (direction 2 is the other direction
  of the bi-directional GM)
  'GM12.AT2' - Ground Motion 2 in direction 1 (direction 1 can be either one of the
  bi-directional GM as we are rotating the ground motions it does not matter)
  'GM22.AT2' - Ground Motion 2 in direction 2 (direction 2 is the other direction
  of the bi-directional GM)

The Ground Motion file must be a vector file with 4 header lines. The first 3 lines
  can have
any content, however, the 4th header line must be written exactly as per the,
  following example:
  'NPTS= 15864, DT= 0.0050'
The 'ReadGMFile.py' can be edited accordingly for any other format

You may run this code in python IDE: 'Spyder' or any other similar IDE

Make sure you have the following python libraries installed:
  os
  sys
  pathlib
  fnmatch
  shutil
  IPython
  pandas
  numpy
  matplotlib.pyplot

INPUT:
This codes provides the option to have 3 different regions of developing the Spectra
  of ground motions with different period intervals (discretizations)
The following inputs within the code are required:
  'Path_to_openpyfiles' --> Path where the library files 'opensees.pyd' and 'LICENSE.rst'
  of OpenSeesPy are included (for further details go to https://openseespydoc.readthedocs.io/en/latest/windows.html)
  'Int_T_Reg_1' --> Period Interval for the first region of the Spectrum
  'End_T_Reg_1' --> Last Period of the first region of the Spectrum (where
  to end the first region)
  'Int_T_Reg_2' --> Period Interval for the second region of the Spectrum
  'End_T_Reg_2' --> Last Period of the second region of the Spectrum (where
  to end the second region)
  'Int_T_Reg_3' --> Period Interval for the third region of the Spectrum

(continues on next page)
'End_T_Reg_3' --> Last Period of the third region of the Spectrum (where→ to end the third region)
'Plot_Spectra' --> whether to plot the generated Spectra of the ground motions (options: 'Yes', 'No')

OUTPUT:
The output will be provided in a separate 'GMi_Spectra.txt' file for each ground motion record, where 'i' denotes the number of ground motion in the same of provided 'GM1i.AT2' and 'GM2i.AT2' files. The output files will be generated in a separate folder 'Spectra' which will be created in the current folder. The 'GMi_Spectra.txt' file will consist of space-separated file with:
'Periods (secs)' 'RotD50 Sa (g)' 'RotD100 Sa (g)'

### =============== CODE BEGINS =============== #######
## Importing Libraries
import os, sys, pathlib, fnmatch
import shutil as st
from IPython import get_ipython
from openseespy.opensees import *
import pandas as pd
import numpy as np
import matplotlib.pyplot as plt

# For periods 0 to 'End_T_Reg_1' in an interval of 'Int_T_Reg_1'
Int_T_Reg_1 = 0.1
End_T_Reg_1 = 1

# For periods ['End_T_Reg_1'+'Int_T_Reg_2'] to 'End_T_Reg_2' in an interval of 'Int_T_Reg_2'
Int_T_Reg_2 = 0.2
End_T_Reg_2 = 2

# For periods ['End_T_Reg_2'+'Int_T_Reg_3'] to 'End_T_Reg_3' in an interval of 'Int_T_Reg_3'
Int_T_Reg_3 = 0.5
End_T_Reg_3 = 5

# Plot Spectra (options: 'Yes' or 'No')
Plot_Spectra = 'Yes'

### ================ INPUTS ================ ######
import warnings
import matplotlib.cbook

warnings.filterwarnings("ignore", category=matplotlib.cbook.mplDeprecation)

wipe()

# Getting Number of Ground Motions from the GM folder
GMdir = os.getcwd()
No_of_GMs = int(len(fnmatch.filter(os.listdir(GMdir),'*.AT2'))/2)
print('
Generating Spectra for {} provided GMs

'.format(np.round(No_of_GMs,0)))

# Initializations
DISPLACEMENTS = pd.DataFrame(columns=['uX','uY'])
GM_SPECTRA = pd.DataFrame(columns=['Period(s)','RotD50Sa(g)', 'RotD100Sa(g)'])
SDOF_RESPONSE = [[]]
GM_RESPONSE = [[]]

# Spectra Generation
for iEQ in range(1,No_of_GMs+1):
    print('Generating Spectra for GM: {} ...
'.format(np.round(iEQ,0)))
    Periods = np.concatenate((list(np.arange(Int_T_Reg_1,End_T_Reg_1+Int_T_Reg_1,Int_T_Reg_1)),
                              list(np.arange(End_T_Reg_1+Int_T_Reg_2,End_T_Reg_2+Int_T_Reg_2,Int_T_Reg_2)),
                              list(np.arange(End_T_Reg_2+Int_T_Reg_3,End_T_Reg_3+Int_T_Reg_3,Int_T_Reg_3))),
                             axis=0)
    ii = 0
    for T in Periods:
        ii = ii+1
        GMinter = 0
        # Storing Periods
        GM_SPECTRA.loc[ii-1,'Period(s)'] = T
        # Setting modelbuilder
        model('basic', '-ndm', 3, '-ndf', 6)
        # Setting SODF Variables
        g = 386.1 # value of g
        L = 1.0 # Length
        d = 2 # Diameter
        r = d/2 # Radius
        A = np.pi*(r**2) # Area
        E = 1.0 # Elastic Modulus
        G = 1.0 # Shear Modulus
        I3 = np.pi*(r**4)/4 # Moment of Inertia (zz)
        J = np.pi*(r**4)/2 # Polar Moment of Inertia
        I2 = np.pi*(r**4)/4 # Moment of Inertia (yy)
        K = 3*E*I3/(L**3) # Stiffness
        M = K*(T**2)/(np.pi**2) # Mass
        omega = np.sqrt(K/M) # Natural Frequency
        Tn = 2*np.pi/omega # Natural Period

        # Creating nodes
        node(1, 0.0, 0.0, 0.0)
        node(2, 0.0, 0.0, L)

        # Transformation
        transfTag = 1

(continues on next page)
geomTransf('Linear', transfTag, 0.0, 1.0, 0.0)

# Setting boundary condition
fix(1, 1, 1, 1, 1, 1, 1)

# Defining materials
uniaxialMaterial("Elastic", 11, E)

# Defining elements
element("elasticBeamColumn", 12, 1, 2, A, E, G, J, I2, I3, 1)

# Defining mass
mass(2, M, M, 0.0, 0.0, 0.0, 0.0, 0.0)

# Eigen Value Analysis (Verifying Period)
numEigen = 1
eigenValues = eigen(numEigen)
omega = np.sqrt(eigenValues)
T = 2*np.pi/omega
print('Calculating Spectral Ordinate for Period = {} secs'.format(np.round(T, 3)))

## Reading GM Files
exec(open("ReadGMFile.py").read())

Multinition
iGMinput = 'GM1'+str(iEQ)+' GM2'+str(iEQ)
GMinput = iGMinput.split(' ');
for i in range(0, 2):
inFile = GMdir + '\\'+ GMinput[i]+'.AT2';
dt, NumPts , gmXY = ReadGMFile()

# Storing GM Histories
gmX = gmXY[1]
gmY = gmXY[2]
gmXY_mat = np.column_stack((gmX, gmX, gmY, gmY))

# Bidirectional Uniform Earthquake ground motion (uniform acceleration input at all support nodes)
iGMfile = 'GM1'+str(iEQ)+' GM2'+str(iEQ)
GMfile = iGMfile.split(' ')
GMdirection = [1, 1, 2, 2];
GMfact = [np.cos(GMinter*np.pi/180), np.sin(-GMinter*np.pi/180),
np.sin(GMinter*np.pi/180), np.cos(GMinter*np.pi/180)];
IDTag = 2
loop = [1, 2, 3, 4]

for i in loop:
    # Setting time series to be passed to uniform excitation
timeSeries('Path', IDTag + i, '-dt', dt, '-values', *list(gmXY_mat[:, i-1]),
'-factor', GMfact[i-1]*g)
    # Creating UniformExcitation load pattern
    pattern('UniformExcitation', IDTag+i, GMdirection[i-1], '- accel',
    -IDTag+i)

# Defining Damping
# Applying Rayleigh Damping from $x_\text{Damp}$
# D=$alphaM*M + $betaKcurr*Kcurrent + $betaKcomm*KlastCommit + $beatKinit* 

→ $Kinitial 

→ xDamp = 0.05;  # 5% damping ratio 
→ alphaM = 0.;  # M-prop. damping; 
→ D = alphaM*M 
→ betaKcurr = 0.;  # K-proportional damping; 
→ +beakKcurr*Kcurrent 
→ betaKcomm = 2.*xDamp/omega;  # K-
   prop. damping parameter;  +betaKcomm*KlastCommit 
→ betaKinit = 0.;  # initial-stiffness proportional, 
→ damping  +betaKinit*Kini 
→ rayleigh(alphaM,betaKcurr,betaKinit,betaKcomm);  # RAYLEIGH damping 

# Creating the analysis

wipeAnalysis();  # clear previously-defined

→ analysis parameters 
→ constraints("Penalty",1e18, 1e18)  # how to handle boundary conditions 
→ numberer("RCM")  # renumber dof’s to minimize band-width 
→ (optimization), if you want to 
→ system('SparseGeneral')  # how to store and solve the system of 
→ equations in the analysis 
→ algorithm('Linear')  # use Linear algorithm for linear 
→ analysis 
→ integrator("TRBDF2")  # determine the next time step for an 
→ analysis 
→ algorithm("NewtonLineSearch")  # define type of analysis: time-dependent 
→ test('EnergyIncr',1.0e-6, 100, 0) 
→ analysis("Transient") 

# Variables (Can alter the speed of analysis)

dtAnalysis = dt 
TmaxAnanlysis = dt*NumPts 
tFinal = int(TmaxAnanlysis/dtAnalysis) 
tCurrent = getTime() 
ok = 0 
time = [tCurrent] 

# Initializations of response 

u1 = [0.0] 
u2 = [0.0] 

# Performing the transient analysis (Performance is slow in this loop, can be_
→ altered by changing the parameters) 

→ while ok == 0 and tCurrent < tFinal: 
→  ok = analyze(1, dtAnalysis) 
→  # if the analysis fails try initial tangent iteration 
→  if ok ! = 0: 
→     print("Iteration failed .. lets try an initial stiffness for this step 
→ ") 
→         test('NormDispIncr', 1.0e-12, 100, 0) 
→         algorithm('ModifiedNewton', '-initial') 
→     ok =analyze( 1, .001) 

(continues on next page)
```python
if ok == 0:
    print("that worked .. back to regular newton")
    test('NormDispIncr', 1.0e-12, 10 )
    algorithm('Newton')

    tCurrent = getTime()
    time.append(tCurrent)
    u1.append(nodeDisp(2,1))
    u2.append(nodeDisp(2,2))

    # Storing responses
    DISPLACEMENTS.loc[ii-1,'uX'] = np.array(u1)
    DISPLACEMENTS.loc[ii-1,'uY'] = np.array(u2)
    DISP_X_Y = np.column_stack((np.array(u1),np.array(u2)))

    # Rotating the Spectra (Projections)
    Rot_Matrix = np.zeros((2,2))
    Rot_Disp = np.zeros((180,1))
    for theta in range (0,180,1):
        Rot_Matrix [0,0] = np.cos(np.deg2rad(theta))
        Rot_Matrix [0,1] = np.sin(np.deg2rad(-theta))
        Rot_Matrix [1,0] = np.sin(np.deg2rad(theta))
        Rot_Matrix [1,1] = np.cos(np.deg2rad(theta))
        Rot_Disp[theta,0] = np.max(np.matmul(DISP_X_Y,Rot_Matrix)[:,0])

    # Storing Spectra
    Rot_Acc = np.dot(Rot_Disp,(omega**2)/g)
    GM_SPECTRA.loc[ii-1,'RotD50Sa(g)'] = np.median(Rot_Acc)
    GM_SPECTRA.loc[ii-1,'RotD100Sa(g)']= np.max(Rot_Acc)
    wipe()

    # Writing Spectra to Files
    if not os.path.exists('Spectra'):
        os.makedirs('Spectra')
    GM_SPECTRA.to_csv('Spectra//GM'+str(iEQ)+'_Spectra.txt', sep=' ',header=True, index=False)

    # Plotting Spectra
    if Plot_Spectra == 'Yes':
        def plot_spectra(PlotTitle,SpectraType,iGM):
            axes = fig.add_subplot(1, 1, 1)
            axes.plot(GM_SPECTRA['Period(s)'] , GM_SPECTRA[SpectraType] , '.-',lw=7, markersize=20, label='GM'+str(iGM))
            axes.set_xlabel('Period (sec)',fontsize=30,fontweight='bold')
            axes.set_ylabel(SpectraType,fontsize=30,fontweight='bold')
            axes.set_title(PlotTitle,fontsize=40,fontweight='bold')
            axes.tick_params(labelsize= 25)
            axes.grid(True)
            axes.set_xlim(0, np.ceil(max(GM_SPECTRA['Period(s)'])))
            axes.set_ylim(0, np.ceil(max(GM_SPECTRA[SpectraType])))
            axes.axhline(linewidth=10,color='black')
            axes.axvline(linewidth=10,color='black')
            axes.legend(fontsize =30)
            fig = plt.figure(1,figsize=(18,12))
```

(continues on next page)
```python
plot_spectra('RotD50 Spectra','RotD50Sa(g)',iEQ)
fig = plt.figure(2,figsize=(18,12))
plot_spectra('RotD100 Spectra','RotD100Sa(g)',iEQ)

SDOF_RESPONSE.insert(iEQ-1,DISPLACEMENTS)
GM_RESPONSE.insert(iEQ-1,GM_SPECTRA)

print('Generated Spectra for GM: \n').format(np.round(iEQ,0))
```

**Portal 2D Frame - Dynamic EQ Ground Motion**

1. This is a simple model of an elastic portal frame with EQ ground motion and gravity loading. Here the structure is excited using uniform excitation load pattern
2. All units are in kip, inch, second
3. To run EQ ground-motion analysis, BM68elc.acc needs to be downloaded into the same directory
4. The source code is shown below, which can be downloaded here.
5. The detailed problem description can be found here (example: 1b)

```bash
#!/usr/bin/env python
import matplotlib.pyplot as plt
import numpy as np

from openseespy.opensees import *

G = 2.08  # Design shear modulus
f_y = 60.0  # Design yield stress

# Use SetAnalysisTime to set the time steps
SetAnalysisTime(AnalysisTime.WAVEFORM_FILE, 'BM68elc.acc')

# Define the SDOF system response
SDOF_RESPONSE = createResponse('SPECTRA', 'linear', 1.0)

# Define the GM response
GM_RESPONSE = createResponse('SPECTRA', 'linear', 1.0)

# Create the response for the GM
iEQ = 1
SDOF_RESPONSE.insert(iEQ-1,DISPLACEMENTS)
GM_RESPONSE.insert(iEQ-1,GM_SPECTRA)

print('Generated Spectra for GM: ')
```

---

**Portal 2D Frame - Dynamic EQ Ground Motion**

1. This is a simple model of an elastic portal frame with EQ ground motion and gravity loading. Here the structure is excited using uniform excitation load pattern
2. All units are in kip, inch, second
3. To run EQ ground-motion analysis, BM68elc.acc needs to be downloaded into the same directory
4. The source code is shown below, which can be downloaded here.
5. The detailed problem description can be found here (example: 1b)
# (1) (2) LCol
#  |  |  |
#  |  |  |
#  |  |  |
# =1= =2= _- -------->X
# |----------LBeam------------|
#

# SET UP -----------------------------------------------
import openseespy.opensees as op
#import the os module
import os
op.wipe()

######################################################################################
→

# to create a directory at specified path with name "Data"
os.chdir('C:\Opensees Python\OpenseesPy examples')

#this will create the directory with name 'Data' and will update it when we rerun the
→analysis, otherwise we have to keep deleting the old 'Data' Folder
if not os.path.exists(dir):
    os.makedirs(dir)

#this will create just 'Data' folder
#os.mkdir("Data-1b")

#detect the current working directory
#path1 = os.getcwd()
#pd.print(path1)

h = 432.0
w = 504.0

op.node(1, 0.0, 0.0)
op.node(2, h, 0.0)
op.node(3, 0.0, w)
op.node(4, h, w)

op.fix(1, 1,1,1)
op.fix(2, 1,1,1)
op.fix(3, 0,0,0)
op.fix(4, 0,0,0)

op.mass(3, 5.18, 0.0, 0.0)
op.mass(4, 5.18, 0.0, 0.0)

op.geomTransf('Linear', 1)
A = 3600000000.0
E = 4227.0

(continues on next page)
Iz = 1080000.0
A1 = 5760000000.0
Iz1 = 4423680.0
op.element('elasticBeamColumn', 1, 1, 3, A, E, Iz, 1)
op.element('elasticBeamColumn', 2, 2, 4, A, E, Iz, 1)
op.element('elasticBeamColumn', 3, 3, 4, A1, E, Iz1, 1)
op.element('elasticBeamColumn', 2, 2, 4, A, E, Iz, 1)
op.recorder('Node', '-file', 'Data-1b/DFree.out', '-time', '-node', 3, 4, '-dof', 1, 2, 3, 'disp')
op.recorder('Node', '-file', 'Data-1b/DBase.out', '-time', '-node', 1, 2, '-dof', 1, 2, 3, 'disp')
op.recorder('Node', '-file', 'Data-1b/RBase.out', '-time', '-node', 1, 2, '-dof', 1, 2, 3, 'reaction')
op.recorder('Element', '-file', 'Data-1b/DFCol.out', '-time', '-ele', 1, 2, 'globalForce')
op.recorder('Element', '-file', 'Data-1b/DCol.out', '-time', '-ele', 3, 'deformations')
# defining gravity loads
op.timeSeries('Linear', 1)
op.pattern('Plain', 1, 1)
op.eleLoad('-ele', 3, '-type', '-beamUniform', -7.94)
op.constraints('Plain')
op.numberer('Plain')
op.system('BandGeneral')
op.test('NormDispIncr', 1e-8, 6)
op.algorithm('Newton')
op.integrator('LoadControl', 0.1)
op.analysis('Static')
op.analyze(10)
op.loadConst('-time', 0.0)
# applying Dynamic Ground motion analysis
op.timeSeries('Path', 2, '-dt', 0.01, '-filePath', 'BM68elc.acc', '-factor', 1.0)
op.pattern('UniformExcitation', 2, 1, '-accel', 2) # how to give accelseriesTag?
eigen = op.eigen('-fullGenLapack', 1)
import math
power = math.pow(eigen, 0.5)
betaKcomm = 2 * (0.02/power)
op.rayleigh(0.0, 0.0, 0.0, betaKcomm)
op.wipeAnalysis()
op.constraints('Plain')
op.numberer('Plain')
op.system('BandGeneral')
op.test('NormDispIncr', 1e-8, 10)
op.algorithm('Newton')
op.integrator('Newmark', 0.5, 0.25)
op.analysis('Transient')
op.analyze(1000, 0.02)
2D Column - Dynamic EQ Ground Motion

Converted to openseespy by: Pavan Chigullapally
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1. EQ ground motion with gravity- uniform excitation of structure
2. All units are in kip, inch, second
3. Note: In this example, all input values for Example 1a are replaced by variables. The objective of this example is to demonstrate the use of variables in defining
4. The OpenSees input and also to run various tests and algorithms at once to increase the chances of convergence
5. To run EQ ground-motion analysis (BM68elc.acc needs to be downloaded into the same directory)
6. The detailed problem description can be found here (example:2a)
7. The source code is shown below, which can be downloaded here.

```python
# -*- coding: utf-8 -*-
""
Created on Mon Apr 22 15:12:06 2019

@author: pchi893
""
# Converted to openseespy by: Pavan Chigullapally
# University of Auckland
# Email: pchi893@aucklanduni.ac.nz

# EQ ground motion with gravity- uniform excitation of structure
# all units are in kip, inch, second
# Note: In this example, all input values for Example 1a are replaced by variables.
The objective of this example is to demonstrate the use of variables in defining
# the OpenSees input and also to run various tests and algorithms at once to increase
# the chances of convergence
# Example 2a. 2D cantilever column, dynamic eq ground motion
# To run EQ ground-motion analysis (BM68elc.acc needs to be downloaded into the same directory)
# The detailed problem description can be found here: http://opensees.berkeley.edu/wiki/index.php/Examples_Manual (example:2a)
# OpenSees (Tcl) code by: Silvia Mazzoni & Frank McKenna, 2006

# ^Y
# |
# 2 __
# | |
```

(continues on next page)
import openseespy.opensees as op
import os
import math
op.wipe()

op.model('basic', '-ndm', 2, '-ndf', 3)

os.chdir('C:\Opensees Python\OpenseesPy examples')

if not os.path.exists('Data-2a'):
    os.makedirs('Data-2a')

LCol = 432.0  # column length
Weight = 2000.0  # superstructure weight

HCol = 60.0  # Column Depth
BCol = 60.0  # Column Width

PCol = Weight  # nodal dead-load weight per column
g = 386.4
Mass = PCol/g

ACol = HCol*BCol*1000  # cross-sectional area, make stiff
IzCol = (BCol*math.pow(HCol,3))/12  # Column moment of inertia

op.node(1, 0.0, 0.0)
op.node(2, 0.0, LCol)
op.fix(1, 1, 1, 1)
op.mass(2, Mass, 1e-9, 0.0)
ColTransfTag = 1
op.geomTransf('Linear', ColTransfTag)
#A = 3600000000.0
#E = 4227.0
#Iz = 1080000.0
fc = -4.0 # CONCRETE Compressive Strength (+Tension, -Compression)
Ec = 57*math.sqrt(-fc*1000) # Concrete Elastic Modulus (the term in sqr root needs to be in psi
op.element('elasticBeamColumn', 1, 1, 2, ACol, Ec, IzCol, ColTransfTag)

op.recorder('Node', '-file', 'Data-2a/DFree.out', '-time', '-node', 2, '-dof', 1,2,3, 'disp')

op.recorder('Node', '-file', 'Data-2a/DBase.out', '-time', '-node', 1, '-dof', 1,2,3, 'disp')

op.recorder('Node', '-file', 'Data-2a/RBase.out', '-time', '-node', 1, '-dof', 1,2,3, 'reaction')

#op.recorder('Drift', '-file', 'Data-2a/Drift.out', '-time', '-node', 1, '-dof', 1,2,3, 'disp')

op.recorder('Element', '-file', 'Data-2a/FCol.out', '-time', '-ele', 1, 'globalForce')

op.recorder('Element', '-file', 'Data-2a/DCol.out', '-time', '-ele', 1, 'deformations')

# defining gravity loads
op.timeSeries('Linear', 1)

op.pattern('Plain', 1, 1)

op.load(2, 0.0, -PCol, 0.0)

tol = 1e-8 # convergence tolerance for test

NstepGravity = 10

DGravity = 1/NstepGravity

op.numberer('Plain') # renumber dof's to minimize band-width (optimization), if you want to

op.system('BandGeneral') # how to store and solve the system of equations in the analysis

op.constraints('Plain') # how it handles boundary conditions

op.test('NormDispIncr', Tol, 6) # determine if convergence has been achieved at the end of an iteration step

op.algorithm('Newton') # use Newton's solution algorithm: updates tangent stiffness at every iteration

op.analysis('Static') # define type of analysis static or transient

op.analyze(NstepGravity) # apply gravity

op.loadConst('-time', 0.0) # maintain constant gravity loads and reset time to zero

# applying Dynamic Ground motion analysis

GMdirection = 1

GMfile = 'BM68elc.acc'

GMfact = 1.0

Lambda = op.eigen('-fullGenLapack', 1) # eigenvalue mode 1

import math
Omega = math.pow(Lambda, 0.5)
betaKcomm = 2 * (0.02/Omega)

xDamp = 0.02  # 2% damping ratio
alphaM = 0.0  # M-prop. damping; D = alphaM*M
betaKcurr = 0.0  # K-proportional damping; +beatKcurr*KCurrent
betaKinit = 0.0  # initial-stiffness proportional damping +beatKinit*Kini

op.rayleigh(alphaM, betaKcurr, betaKinit, betaKcomm)  # RAYLEIGH damping

# Uniform EXCITATION: acceleration input
IDloadTag = 400  # load tag
dt = 0.01  # time step for input ground motion
GMfatt = 1.0  # data in input file is in g Unifts -- ACCELERATION TH
maxNumIter = 10
op.timeSeries('Path', 2, '-dt', dt, '-filePath', GMfile, '-factor', GMfact)
op.pattern('UniformExcitation', IDloadTag, GMdirection, '-accel', 2)
op.wipeAnalysis()
op.constraints('Transformation')
op.numberer('Plain')
op.system('BandGeneral')
op.test('EnergyIncr', Tol, maxNumIter)
op.algorithm('ModifiedNewton')

NewmarkGamma = 0.5
NewmarkBeta = 0.25
op.integrator('Newmark', NewmarkGamma, NewmarkBeta)
op.analysis('Transient')

DtAnalysis = 0.01
TmaxAnalysis = 10.0
Nsteps = int(TmaxAnalysis/ DtAnalysis)
ok = op.analyze(Nsteps, DtAnalysis)
tCurrent = op.getTime()

# for gravity analysis, load control is fine, 0.1 is the load factor increment (http://opensees.berkeley.edu/wiki/index.php/Load_Control)

for i in test:
    for j in algorithm:
        if ok != 0:
            if j < 4:
                op.algorithm(algorithm[j], '-initial')
            else:
                op.algorithm(algorithm[j])
```python
op.test(test[i], Tol, 1000)
ok = op.analyze(Nsteps, DtAnalysis)
print(test[i], algorithm[j], ok)
if ok == 0:
    break
else:
    continue
u2 = op.nodeDisp(2, 1)
print("u2 = ", u2)
op.wipe()
```

Nonlinear Canti Col Uniaxial Inelastic Section- Dyn EQ GM

1. EQ ground motion with gravity- uniform excitation of structure
2. The nonlinear beam-column element that replaces the elastic element of Example 2a requires the definition of the element cross section, or its behavior. In this example,
3. The Uniaxial Section used to define the nonlinear moment-curvature behavior of the element section is “aggregated” to an elastic response for the axial behavior to define
4. The required characteristics of the column element in the 2D model. In a 3D model, torsional behavior would also have to be aggregated to this section.
5. Note: In this example, both the axial behavior (typically elastic) and the flexural behavior (moment curvature) are defined independently and are then “aggregated” into a section.
6. This is a characteristic of the uniaxial section: there is no coupling of behaviors.
7. To run EQ ground-motion analysis (BM68elc.acc needs to be downloaded into the same directory)
8. The problem description can be found here (example:2b)
9. The source code is shown below, which can be downloaded here.

```
# -*- coding: utf-8 -*-

Created on Mon Apr 22 15:12:06 2019

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# Converted to openseespy by: Pavan Chigullapally
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# Example 2b. 2D cantilever column, dynamic eq ground motion
# EQ ground motion with gravity- uniform excitation of structure
# the nonlinear beam-column element that replaces the elastic element of Example 2a requires the definition of the element cross section, or its behavior. In this example, the Uniaxial Section used to define the nonlinear moment-curvature behavior of the element section is "aggregated" to an elastic response for the axial behavior to define
```
# the required characteristics of the column element in the 2D model. In a 3D model, torsional behavior would also have to be aggregated to this section.
# Note: In this example, both the axial behavior (typically elastic) and the flexural behavior (moment curvature) are defined independently and are then "aggregated" into a section.
# This is a characteristic of the uniaxial section: there is no coupling of behaviors.

# To run EQ ground-motion analysis (BM68elc.acc needs to be downloaded into the same directory)
# the problem description can be found here: http://opensees.berkeley.edu/wiki/index.php/Examples_Manual(example:2b)

---

```
# OpenSees (Tcl) code by: Silvia Mazzoni & Frank McKenna, 2006
#
# ^Y
# |
# 2 __
# | |
# | |
# | |
# (1) LCol
#
# =1= _|_ -------->X
#

# SET UP -------------------------------
import openseespy.opensees as op
import os
import math
op.wipe()
#to create a directory at specified path with name "Data"
path1 = os.getcwd()
print(path1)
#detect the current working directory
#this will create just 'Data' folder
os.makedirs(path1 + '\Data-2b')
if not os.path.exists(path1 + '\Data-2b'):
    os.makedirs(path1 + '\Data-2b')
#this will create the directory with name 'Data' and will update it when we rerun the analysis, otherwise we have to keep deleting the old 'Data' Folder
```

LCol = 432.0 # column length
Weight = 2000.0 # superstructure weight

# define section geometry
HCol = 60.0 # Column Depth
BCol = 60.0 # Column Width

PCol = Weight # nodal dead-load weight per column
g = 386.4
Mass = PCol/g

ACol = HCol*BCol*1000 # cross-sectional area, make stiff
IzCol = (BCol*math.pow(HCol,3))/12 # Column moment of inertia

op.node(1, 0.0, 0.0)
op.node(2, 0.0, LCol)
op.fix(1, 1, 1, 1)
op.mass(2, Mass, 1e-9, 0.0)

#Define Elements and Sections
ColMatTagFlex = 2
ColMatTagAxial = 3
ColSecTag = 1
BeamSecTag = 2

fc = -4.0 # CONCRETE Compressive Strength (+Tension, -Compression)
Ec = 57*math.sqrt(-fc*1000) # Concrete Elastic Modulus (the term in sqrt needs to be in psi)

#Column Section
EICol = Ec*IzCol # EI, for moment-curvature relationship
EACol = Ec*ACol # EA, for axial-force-strain relationship
MyCol = 130000.0 #yield Moment calculated

PhiYCol = 0.65e-4 # yield curvature
EIColCrack = MyCol/PhiYCol # cracked section inertia

b = 0.01 # strain-hardening ratio (ratio between post-yield tangent and initial elastic tangent)

op.uniaxialMaterial('Steel01', ColMatTagFlex, MyCol, EIColCrack, b) #steel moment-curvature issued for Mz of the section only, # bilinear behavior for flexure
op.uniaxialMaterial('Elastic', ColMatTagAxial, EACol) # this is not used as a material, this is an axial-force-strain response
op.section('Aggregator', ColSecTag, ColMatTagAxial, 'P', ColMatTagFlex, 'Mz') # combine axial and flexural behavior into one section (no P-M interaction here)

ColTransfTag = 1
op.geomTransf('Linear', ColTransfTag)
numIntgrPts = 5
eleTag = 1
op.element('nonlinearBeamColumn', eleTag, 1, 2, numIntgrPts, ColSecTag, ColTransfTag)

op.recorder('Node', '-file', 'Data-2b/DFree.out','-time', '-node', 2, '-dof', 1,2,3,'disp')
op.recorder('Node', '-file', 'Data-2b/DBase.out','-time', '-node', 1, '-dof', 1,2,3,'disp')
op.recorder('Node', '-file', 'Data-2b/RBase.out','-time', '-node', 1, '-dof', 1,2,3,'reaction')
111 # op.recorder('Drift', '-file', 'Data-2b/Drift.out', '-time', '-node', 1, '-dof', 1, 2, 3, → 'disp')
112 op.recorder('Element', '-file', 'Data-2b/FCol.out', '-time', '-ele', 1, 'globalForce')
113 op.recorder('Element', '-file', 'Data-2b/ForceColSec1.out', '-time', '-ele', 1, → 'section', 1, 'force')
114 op.recorder('Element', '-file', 'Data-2b/DCol.out', '-time', '-ele', 1, 'deformations → ')

# defining gravity loads
116 op.timeSeries('Linear', 1)
117 op.pattern('Plain', 1, 1)
118 op.load(2, 0.0, -PCol, 0.0)

119 # defining gravity loads
120 Tol = 1e-8 # convergence tolerance for test
121 NstepGravity = 10
122 DGravity = 1/NstepGravity
123 op.integrator('LoadControl', DGravity) # determine the next time step for an analysis
124 op.numberer('Plain') # renumber dof’s to minimize band-width (optimization), if you → want to
125 op.system('BandGeneral') # how to store and solve the system of equations in the → analysis
126 op.constraints('Plain') # how it handles boundary conditions
127 op.test('NormDispIncr', Tol, 6) # determine if convergence has been achieved at the → end of an iteration step
128 op.algorithm('Newton') # use Newton’s solution algorithm: updates tangent stiffness → at every iteration
129 op.analyze('Static') # define type of analysis static or transient
130 op.analyze(NstepGravity) # apply gravity
131 op.loadConst('-time', 0.0) # maintain constant gravity loads and reset time to zero

132 # applying Dynamic Ground motion analysis
133 GMdirection = 1
134 GMfile = 'BM68elc.acc'
135 GMfact = 1.0
136
137 Lambda = op.eigen('-fullGenLapack', 1) # eigenvalue mode 1
138 import math
139 Omega = math.pow(Lambda, 0.5)
140 betaKcomm = 2 * (0.02/Omega)
141
142 xDamp = 0.02 # 2% damping ratio
143 alphaM = 0.0 # M-prop. damping; D = alphaM*M
144 betaKcurr = 0.0 # K-proportional damping; +beatKcurr*KCurrent
145 betaKinit = 0.0 # initial-stiffness proportional damping +beatKinit*Kini
146
147 op.rayleigh(alphaM, betaKcurr, betaKinit, betaKcomm) # RAYLEIGH damping
148
149 # Uniform EXCITATION: acceleration input
150 IDLoadTag = 400 # load tag
151 dt = 0.01 # time step for input ground motion
152 GMfatt = 1.0 # data in input file is in g Units -- → ACCELERATION TH
153 maxNumIter = 10
154 op.timeSeries('Path', 2, '-dt', dt, '-filePath', GMfile, '-factor', GMfatt)
op.pattern('UniformExcitation', IDloadTag, GMdirection, '-accel', 2)
op.wipeAnalysis()
op.constraints('Transformation')
op.numberer('Plain')
op.system('BandGeneral')
op.test('EnergyIncr', Tol, maxNumIter)
op.algorithm('ModifiedNewton')

NewmarkGamma = 0.5
NewmarkBeta = 0.25
op.integrator('Newmark', NewmarkGamma, NewmarkBeta)
op.analysis('Transient')

DtAnalysis = 0.01
TmaxAnalysis = 10.0
Nsteps = int(TmaxAnalysis/ DtAnalysis)
ok = op.analyze(Nsteps, DtAnalysis)
tCurrent = op.getTime()

# for gravity analysis, load control is fine, 0.1 is the load factor increment (http://opensees.berkeley.edu/wiki/index.php/Load_Control)

for i in test:
    for j in algorithm:
        if ok != 0:
            if j < 4:
                op.algorithm(algorithm[j], '-initial')
            else:
                op.algorithm(algorithm[j])
            op.test(test[i], Tol, 1000)
            ok = op.analyze(Nsteps, DtAnalysis)
            print(test[i], algorithm[j], ok)
            if ok == 0:
                break
        else:
            continue

u2 = op.nodeDisp(2, 1)
print("u2 = ", u2)
op.wipe()
1. EQ ground motion with gravity- uniform excitation of structure
2. In this example, the Uniaxial Section of Example 2b is replaced by a fiber section. Inelastic uniaxial materials are used in this example,
3. Which are assigned to each fiber, or patch of fibers, in the section.
4. In this example the axial and flexural behavior are coupled, a characteristic of the fiber section.
5. The nonlinear/inelastic behavior of a fiber section is defined by the stress-strain response of the uniaxial materials used to define it.
6. To run EQ ground-motion analysis (BM68elc.acc needs to be downloaded into the same directory)
7. The problem description can be found here (example:2c)
8. The source code is shown below, which can be downloaded here.

```python
# -*- coding: utf-8 -*-

Created on Mon Apr 22 15:12:06 2019

@author: pchi893

# Converted to openseespy by: Pavan Chigullapally
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# Example 2c. 2D cantilever column, dynamic eq ground motion
# EQ ground motion with gravity- uniform excitation of structure
# In this example, the Uniaxial Section of Example 2b is replaced by a fiber section.
# Inelastic uniaxial materials are used in this example,
# which are assigned to each fiber, or patch of fibers, in the section.
# In this example the axial and flexural behavior are coupled, a characteristic of the fiber section.
# The nonlinear/inelastic behavior of a fiber section is defined by the stress-strain response of the uniaxial materials used to define it.
# To run EQ ground-motion analysis (BM68elc.acc needs to be downloaded into the same directory)
# the problem description can be found here: http://opensees.berkeley.edu/wiki/index.php/Examples_Manual(example: 2c)
# ---------------------------------------------------------------
# OpenSees (Tcl) code by: Silvia Mazzoni & Frank McKenna, 2006
```

(continues on next page)
import openseespy.opensees as op
import os
import math
op.wipe()
# to create a directory at specified path with name "Data"
o.s.chdir('C:Opensees Python\OpenseesPy examples')
# this will create the directory with name 'Data' and will update it when we rerun the analysis, otherwise we have to keep deleting the old 'Data' Folder
if not os.path.exists('C:Opensees Python\OpenseesPy examples\Data-2c':
    os.makedirs('C:Opensees Python\OpenseesPy examples\Data-2c')
# define section geometry
HCol = 60.0  # Column Depth
BCol = 60.0  # Column Width
PCol = Weight  # nodal dead-load weight per column
g = 386.4
Mass = PCol/g
ACol = HCol*BCol*1000  # cross-sectional area, make stiff
IzCol = (BCol*math.pow(HCol,3))/12  # Column moment of inertia
op.node(1, 0.0, 0.0)
op.node(2, 0.0, LCol)
op.fix(1, 1, 1, 1)
op.mass(2, Mass, 1e-9, 0.0)
ColSecTag = 1  # assign a tag number to the column section
coverCol = 5.0  # Column cover to reinforcing steel NA.
numBarsCol = 16  # number of longitudinal-reinforcement bars in column. (symmetric -top & bot)
barAreaCol = 2.25  # area of longitudinal-reinforcement bars

# MATERIAL parameters
IDconcU = 1    # material ID tag -- unconfined cover concrete
   (here used for complete section)
IDreinf = 2   # material ID tag -- reinforcement

# nominal concrete compressive strength
fc = -4.0     # CONCRETE Compressive Strength (+Tension, -Compression)
Ec = 57*math.sqrt(-fc*1000)  # Concrete Elastic Modulus (the term in sqrt root needs to be in psi)

# unconfined concrete
fclU = fc # UNCONFINED concrete (toeschini parabolic model),
     # maximum stress
eps1U = -0.003   # strain at maximum strength of unconfined concrete
fcsU = 0.2*fclU # ultimate stress
eps2U = -0.01   # strain at ultimate stress
Lambda = 0.1 # ratio between unloading slope at $\varepsilon_2$ and initial slope $\varepsilon_1$

# tensile-strength properties
ftU = -0.14*fclU # tensile strength +tension
Ets = ftU/0.002 # tension softening stiffness

Fy = 66.8 # STEEL yield stress
Es = 29000.0 # modulus of steel
Bs = 0.01  # strain-hardening ratio
R0 = 18.0 # control the transition from elastic to plastic branches

# control the transition from elastic to plastic branches
CR1 = 0.925
CR2 = 0.15

op.uniaxialMaterial('Concrete02', IDconcU, fclU, eps1U, fcsU, eps2U, Lambda, ftU, Ets) # build cover concrete (unconfined)

op.uniaxialMaterial('Steel02', IDreinf, Fy, Es, Bs, R0, CR1, CR2) # build reinforcement material

# FIBER SECTION properties -----------------------------------------------------------
# symmetric section
#   y   ^
# /    |
# ------------------- -- --
# | o o o o | | -- cover
# |          |
# |          |
# |          |
# z --- | +   | H
# |
# |              |
# |              |
# |              |
# | o o o o | | -- cover
# | -------------- -- --
# |          B --------}
# RC section:

coverY = HCol/2.0  # The distance from the section z-axis to the edge of the
      # cover concrete -- outer edge of cover concrete
coverZ = BCol/2.0  # The distance from the section y-axis to the edge of the
      # cover concrete -- outer edge of cover concrete

coreY = coverY-coverCol
coreZ = coverZ-coverCol

nfY = 16  # number of fibers for concrete in y-direction
nfZ = 4   # number of fibers for concrete in z-direction

op.section('Fiber', ColSecTag)
op.patch('quad', IDconcU, nfZ, nfY, -coverY,coverZ, -coverY,-coverZ, coverY,-coverZ,
       coverY,coverZ)  # Define the concrete patch

op.layer('straight', IDreinf, numBarsCol, barAreaCol, -coreY,coreZ,-coreY,-coreZ)
op.layer('straight', IDreinf, numBarsCol, barAreaCol, coreY,coreZ, coreY,-coreZ)

ColTransfTag = 1

op.geomTransf('Linear', ColTransfTag)

numIntgrPts = 5
eleTag = 1

# import InelasticFiberSection

op.element('nonlinearBeamColumn', eleTag, 1, 2, numIntgrPts, ColSecTag, ColTransfTag)

op.recorder('Node', '-file', 'Data-2c/DFree.out','-time', '-node', 2, '-dof', 1,2,3,
       '-disp')
op.recorder('Node', '-file', 'Data-2c/DBase.out','-time', '-node', 1, '-dof', 1,2,3,
       '-disp')
op.recorder('Node', '-file', 'Data-2c/RBase.out','-time', '-node', 1, '-dof', 1,2,3,
       '-reaction')

#op.recorder('Drift', '-file', 'Data-2c/Drift.out','-time', '-node', 1, '-dof', 1,2,3,
       '-disp')

op.recorder('Element', '-file', 'Data-2c/ForceColSec1.out','-time', '-ele', 1, 'section', 1, 'force')

#op.recorder('Element', '-file', 'Data-2c/DCol.out','-time', '-ele', 1, 'deformations')

#defining gravity loads

op.timeSeries('Linear', 1)
op.pattern('Plain', 1, 1)
op.load(2, 0.0, -PCol, 0.0)

tol = 1e-8  # convergence tolerance for test
NstepGravity = 10
DGravity = 1/NstepGravity

op.integrator('LoadControl', DGravity)  # determine the next time step for an analysis
op.numberer('Plain')  # renumber dof’s to minimize band-width (optimization), if you
       # want to
op.system('BandGeneral')  # how to store and solve the system of equations in the
       # analysis

op.constraints('Plain')  # how it handles boundary conditions

op.test('NormDispIncr', tol, 6)  # determine if convergence has been achieved at the
       # end of an iteration step

op.algorithm('Newton')  # use Newton’s solution algorithm: updates tangent stiffness
       # at every iteration

(continues on next page)
op.analysis('Static') # define type of analysis static or transient
op.analyze(NstepGravity) # apply gravity
op.loadConst('-time', 0.0) # maintain constant gravity loads and reset time to zero

# applying Dynamic Ground motion analysis
GMdirection = 1
GMfile = 'BM68elc.acc'
GMfact = 1.0

Lambda = op.eigen('-fullGenLapack', 1) # eigenvalue mode 1
import math
Omega = math.pow(Lambda, 0.5)
betakcomm = 2 * (0.02/Omega)
xDamp = 0.02 # 2% damping ratio
alphaM = 0.0 # M-prop. damping; D = alphaM*M
betakcurr = 0.0 # K-proportional damping; +beatKcurr*KCurrent
betakinit = 0.0 # initial-stiffness proportional damping +betakinit*Kini
op.rayleigh(alphaM,betakcurr, betakinit, betakcomm) # RAYLEIGH damping

# Uniform EXCITATION: acceleration input
IDloadTag = 400 # load tag
dt = 0.01 # time step for input ground motion
GMfatt = 1.0 # data in input file is in g Unfits --
maxNumIter = 10
op.timeSeries('Path', 2, '-dt', dt, '-filePath', GMfile, '-factor', GMfact)
op.pattern('UniformExcitation', IDloadTag, GMdirection, '-accel', 2)
op.wipeAnalysis()
op.constraints('Transformation')
op.numberer('Plain')
op.system('BandGeneral')
op.test('EnergyIncr', Tol, maxNumIter)
op.algorithm('ModifiedNewton')

NewmarkGamma = 0.5
NewmarkBeta = 0.25
op.integrator('Newmark', NewmarkGamma, NewmarkBeta)
op.analysis('Transient')

DtAnalysis = 0.01
TmaxAnalysis = 10.0
Nsteps = int(TmaxAnalysis/ DtAnalysis)
ok = op.analyze(Nsteps, DtAnalysis)
tCurrent = op.getTime()

# for gravity analysis, load control is fine, 0.1 is the load factor increment (http://opensees.berkeley.edu/wiki/index.php/Load_Control)

for i in test:
    for j in algorithm:
        if ok != 0:
            if j < 4:
                op.algorithm(algorithm[j], '-initial')
            else:
                op.algorithm(algorithm[j])

        op.test(test[i], Tol, 1000)
        ok = op.analyze(Nsteps, DtAnalysis)
        print (test[i], algorithm[j], ok)
        if ok == 0:
            break
        else:
            continue

u2 = op.nodeDisp(2, 1)
print("u2 = ", u2)

op.wipe()
Email: pchi893@aucklanduni.ac.nz

Example 3. 2D Cantilever -- EQ ground motion

To run Uniaxial Inelastic Material, Fiber Section, Nonlinear Mode, Uniform Earthquake Excitation: First import the InelasticFiberSection.py (upto gravity loading is already in this script)

and run the current script

To run EQ ground-motion analysis (BM68elc.acc needs to be downloaded into the same directory)

Same acceleration input at all nodes restrained in specified direction (uniform acceleration input at all support nodes)

detailed problem description can be found here: http://opensees.berkeley.edu/wiki/index.php/Examples_Manual (example: 3)

OpenSees (Tcl) code by: Silvia Mazzoni & Frank McKenna, 2006

import opensespy.opensees as op

import the os module

import os

import math

op.wipe()

import InelasticFiberSection

applying Dynamic Ground motion analysis

Tol = 1e-8

GMdirection = 1

GMfile = 'BM68elc.acc'

GMfact = 1.0

Lambda = op.eigen('-fullGenLapack', 1) # eigenvalue mode 1

Omega = math.pow(Lambda, 0.5)

betaKcomm = 2 * (0.02/Omega)

xDamp = 0.02 # 2% damping ratio

alphaM = 0.0 # M-prop. damping; D = alphaM*M

betaKcurr = 0.0 # K-proportional damping; +beatKcurr*KCurrent

betaKinit = 0.0 # initial-stiffness proportional damping +beatKinit*Kini

op.rayleigh(alphaM, betaKcurr, betaKinit, betaKcomm) # RAYLEIGH damping

Uniform EXCITATION: acceleration input

IDloadTag = 400 # load tag

dt = 0.01 # time step for input ground motion

GMfatt = 1.0 # data in input file is in g Unifts -- ACCELERATION TH

maxNumIter = 10

op.timeSeries('Path', 2, '-dt', dt, '-filePath', GMfile, '-factor', GMfact)

op.pattern('UniformExcitation', IDloadTag, GMdirection, '-accel', 2)

op.wipeAnalysis()

op.constraints('Transformation')

op.numberer('Plain')

op.system('BandGeneral')

op.test('EnergyIncr', Tol, maxNumIter)

op.algorithm('ModifiedNewton')

(continues on next page)
NewmarkGamma = 0.5
NewmarkBeta = 0.25
op.integrator('Newmark', NewmarkGamma, NewmarkBeta)
op.analysis('Transient')

DtAnalysis = 0.01 # time-step Dt for lateral analysis
TmaxAnalysis = 10.0 # maximum duration of ground-motion analysis

Nsteps = int(TmaxAnalysis/ DtAnalysis)
ok = op.analyze(Nsteps, DtAnalysis)
tCurrent = op.getTime()

# for gravity analysis, load control is fine, 0.1 is the load factor increment (http://opensees.berkeley.edu/wiki/index.php/Load_Control)

for i in test:
    for j in algorithm:
        if ok != 0:
            if j < 4:
                op.algorithm(algorithm[j], '-initial')
            else:
                op.algorithm(algorithm[j])
        else:
            op.test(test[i], Tol, 1000)
        ok = op.analyze(Nsteps, DtAnalysis)
        print(test[i], algorithm[j], ok)
        if ok == 0:
            break
    else:
        continue

u2 = op.nodeDisp(2, 1)
print("u2 = ", u2)
op.wipe()

Cantilever 2D Column with Units-Static Pushover

Converted to openseespy by: Pavan Chigullapally
University of Auckland
Email: pchi893@aucklanduni.ac.nz

1. To run Uniaxial Inelastic Material, Fiber Section, Nonlinear Mode, Static Pushover Analysis
2. First import the InelasticFiberSection.py (upto gravity loading is already in this script) and run the current script
3. To run EQ ground-motion analysis BM68elc.acc needs to be downloaded into the same directory.

4. Same acceleration input at all nodes restrained in specified direction (uniform acceleration input at all support nodes)

5. The problem description can be found here (example:3)

6. The source code is shown below, which can be downloaded here.

```python
# -*- coding: utf-8 -*-

""
Created on Mon Apr 22 15:12:06 2019

@author: pchi893
""
# Converted to openseespy by: Pavan Chigullapally
# University of Auckland
# Email: pchi893@aucklanduni.ac.nz

# Example 3. 2D Cantilever -- Static Pushover
# To run Uniaxial Inelastic Material, Fiber Section, Nonlinear Mode, Static Pushover...
# Analysis: First import the InelasticFiberSection.py (upto gravity loading is already...
# in this script)
# and run the current script
# the detailed problem description can be found here: http://opensees.berkeley.edu/
# wiki/index.php/Examples_Manual  (example: 3)
""

# OpenSees (Tcl) code by: Silvia Mazzoni & Frank McKenna, 2006
# characteristics of pushover analysis
# ###############################################################################
# ###############################################################################
import openseespy.opensees as op
# import the os module
# import os

from InelasticFiberSection import *

Dmax = 0.05*LCol
Dincr = 0.001*LCol
Hload = Weight
maxNumIter = 6
tol = 1e-8

op.timeSeries('Linear', 2)
op.pattern('Plain', 200, 2)
op.load(2, Hload, 0.0, 0.0)

op.wipeAnalysis()
op.constraints('Plain')
op.numberer('Plain')
op.system('BandGeneral')
op.test('EnergyIncr', Tol, maxNumIter)
op.algorithm('Newton')
op.integrator('DisplacementControl', IDctrlNode, IDctrlDOF, Dincr)
op.analysis('Static')
```

(continues on next page)
Nsteps = int(Dmax/Dincr)
ok = op.analyze(Nsteps)
print(ok)

# for gravity analysis, load control is fine, 0.1 is the load factor increment (http://
→opensees.berkeley.edu/wiki/index.php/Load_Control)
test = {1:'NormDispIncr', 2: 'RelativeEnergyIncr', 4: 'RelativeNormUnbalance',5:
→'RelativeNormDispIncr', 6: 'NormUnbalance'}
algorithm = {1:'KrylovNewton', 2: 'SecantNewton', 4: 'RaphsonNewton',5:
→'PeriodicNewton', 6: 'BFGS', 7: 'Broyden', 8: 'NewtonLineSearch'}

for i in test:
    for j in algorithm:
        if ok != 0:
            if j < 4:
                op.algorithm(algorithm[j], '-initial')
            else:
                op.algorithm(algorithm[j])
            op.test(test[i], Tol, 1000)
            ok = op.analyze(Nsteps)
            print(test[i], algorithm[j], ok)
            if ok == 0:
                break
        else:
            continue

u2 = op.nodeDisp(2, 1)
print("u2 = ", u2)
op.wipe()
# -- coding: utf-8 --
#
Created on Mon Apr 22 15:12:06 2019

@author: pchi893
#
# Converted to openseespy by: Pavan Chigullapally
# University of Auckland
# Email: pchi893@aucklanduni.ac.nz
# Example4. 2D Portal Frame-- Dynamic EQ input analysis

#To run Uniaxial Inelastic Material, Fiber Section, Nonlinear Mode, Uniform
---Earthquake Excitation:First import the InelasticFiberSectionPortal2Dframe.py
#(upto gravity loading is already in this script) and run the current script
#To run EQ ground-motion analysis (ReadRecord.py, H-E12140.AT2 needs to be downloaded
---into the same directory)
#Same acceleration input at all nodes restrained in specified direction (uniform
---acceleration input at all support nodes)
#the problem description can be found here:
# ----------------------------------
import openseespy.opensees as op
#import the os module
#import os
import math
op.wipe()
#---------------------------------------------------------------
from InelasticFiberSectionPortal2Dframe import *
#applying Dynamic Ground motion analysis
Tol = 1e-8
maxNumIter = 10
GMdirection = 1
GMfact = 1.5
GMfatt = g*GMfact
DtAnalysis = 0.01*sec # time-step Dt for lateral analysis
TmaxAnalysis = 10.0*sec # maximum duration of ground-motion analysis

Lambda = op.eigen('-fullGenLapack', 1) # eigenvalue mode 1
Omega = math.pow(Lambda, 0.5)
betakComm = 2 * (0.02/Omega)
xDamp = 0.02 # 2% damping ratio
alphaM = 0.0 # M-prop. damping; D = alphaM*M
betakcurr = 0.0 # K-proportional damping; +beatKcurr*KCurrent
betakinit = 0.0 # initial-stiffness proportional damping +beatKinit*Kini
op.rayleigh(alphaM,betakcurr, betakinit, betakcomm) # RAYLEIGH damping

# Set some parameters

(continues on next page)
```python
record = 'H-E12140'

import ReadRecord

# Perform the conversion from SMD record to OpenSees record
dt, nPts = ReadRecord.ReadRecord(record+'.at2', record+'.dat')
# print(dt, nPts)

# Uniform EXCITATION: acceleration input
IDloadTag = 400  # load tag
op.timeSeries('Path', 2, '-dt', dt, '-filePath', record+'.dat', '-factor', GMfatt)
op.pattern('UniformExcitation', IDloadTag, GMdirection, '-accel', 2)

op.wipeAnalysis()

op.constraints('Transformation')
op.numberer('RCM')
op.system('BandGeneral')
# op.test('EnergyIncr', Tol, maxNumIter)
# op.algorithm('ModifiedNewton')
NewmarkGamma = 0.5
NewmarkBeta = 0.25
op.integrator('Newmark', NewmarkGamma, NewmarkBeta)

op.analysis('Transient')

Nsteps = int(TmaxAnalysis/dtAnalysis)

ok = op.analyze(Nsteps, DtAnalysis)

tCurrent = op.getTime()

tFinal = nPts*dt

# for gravity analysis, load control is fine, 0.1 is the load factor increment (http://opensees.berkeley.edu/wiki/index.php/Load_Control)

tFinal = nPts*dt

# tFinal = 10.0*sec
time = [tCurrent]
u3 = [0.0]
u4 = [0.0]
ok = 0
while tCurrent < tFinal:
    # ok = op.analyze(1, .01)
    for i in test:
        for j in algorithm:
            if j < 4:
                op.algorithm(algorithm[j], '-initial')
            else:
                op.algorithm(algorithm[j])
                while ok == 0 and tCurrent < tFinal:
                    op.test(test[i], Tol, maxNumIter)
```

(continues on next page)
NewmarkGamma = 0.5
NewmarkBeta = 0.25
op.integrator('Newmark', NewmarkGamma, NewmarkBeta)
op.analysis('Transient')
ok = op.analyze(1, .01)

if ok == 0 :
tCurrent = op.getTime()
time.append(tCurrent)
u3.append(op.nodeDisp(3,1))
u4.append(op.nodeDisp(4,1))
print(test[i], algorithm[j], 'tCurrent=', tCurrent)

import matplotlib.pyplot as plt
plt.figure(figsize=(8,8))
plt.plot(time, u3)
plt.ylabel('Horizontal Displacement of node 3 (in)')
plt.xlabel('Time (s)')
plt.savefig('Horizontal Disp at Node 3 vs time.jpeg', dpi = 500)
plt.show()

plt.figure(figsize=(8,8))
plt.plot(time, u4)
plt.ylabel('Horizontal Displacement of node 4 (in)')
plt.xlabel('Time (s)')
plt.savefig('Horizontal Disp at Node 4 vs time.jpeg', dpi = 500)
plt.show()

op.wipe()
# Example4. 2D Portal Frame-- Dynamic EQ input analysis-- multiple-support excitation using acceleration timeseries

To run Uniaxial Inelastic Material, Fiber Section, Nonlinear Mode, MultipleSupportEarthquake ground motion: First import the InelasticFiberSectionPortal2Dframe.py

To run EQ ground-motion analysis (ReadRecord.py, H-E12140.AT2 needs to be downloaded into the same directory)

MultipleSupport Earthquake ground motion (different acceleration input at specified support nodes) -- two nodes here

The problem description can be found here:


# OpenSees (Tcl) code by: Silvia Mazzoni & Frank McKenna, 2006

```python
import openseespy.opensees as op
import os
import math
op.wipe()

from InelasticFiberSectionPortal2Dframe import *

# MultipleSupport Earthquake ground motion (different displacement input at spec'd support nodes) -- two nodes here

# applying Dynamic Ground motion analysis
iSupportNode = [1, 2]
iGMfact = [1.5, 1.5]
iGMdirection = [1, 1]
iGMfile = ['H-E12140', 'H-E12140']
DtAnalysis = 0.01*sec  # time-step Dt for lateral analysis
TmaxAnalysis = 10.0*sec  # maximum duration of ground-motion analysis
Tol = 1e-8

# define DAMPING-----------------------------------------------

# apply Rayleigh DAMPING from $xDamp
D = alphaM*M + betaKcurr*Kcurrent + betaKcomm*KlastCommit + beatKinit*Kinitial
Lambda = op.eigen('-fullGenLapack', 1)  # eigenvalue mode 1
Omega = math.pow(Lambda, 0.5)
betaKcomm = 2 * (0.02/Omega)
xDamp = 0.02  # 2% damping ratio
```

(continues on next page)
alphaM = 0.0  # M-prop. damping; D = alphaM*M
betaKcurr = 0.0  # K-proportional damping; +beatKcurr*KCurrent
betaKinit = 0.0  # initial-stiffness proportional damping +beatKinit*Kini

op.rayleigh(alphaM, betaKcurr, betaKinit, betaKcomm)  # RAYLEIGH damping

# perform Dynamic Ground-Motion Analysis
# the following commands are unique to the Multiple-Support Earthquake excitation
# Set some parameters
IDloadTag = 400  # load tag
IDgmSeries = 500  # for multipleSupport Excitation

# read a PEER strong motion database file, extracts dt from the header and converts
# to the format OpenSees expects for Uniform/multiple-support ground motions
record = ['H-E12140', 'H-E12140']

import ReadRecord

# perform the conversion from SMD record to OpenSees record
for i in range(len(iSupportNode)):
    record_single = record[i]
    GMfatt = (iGMfact[i])*g
    dt, nPts = ReadRecord.ReadRecord(record_single+'.AT2', record_single+'.dat')
    op.timeSeries('Path', count, '-dt', dt, '-filePath', record_single+'.dat', '-factor', GMfatt)
    op.groundMotion(IDgmSeries+count, 'Plain', '-accel', count)
    op.imposedMotion(iSupportNode[i], iGMdirection[i], IDgmSeries+count)
    count = count + 1

maxNumIter = 10
op.wipeAnalysis()
op.constraints('Transformation')
op.numberer('RCM')
op.system('BandGeneral')
# op.test('EnergyIncr', Tol, maxNumIter)
# op.algorithm('ModifiedNewton')
# NewmarkGamma = 0.5
# NewmarkBeta = 0.25
# op.integrator('Newmark', NewmarkGamma, NewmarkBeta)
# op.analysis('Transient')

# Nsteps = int(TmaxAnalysis/ DtAnalysis)
# ok = op.analyze(Nsteps, DtAnalysis)
tCurrent = op.getTime()

# for gravity analysis, load control is fine, 0.1 is the load factor increment (http://opensees.berkeley.edu/wiki/index.php/Load_Control)


tFinal = nPts*dt
time = [tCurrent]
u3 = [0.0]
u4 = [0.0]
ok = 0

while tCurrent < tFinal:
    if ok == 0:
        tCurrent = op.getTime()
        time.append(tCurrent)
u3.append(op.nodeDisp(3,1))
u4.append(op.nodeDisp(4,1))
        print(test[i], algorithm[j], 'tCurrent=', tCurrent)

import matplotlib.pyplot as plt
plt.figure(figsize=(8,8))
plt.plot(time, u3)
plt.ylabel('Horizontal Displacement of node 3 (in)')
plt.xlabel('Time (s)')
plt.savefig('Horizontal Disp at Node 3 vs time-multiple support excitation-acctime.'+'.jpeg', dpi = 500)
plt.show()
2D Portal Frame with Units-Multiple Support Dynamic EQ Ground Motion-disptimeseries

Converted to openseespy by: Pavan Chigullapally
University of Auckland
Email: pchi893@aucklanduni.ac.nz

1. To run Uniaxial Inelastic Material, Fiber Section, Nonlinear Mode, MultipleSupport Earthquake ground motion

2. First import the InelasticFiberSectionPortal2Dframe.py

3. To run EQ ground-motion analysis (ReadRecord.py, H-E12140.DT2 needs to be downloaded into the same directory)

4. MultipleSupport Earthquake ground motion (different displacement input at specified support nodes) – two nodes here

5. The problem description can be found here (example:4)

6. The source code is shown below, which can be downloaded here.

```python
# -*- coding: utf-8 -*-

# Created on Mon Apr 22 15:12:06 2019

@author: pchi893

# Converted to openseespy by: Pavan Chigullapally
# University of Auckland
# Email: pchi893@aucklanduni.ac.nz

# Example4. 2D Portal Frame-- Dynamic EQ input analysis-- multiple-support
->excitation using displacement timeseries

#To run Uniaxial Inelastic Material, Fiber Section, Nonlinear Mode, MultipleSupport
->Earthquake ground motion: First import the InelasticFiberSectionPortal2Dframe.py
#(upto gravity loading is already in this script) and run the current script
#To run EQ ground-motion analysis (ReadRecord.py, H-E12140.DT2 needs to be downloaded
->into the same directory)
# MultipleSupport Earthquake ground motion (different displacement input at specified
->support nodes) -- two nodes here
#the problem description can be found here:
->berkeley.edu/wiki/index.php/OpenSees_Example_4._Portal_Frame(example: 4)

# OpenSees (Tcl) code by: Silvia Mazzoni & Frank McKenna, 2006

# -----------------------------------------------

# import openseespy.opensees as op
# import the os module
# import os

import openseespy.opensees as op
import math
```

(continues on next page)
op.wipe()
# execute this file after you have built the model, and after you apply gravity
# MultipleSupport Earthquake ground motion (different displacement input at spec'd support nodes) -- two nodes here
# applying Dynamic Ground motion analysis
iSupportNode = [1, 2]
iGMfact = [1.5, 1.25]
iGMdirection = [1, 1]
iGMfile = ['H-E12140', 'H-E12140']
DtAnalysis = 0.01*sec # time-step Dt for lateral analysis
TmaxAnalysis = 10.0*sec # maximum duration of ground-motion analysis
Tol = 1e-8

# define DAMPING---------------------------------------------------------------
# apply Rayleigh DAMPING from $xDamp
# $D = \alpha M + \beta Kcurr + \beta Kcomm + \beta Kinit$Kinit
Lambda = op.eigen('-fullGenLapack', 1)  # eigenvalue mode 1
Omega = math.pow(Lambda, 0.5)
betaKcomm = 2 * (0.02/Omega)
xDamp = 0.02  # 2% damping ratio
alphaM = 0.0  # M-prop. damping; $D = \alpha M$
betaKcurr = 0.0  # K-proportional damping; +beatKcurr*KCurrent
betaKinit = 0.0  # initial-stiffness proportional damping +beatKinit*Kini

op.rayleigh(alphaM,betaKcurr, betaKinit, betaKcomm)  # RAYLEIGH damping

#-------------------------------------------------------------------------------------
#--------------------------------- perform Dynamic Ground-Motion Analysis
# the following commands are unique to the Multiple-Support Earthquake excitation
# Set some parameters
IDloadTag = 400  # load tag
IDgmSeries = 500  # for multipleSupport Excitation

# read a PEER strong motion database file, extracts dt from the header and converts
# the file
# to the format OpenSees expects for Uniform/multiple-support ground motions
record = ['H-E12140', 'H-E12140']
#dt =[
#nPts = [

import ReadRecord
# Perform the conversion from SMD record to OpenSees record
#dt, nPts = ReadRecord.ReadRecord(record+'.at2', record+'.dat')
#print(dt, nPts)
count = 2
# use displacement series, create time series('Path'), then create multi-support excitation pattern (gmtag, 'Plain'), then create imposed ground motion
# using groundmotion('nodetag', gmtag), run this in a loop for each support or node
op.pattern('MultipleSupport', IDloadTag)
for i in range(len(iSupportNode)):
    record_single = record[i]
    GMfatt = (iGMfact[i])*cm
dt, nPts = ReadRecord.ReadRecord(record_single+'.DT2', record_single+'.dat')
    op.timeSeries('Path', count, '-dt', dt, '-filePath', record_single+'.dat', '-factor', GMfatt)
    op.groundMotion(IDgmSeries+count, 'Plain', '-disp', count)
    op.imposedMotion(iSupportNode[i], iGMdirection[i], IDgmSeries+count)
    count = count + 1
maxNumIter = 10
op.wipeAnalysis()
op.constraints('Transformation')
op.numberer('RCM')
op.system('BandGeneral')
#op.test('EnergyIncr', Tol, maxNumIter)
#op.algorithm('ModifiedNewton')
#NewmarkGamma = 0.5
#NewmarkBeta = 0.25
#op.integrator('Newmark', NewmarkGamma, NewmarkBeta)
#op.analysis('Transient')
#
#Nsteps = int(TmaxAnalysis/ DtAnalysis)
#
#ok = op.analyze(Nsteps, DtAnalysis)
#
tCurrent = op.getTime()

# for gravity analysis, load control is fine, 0.1 is the load factor increment (http://opensees.berkeley.edu/wiki/index.php/Load_Control)
#tFinal = TmaxAnalysis
#tFinal = nPts*dt
time = [tCurrent]
u3 = [0.0]
u4 = [0.0]
ok = 0

while tCurrent < tFinal:
    # ok = op.analyze(1, .01)
    # for i in test:
    #    for j in algorithm:
    #        if j < 4:
    #            op.algorithm(algorithm[j], '-initial')
    #        else:
    #            op.algorithm(algorithm[j])
    #    while ok == 0 and tCurrent < tFinal:
    #        op.test(test[i], Tol, maxNumIter)

(continues on next page)
NewmarkGamma = 0.5
NewmarkBeta = 0.25
op.integrator('Newmark', NewmarkGamma, NewmarkBeta)
op.analysis('Transient')
ok = op.analyze(1, .01)

if ok == 0 :
tCurrent = op.getTime()
time.append(tCurrent)
u3.append(op.nodeDisp(3,1))
u4.append(op.nodeDisp(4,1))
print(test[i], algorithm[j], 'tCurrent=', tCurrent)

import matplotlib.pyplot as plt
plt.figure(figsize=(8,8))
plt.plot(time, u3)
plt.ylabel('Horizontal Displacement of node 3 (in)')
plt.xlabel('Time (s)')
plt.savefig('Horizontal Disp at Node 3 vs time-multiple support excitation-disptime.jpeg', dpi = 500)
plt.show()

plt.figure(figsize=(8,8))
plt.plot(time, u4)
plt.ylabel('Horizontal Displacement of node 4 (in)')
plt.xlabel('Time (s)')
plt.savefig('Horizontal Disp at Node 4 vs time-multiple support excitation-disptime.jpeg', dpi = 500)
plt.show()

op.wipe()
Created on Mon Apr 22 15:12:06 2019

@author: pchi893

""

# Converted to openseespy by: Pavan Chigullapally
# University of Auckland
# Email: pchi893@aucklanduni.ac.nz

# Example4. 2D Portal Frame-- Dynamic EQ input analysis-- Bidirectional-uniform
# support excitation using acceleration timeseries

#To run Uniaxial Inelastic Material, Fiber Section, Nonlinear Mode, Bidirectional-
#uniform earthquake ground motion:First import the
#InelasticFiberSectionPortal2Dframe.py
#(upto gravity loading is already in this script) and run the current script
#To run EQ ground-motion analysis (ReadRecord.py, H-E12140.AT2 and H-E01140.AT2 needs
#to be downloaded into the same directory)
# Bidirectional-uniform support excitation using acceleration timeseries (different
# accelerations are input at all support nodes in two directions) -- two support
# nodes here
#the problem description can be found here:
#berkeley.edu/wiki/index.php/OpenSees_Example_4._Portal_Frame(example: 4)
# ------------------------------------------------------------------------------------
# OpenSees (Tcl) code by: Silvia Mazzoni & Frank McKenna, 2006
# ------------------------------------------------------------------------------------

import openseespy.opensees as op
import os
import math
op.wipe()

# define DAMPING----------------------------------------------------------------------
# apply Rayleigh DAMPING from $xDamp

# execute this file after you have built the model, and after you apply gravity
#
# MultipleSupport Earthquake ground motion (different displacement input at spec'd
# support nodes) -- two nodes here

#applying Dynamic Ground motion analysis
#iSupportNode = [1, 2]
iGMfact = [1.5, 0.25]
iGMdirection = [1, 2]
iGMfile = ['H-E01140', 'H-E12140']
DtAnalysis = 0.01*sec # time-step Dt for lateral analysis
TmaxAnalysis = 10.0*sec # maximum duration of ground-motion analysis
Tol = 1e-8

# define DAMPING---------------------------------------------------------------------
# apply Rayleigh DAMPING from $xDamp
# D = $\alpha M + \beta K \text{current} + \beta K \text{comm} \cdot K \text{last commit} + \beta K \text{init} \cdot K \text{init}$

Lambda = op.eigen('-fullGenLapack', 1) # eigenvalue mode 1

Omega = math.pow(Lambda, 0.5)

$\beta K \text{comm} = 2 \times (0.02/\Omega)$

$\alpha M = 0.0$ # M-proportional damping; $D = \alpha M \cdot M$

$\beta K \text{curr} = 0.0$ # K-proportional damping; +beatKcurr*KCurrent

$\beta K \text{init} = 0.0$ # initial-stiffness proportional damping +beatKinit*Kinit

op.rayleigh(alphaM, betaKcurr, betaKinit, betaKcomm) # RAYLEIGH damping

# perform Dynamic Ground-Motion Analysis

# the following commands are unique to the Multiple-Support Earthquake excitation

IDloadTag = 400 # load tag

# read a PEER strong motion database file, extracts dt from the header and converts
# to the format OpenSees expects for Uniform/multiple-support ground motions

record = ['H-E01140', 'H-E12140']

dt = []
nPts = []

import ReadRecord

# this is similar to uniform excitation in single direction

count = 2

for i in range(len(iGMdirection)):
    IDloadTag = IDloadTag+count
    record_single = record[i]
    GMfatt = (iGMfact[i])*g
dt, nPts = ReadRecord.ReadRecord(record_single+'.AT2', record_single+'.dat')

op.timeSeries('Path', count, '-dt', dt, '-filePath', record_single+'.dat', '-factor', GMfatt)

op.pattern('UniformExcitation', IDloadTag, iGMdirection[i], '-accel', 2)

count = count + 1

maxNumIter = 10

op.wipeAnalysis()

op.constraints('Transformation')

op.numberer('RCM')

op.system('BandGeneral')

# op.test('EnergyIncr', Tol, maxNumIter)

# op.algorithm('ModifiedNewton')

# NewmarkGamma = 0.5

# NewmarkBeta = 0.25

# op.integrator('Newmark', NewmarkGamma, NewmarkBeta)

# op.analysis('Transient')

# Nsteps = int(TmaxAnalysis/ DtAnalysis)

# ok = op.analyze(Nsteps, DtAnalysis)
tCurrent = op.getTime()

# for gravity analysis, load control is fine, 0.1 is the load factor increment (http://opensees.berkeley.edu/wiki/index.php/Load_Control)

#tFinal = TmaxAnalysis
tFinal = nPts*dt
time = [tCurrent]
u3 = [0.0]
u4 = [0.0]
ok = 0

while tCurrent < tFinal:
    # ok = op.analyze(1, .01)
    for i in test:
        for j in algorithm:
            if j < 4:
                op.algorithm(algorithm[j], '-initial')
            else:
                op.algorithm(algorithm[j])
                while ok == 0 and tCurrent < tFinal:
                    op.test(test[i], Tol, maxNumIter)
                    NewmarkGamma = 0.5
                    NewmarkBeta = 0.25
                    op.integrator('Newmark', NewmarkGamma, NewmarkBeta)
                    ok = op.analyze(1, .01)
                if ok == 0:
                    tCurrent = op.getTime()
                    time.append(tCurrent)
                    u3.append(op.nodeDisp(3,1))
                    u4.append(op.nodeDisp(4,1))
                    print(test[i], algorithm[j], 'tCurrent=', tCurrent)

import matplotlib.pyplot as plt
plt.figure(figsize=(8,8))
plt.plot(time, u3)
plt.ylabel('Horizontal Displacement of node 3 (in)')
plt.xlabel('Time (s)')
plt.savefig('Horizontal Disp at Node 3 vs time-uniform excitation-acctime.jpeg', dpi=500)
plt.show()

plt.figure(figsize=(8,8))
plt.plot(time, u4)
plt.ylabel('Horizontal Displacement of node 4 (in)')
plt.xlabel('Time (s)')
plt.savefig('Horizontal Disp at Node 4 vs time-uniform excitation-acctime.jpeg', dpi=500)
1.12.3 Tsunami Examples

1. Dambreak Analysis
2. Dambreak with Elastic Obstacle Analysis

Dambreak Analysis

1. The source code is shown below, which can be downloaded [here](#).
2. The folder `dambreak/` must exist before running the script.
3. Run the source code in your favorite Python program.
4. The ParaView is needed to view the results. To view the displaced shape of fluid, use the “Warp By Vector” filter with scale factor = 1.0.

```python
from openseespy.opensees import *
import numpy as np
import matplotlib.pyplot as plt

# ------------------------------
# Start of model generation
# ------------------------------

# remove existing model
op.wipe()

# set modelbuilder
model('basic', '-ndm', 2, '-ndf', 2)

# geometric
L = 0.146
H = L*2
H2 = 0.3
h = 0.005
alpha = 1.2
tw = 3*h

# material
rho = 1000.0
mu = 0.0001
b1 = 0.0
b2 = -9.81
thk = 0.012
kappa = -1.0

# time steps
dtmax = 1e-3
```

(continues on next page)
dtmin = 1e-6

totaltime = 1.0

# filename
filename = 'dambreak'

# recorder
recorder('PVD', filename, 'disp', 'vel', 'pressure')

# nodes
node(1, 0.0, 0.0)
nod(2, L, 0.0)
nod(3, L, H)
nod(4, 0.0, H)
nod(5, 0.0, H2)
nod(6, 4*L, 0.0)
nod(7, 4*L, H2)
nod(8, -tw, H2)
nod(9, -tw, -tw)
nod(10, 4*L+tw, -tw)
nod(11, 4*L+tw, H2)

# fluid mesh
fluid = 4
ndf = 2
id = -1
mesh('line', 1, 9, 4,5,8,9,10,11,7,6,2, id, ndf, h)
mesh('line', 2, 3, 2,1,4, id, ndf, h)
mesh('line', 3, 3, 2,3,4, id, ndf, h)
eleArgs = ['PFEMElementBubble',rho,mu,b1,b2,thk,kappa]
mesh('tri', fluid, 2, 2,3, id, ndf, h, *eleArgs)

# wall mesh
wall = 5
id = 1
mesh('tri', wall, 2, 1,2, id, ndf, h)

for nd in getNodeTags('-mesh', wall):
    fix(nd, 1,1)

# save the original modal
record()

# create constraint object
constraints('Plain')

# create numberer object
numberer('Plain')

# create convergence test object
test('PFEM', 1e-5, 1e-5, 1e-5, 1e-5, 1e-15, 1e-15, 100, 3, 1, 2)

# create algorithm object
algorithm('Newton')

# create integrator object
integrator('PFEM')

# create SOE object
system('PFEM')

# create analysis object
analysis('PFEM', dtmax, dtmin, b2)

# analysis
while getTime() < totaltime:

    # analysis
    if analyze() < 0:
        break

    remesh(alpha)

**Dambreak with Elastic Obstacle Analysis**

1. The source code is shown below, which can be downloaded [here](#).
2. The folder **obstacle/** must exist before running the script.
3. Run the source code in your favorite Python program.
4. The **ParaView** is needed to view the results. To view the displaced shape of fluid, use the “Warp By Vector” filter with scale factor = 1.0.

```python
from openseespy.opensees import *
import numpy as np
import matplotlib.pyplot as plt

# Start of model generation

# remove existing model
wipe()

# set modelbuilder
model('basic', '-ndm', 2, '-ndf', 3)

# geometric
L = 0.146
H = 2*L
H2 = 0.3
b = 0.012
h = 0.005
alpha = 1.2
Hb = 20.0*b/3.0
tw = 3*h
```

(continues on next page)
# material
rho = 1000.0
mu = 0.0001
b1 = 0.0
b2 = -9.81
thk = 0.012
kappa = -1.0
#kappa = 2.15e9

rhos = 2500.0
A = thk*thk
E = 1e6
Iz = thk*thk*thk*thk/12.0
bmass = A+Hb*rhos

# analysis
dtmax = 1e-3
dtmin = 1e-6
totaltime = 1.0
filename = 'obstacle'

# recorder
recorder('PVD', filename, 'disp', 'vel', 'pressure')

# nodes
node(1, 0.0, 0.0)
nod(2, L, 0.0)
nod(3, L, H, '-ndf', 2)
node(4, 0.0, H)
node(5, 0.0, H2)
node(6, 4*L, 0.0)
node(7, 4*L, H2)
node(8, -tw, H2)
node(9, -tw, -tw)
node(10, 4*L-tw, -tw)
node(11, 4*L-tw, H2)
node(12, 2*L, 0.0)
node(13, 2*L, Hb)

# transformation
transfTag = 1
geomTransf('Corotational', transfTag)

# section
secTag = 1
section( 'Elastic', secTag, E, A, Iz)

# beam integration
inteTag = 1
numpts = 2
beamIntegration( 'Legendre', inteTag, secTag, numpts)

# beam mesh
beam = 6
id = 1
n_df = 3
mesh('line', beam, 2, 12, 13, id, n_df, h, 'dispBeamColumn', transfTag, inteTag)

# fluid mesh
fluid = 4
n_df = 2
id = -1
mesh('line', 1, 10, 4, 5, 8, 9, 10, 11, 7, 6, 12, 2, id, n_df, h)
mesh('line', 2, 3, 2, 1, 4, id, n_df, h)
mesh('line', 3, 3, 2, 3, 4, id, n_df, h)
eleArgs = ['PFEMElementBubble', rho, mu, b1, b2, thk, kappa]
mesh('tri', fluid, 2, 2, 3, id, n_df, h, *eleArgs)

# wall mesh
wall = 5
id = 1
mesh('tri', wall, 2, 1, 2, id, n_df, h)

for nd in getNodeTags('-mesh', wall):
    fix(nd, 1, 1, 1)

# save the original modal
record()

# create constraint object
constraints('Plain')

# create numberer object
numberer('Plain')

# create convergence test object
test('PFEM', 1e-5, 1e-5, 1e-5, 1e-5, 1e-15, 1e-15, 100, 3, 1, 2)

# create algorithm object
algorithm('Newton')

# create integrator object
integrator('PFEM')

# create SOE object
system('PFEM')

# create analysis object
analysis('PFEM', dtmax, dtmin, b2)

while getTime() < totaltime:

    # analysis
    if analyze() < 0:
        break

    remesh(alpha)
1.12.4 GeoTechnical Examples

1. Laterally-Loaded Pile Foundation

Laterally-Loaded Pile Foundation

1. The original model can be found here.

2. The Python code is converted by Pavan Chigullapally from University of Auckland, Auckland (pchi893@aucklanduni.ac.nz), and shown below, which can be downloaded here.

```python
# -*- coding: utf-8 -*-

Created on Thu Jan 10 18:24:47 2019

@Author: pchi893

# references
# American Petroleum Institute (API) (1987). Recommended Practice for Planning,
# Designing and Constructing Fixed Offshore Platforms. API Recommended Practice 2A(RP-2A),
#
# transversal forces.” Bulletin No. 12, Geoteknisk Institute, Copenhagen, 59.
#
# Boulanger, R. W., Kutter, B. L., Brandenberg, S. J., Singh, P., and Chang, D.
# (2003). Pile Foundations in liquefied and laterally spreading ground during earthquakes:
# Centrifuge experiments and analyses. Center for Geotechnical Modeling, University of California at Davis,
# Davis, CA.
#
# Rep. UCD/CGM-03/01.
```

(continues on next page)
import math

def get_pyParam ( pyDepth, gamma, phiDegree, b, pEleLength, puSwitch, kSwitch, gwtSwitch):

    #----------------------------------------------------------
    # define ultimate lateral resistance, pult
    #----------------------------------------------------------

    # pult is defined per API recommendations (Reese and Van Impe, 2001 or API, 1987) for puSwitch = 1
    # OR per the method of Brinch Hansen (1961) for puSwitch = 2

    pi = 3.14159265358979
    phi = phiDegree * (pi/180)
    zbRatio = pyDepth / b

    #-------API recommended method-------

    if puSwitch == 1:
        # obtain loading-type coefficient A for given depth-to-diameter ratio zb
        # ---> values are obtained from a figure and are therefore approximate
        zb = []
        dataNum = 41
        for i in range(dataNum):
            b1 = i * 0.125
            zb.append(b1)
            As = [2.8460, 2.7105, 2.6242, 2.4271, 2.3409, 2.2546, 2.1437, 2.0575, 1.9589, 1.8973, 1.8111, 1.6632, 1.5893, 1.5277, 1.4415, 1.3799, 1.3368, 1.3036, 1.2905, 1.2590, 1.2074, 1.1581, 1.1211, 1.0780, 1.0349, 1.0164, 0.9979, 0.9733, 0.9610, 0.9487, 0.9363, 0.9211, 0.8994, 0.8994, 0.8871, 0.8871, 0.8809, 0.8809, 0.8809, 0.8809, 0.8809]

        # linear interpolation to define A for intermediate values of depth:diameter ratio
        for i in range(dataNum):
            if zbRatio >= 5.0:
                A = 0.88
            elif zb[i] <= zbRatio and zbRatio <= zb[i+1]:
                A = (As[i+1] - As[i])/(zb[i+1] - zb[i]) * (zbRatio-zb[i]) + As[i]

    # define common terms
    alpha = phi / 2
    beta = pi / 4 + phi / 2
    K0 = 0.4
    tan_1 = math.tan(pi / 4 - phi / 2)
    Ka = math.pow(tan_1 , 2)

    # terms for Equation (3.44), Reese and Van Impe (2001)
    tan_2 = math.tan(phi)
    tan_3 = math.tan(beta - phi)

(continues on next page)
\begin{verbatim}

sin_1 = math.sin(beta)
c1 = K0 * tan_2 * sin_1 / (tan_3*cos_1)
tan_4 = math.tan(beta)
tan_5 = math.tan(alpha)
c2 = (tan_4/tan_3)*tan_4 * tan_5
c3 = K0 * tan_4 * (tan_2 * sin_1 - tan_5)
c4 = tan_4 / tan_3 - Ka

tan_6 = math.tan(pi/4+phi/2)
sin_2 = math.sin(phi)
sin_3 = math.sin(pi/4 + phi/2)
exp_1 = math.exp((pi/2+phi)*tan_2)
exp_2 = math.exp(-(pi/2-phi) * tan_2)
Kqo = exp_1 * cos_2 * tan_6 - exp_2 * cos_2 * tan_1
Kco = (1/tan_2) * (exp_1 * cos_2 * tan_6 - 1)

# terms for Equation (3.45), Reese and Van Impe (2001)
pow_1 = math.pow(tan_4,8)
pow_2 = math.pow(tan_4,4)
c5 = Ka * (pow_1-1)
c6 = K0 * tan_2 * pow_2

# Equation (3.44), Reese and Van Impe (2001)
pst = gamma * pyDepth * (pyDepth * (c1 + c2 + c3) + b * c4)

# Equation (3.45), Reese and Van Impe (2001)
psd = b * gamma * pyDepth * (c5 + c6)

# pult is the lesser of pst and psd. At surface, an arbitrary value is defined
if pst <=psd:
    if pyDepth == 0:
        pu = 0.01
    else:
        pu = A * pst
else:
    pu = A * psd

# PySimple1 material formulated with pult as a force, not force/length. Multiply by trib. length
pult = pu * pEleLength

#-------Brinch Hansen method-------
elif puSwitch == 2:
    # pressure at ground surface
cos_2 = math.cos(phi)
tan_6 = math.tan(pi/4+phi/2)
sin_2 = math.sin(phi)
sin_3 = math.sin(pi/4 + phi/2)
exp_1 = math.exp((pi/2+phi)*tan_2)
exp_2 = math.exp(-(pi/2-phi) * tan_2)
Kqo = exp_1 * cos_2 * tan_6 - exp_2 * cos_2 * tan_1
Kco = (1/tan_2) * (exp_1 * cos_2 * tan_6 - 1)

# pressure at great depth
exp_3 = math.exp(pi * tan_2)

\end{verbatim}

(continues on next page)
147  pow_3 = math.pow(tan_2, 4)
148  pow_4 = math.pow(tan_6, 2)
149  dcinf = 1.58 + 4.09 * (pow_3)
150  Nc = (1/tan_2)*exp_3*(pow_4 - 1)
151  Ko = 1 - sin_2
152  Kcinf = Nc * dcinf
153  Kqinf = Kcinf * Ko * tan_2

154  # pressure at an arbitrary depth
155  aq = (Kqo/(Kqinf - Kqo))*(Ko*sin_2/sin_3)
156  KqD = (Kqo + Kqinf * aq * zbRatio)/(1 + aq * zbRatio)

157  # ultimate lateral resistance
158  if pyDepth == 0:
159      pu = 0.01
160  else:
161      pu = gamma * pyDepth * KqD * b

162  # PySimple1 material formulated with pult as a force, not force/length, multiply by trib. length
163  pult = pu * pEleLength

164  #----------------------------------------------------------
165  # define displacement at 50% lateral capacity, y50
166  #----------------------------------------------------------
167  # values of y50 depend of the coefficient of subgrade reaction, k, which can be defined in several ways.
168  # for gwtSwitch = 1, k reflects soil above the groundwater table
169  # for gwtSwitch = 2, k reflects soil below the groundwater table
170  # a linear variation of k with depth is defined for kSwitch = 1 after API (1987)
171  # a parabolic variation of k with depth is defined for kSwitch = 2 after Boulanger et al. (2003)
172  # API (1987) recommended subgrade modulus for given friction angle, values obtained from figure (approximate)
173  ph = [28.8, 29.5, 30.0, 31.0, 32.0, 33.0, 34.0, 35.0, 36.0, 37.0, 38.0, 39.0, 40.0]

174  # subgrade modulus above the water table
175  if gwtSwitch == 1:
176      k = [10, 23, 45, 61, 80, 100, 140, 160, 182, 215, 250, 275]
177  else:
178      k = [10, 20, 33, 42, 50, 60, 70, 85, 95, 107, 122, 141, 155]

179  dataNum = 13
180  for i in range(dataNum):
181      if ph[i] <= phiDegree and phiDegree <= ph[i+1]:
182          khat = (k[i+1]-k[i])/(ph[i+1]-ph[i])*(phiDegree - ph[i]) + k[i]
183  # change units from (lb/in^3) to (kN/m^3)
184  k_SIunits = khat * 271.45
185  # define parabolic distribution of k with depth if desired (i.e. lin_par switch == 2)

(continues on next page)
sigV = pyDepth * gamma

if sigV == 0:
    sigV = 0.01

if kSwitch == 2:
    # Equation (5-16), Boulanger et al. (2003)
    cSigma = math.pow(50 / sigV, 0.5)
    # Equation (5-15), Boulanger et al. (2003)
    k_SIunits = cSigma * k_SIunits
    # define y50 based on pult and subgrade modulus k

    # based on API (1987) recommendations, p-y curves are described using tanh
    # functions.
    # i.e. atanh(x) = 1/2*ln((1+x)/(1-x)), |x| < 1
    # when half of full resistance has been mobilized, p(y50)/pult = 0.5
    x = 0.5
    log_1 = math.log((1+x)/(1-x))
    atanh_value = 0.5 * log_1
    # need to be careful at ground surface (don’t want to divide by zero)
    if pyDepth == 0.0:
        pyDepth = 0.01
    y50 = 0.5 * (pu/ A)/(k_SIunits * pyDepth) * atanh_value
    # return pult and y50 parameters
    outResult = []
    outResult.append(pult)
    outResult.append(y50)
    return outResult

# references
def get_qzParam (phiDegree, b, sigV, G):
    pi = 3.14159265358979
    pa = 101
    sin_4 = math.sin(phiDegree * (pi/180))
    Ko = 1 - sin_4

    # ultimate tip pressure can be computed by qult = Nq*sigV after Meyerhof (1976)
    # where Nq is a bearing capacity factor, phi is friction angle, and sigV is eff.
    # overburden
    phi = phiDegree * (pi/180)

    # rigidity index
    tan_7 = math.tan(phi)
    Ir = G/(sigV * tan_7)

    # bearing capacity factor
    tan_8 = math.tan(pi/4+phi/2)
    sin_5 = math.sin(phi)
    pow_4 = math.pow(tan_8,2)
    pow_5 = math.pow(Ir,4*sin_5)/(3*(1+sin_5))
    exp_4 = math.exp(pi/2-phi)

    Nq = (1+2*Ko)*(1/(3-sin_5))*exp_4*(pow_4)*(pow_5)

    # tip resistance
    qu = Nq * sigV

    # QzSimple1 material formulated with qult as force, not stress, multiply by area of
    # pile tip
    pow_6 = math.pow(b, 2)
    qult = qu * pi*pow_6/4

    # the q-z curve of Vijayvergiya (1977) has the form, q(z) = qult*(z/zc)^(1/3)
    # where zc is critical tip deflection given as ranging from 3-9% of the
    # pile diameter at the tip.

    # assume zc is 5% of pile diameter
    zc = 0.05 * b

    # based on Vijayvergiya (1977) curve, z50 = 0.125*zc
    z50 = 0.125 * zc

    # return values of qult and z50 for use in q-z material
    outResult = []
    outResult.append(qult)
    outResult.append(z50)

(continues on next page)
def get_tzParam (phi, b, sigV, pEleLength):

    # references
    # Mosher, R.L. (1984). "Load transfer criteria for numerical analysis of
    # axial loaded piles in sand." U.S. Army Engineering and Waterways
    # Experimental Station, Automatic Data Processing Center, Vicksburg, Miss.
    #
    #
    pi = 3.14159265358979

    # Compute tult based on tult = Ko*sigV*pi*dia*tan(delta), where
    # Ko is coeff. of lateral earth pressure at rest,
    # taken as Ko = 0.4
    # delta is interface friction between soil and pile,
    # taken as delta = 0.8*phi to be representative of a
    # smooth precast concrete pile after Kulhawy (1991)
    delta = 0.8 * phi * pi/180

    # if z = 0 (ground surface) need to specify a small non-zero value of sigV
    if sigV == 0.0:
        sigV = 0.01
    tan_9 = math.tan(delta)
    tu = 0.4 * sigV * pi * b * tan_9

    # TzSimple1 material formulated with tult as force, not stress, multiply by
    # tributary length of pile
    tult = tu * pEleLength

    # Mosher (1984) provides recommended initial tangents based on friction angle
    # values are in units of psf/in
    kf = [6000, 10000, 10000, 14000, 14000, 18000]
    fric = [28, 31, 32, 34, 35, 38]
dataNum = len(fric)

    # determine kf for input value of phi, linear interpolation for intermediate values
    if phi < fric[0]:
        k = kf[0]
    elif phi > fric[5]:
        k = kf[5]
    else:
        for i in range(dataNum):
            if fric[i] <= phi and phi <= fric[i+1]:
                k = ((kf[i+1] - kf[i])/(fric[i+1] - fric[i])) * (phi - fric[i]) +
        kf[i]

    # need to convert kf to units of kN/m^3
    kSIunits = k * 1.885

    # based on a t-z curve of the shape recommended by Mosher (1984), z50 = tult/kf
    z50 = tult / kSIunits

    # return values of tult and z50 for use in t-z material
    outResult = []
    outResult.append(tult)
    outResult.append(z50)

    return outResult

from openseespy.opensees import *

(continues on next page)


```python
op.wipe()

# all the units are in SI units N and mm
#----------------------------------------------------------
# pile geometry and mesh
#----------------------------------------------------------
# length of pile head (above ground surface) (m)
L1 = 1.0
# length of embedded pile (below ground surface) (m)
L2 = 20.0
# pile diameter
diameter = 1.0

# number of pile elements
nElePile = 84
# pile element length
eleSize = (L1+L2)/nElePile

# number of total pile nodes
nNodePile = 1 + nElePile

#----------------------------------------------------------
# create spring nodes
#----------------------------------------------------------
# spring nodes created with 3 dim, 3 dof
op.model('basic', '-ndm', 3, '-ndf', 3)

# counter to determine number of embedded nodes
count = 0

# create spring nodes
#1 to 85 are spring nodes

pile_nodes = dict()

for i in range(nNodePile):
    zCoord = eleSize * i
    if zCoord <= L2:
        op.node(i+1, 0.0, 0.0, zCoord)
        op.node(i+101, 0.0, 0.0, zCoord)
        pile_nodes[i+1] = (0.0, 0.0, zCoord)
        pile_nodes[i+101] = (0.0, 0.0, zCoord)
        count = count + 1

print("Finished creating all spring nodes...")

# number of embedded nodes
nNodeEmbed = count
```

(continues on next page)
# spring node fixities

```python
for i in range(nNodeEmbed):
    op.fix(i+1, 1, 1, 1)
    op.fix(i+101, 0, 1, 1)

print("Finished creating all spring node fixities...")
```

# soil properties

```python
# soil unit weight (kN/m^3)
gamma = 17.0

# soil internal friction angle (degrees)
phi = 36.0

# soil shear modulus at pile tip (kPa)
Gsoil = 150000.0
```

# select pult definition method for p-y curves

```python
# API (default) --> 1
# Brinch Hansen --> 2
puSwitch = 1
```

# variation in coefficent of subgrade reaction with depth for p-y curves

```python
# API linear variation (default) --> 1
# modified API parabolic variation --> 2
kSwitch = 1
```

# effect of ground water on subgrade reaction modulus for p-y curves

```python
# above gwt --> 1
# below gwt --> 2
gwtSwitch = 1
```

# create spring material objects

```python
# p-y spring material

for i in range(1, nNodeEmbed+1):
    # depth of current py node
    pyDepth = L2 - eleSize * (i-1)
    # procedure to define pult and y50
    pyParam = get_pyParam(pyDepth, gamma, phi, diameter, eleSize, puSwitch, kSwitch, gwtSwitch)
    pult = pyParam[0]
    y50 = pyParam[1]
    op.uniaxialMaterial('PySimple1', i, 2, pult, y50, 0.0)
```

# t-z spring material

```python
for i in range(2, nNodeEmbed+1):
    # depth of current tz node
    pyDepth = eleSize * (i-1)
    # vertical effective stress at current depth
    sigV = gamma * pyDepth
```
# procedure to define tult and z50

tzParam = get_tzParam(phi, diameter, sigV, eleSize)
tult = tzParam [0]
z50 = tzParam [1]

op.uniaxialMaterial('TzSimple1', i+100, 2, tult, z50, 0.0)

# q-z spring material

# vertical effective stress at pile tip, no water table (depth is embedded pile length)
sigVq = gamma * L2

# procedure to define qult and z50
qzParam = get_qzParam (phi, diameter, sigVq, Gsoil)
qult = qzParam [0]
z50q = qzParam [1]

#op.uniaxialMaterial('QzSimple1', 101, 2, qult, z50q) #, 0.0, 0.0

op.uniaxialMaterial('TzSimple1', 101, 2, qult, z50q, 0.0)

print("Finished creating all p-y, t-z, and z-z spring material objects...")

#----------------------------------------------------------
# create zero-length elements for springs
#----------------------------------------------------------

# element at the pile tip (has q-z spring)

op.element('zeroLength', 1001, 1, 101, '-mat', 1, 101, '-dir', 1, 3)

# remaining elements
for i in range(2, nNodeEmbed+1):
    op.element('zeroLength', 1000+i, i, 100+i, '-mat', i, 100+i, '-dir', 1, 3)

print("Finished creating all zero-Length elements for springs...")

#----------------------------------------------------------
# create pile nodes
#----------------------------------------------------------

# pile nodes created with 3 dimensions, 6 degrees of freedom
op.model('basic', '-ndm', 3, '-ndf', 6)

# create pile nodes
for i in range(1, nNodePile+1):
    zCoord = eleSize * i
    op.node(i+200, 0.0, 0.0, zCoord)

print("Finished creating all pile nodes...")

# create coordinate-transformation object
op.geomTransf('Linear', 1, 0.0, -1.0, 0.0)

# create fixity at pile head (location of loading)
op.fix(200+nNodePile, 0, 1, 0, 1, 0, 1)
# create fixities for remaining pile nodes
for i in range(201, 200+nNodePile):
    op.fix(i, 0, 1, 0, 1, 0, 1)
print("Finished creating all pile node fixities...")

# define equal dof between pile and spring nodes
for i in range(1, nNodeEmbed+1):
    op.equalDOF(200+i, 100+i, 1, 3)
print("Finished creating all equal degrees of freedom...")

# pile section
secTag = 1
E = 25000000.0
A = 0.785
Iz = 0.049
Iy = 0.049
G = 9615385.0
J = 0.098
matTag = 3000
op.section('Elastic', 1, E, A, Iz, Iy, G, J)

# elastic torsional material for combined 3D section
op.uniaxialMaterial('Elastic', 3000, 1e10)

# create combined 3D section
secTag3D = 3
op.section('Aggregator', secTag3D, 3000, 'T', '-section', 1)
# create pile elements
#----------------------------------------------------------
op.beamIntegration('Legendre', 1, secTag3D, 3)  # we are using gauss-Legendre integration as it is the default integration scheme used in opensees tcl (check dispBeamColumn)

for i in range(201, 201+nElePile):
  op.element('dispBeamColumn', i, i, i+1, 1, 1)

print("Finished creating all pile elements...")

# create recorders
#----------------------------------------------------------

# record information at specified increments

# record displacements at pile nodes
op.recorder('Node', '-file', 'pileDisp.out','-time', '-dT', timeStep, '-nodeRange', 201, 200 + nNodePile, '-dof', 1,2,3, 'disp')

# record reaction force in the p-y springs
op.recorder('Node', '-file', 'reaction.out','-time', '-dT', timeStep, '-nodeRange', 1, nNodePile, '-dof', 1, 'reaction')

# record element forces in pile elements
op.recorder('Element', '-file', 'pileForce.out','-time', '-dT', timeStep, '-eleRange', 201, 200+nElePile, 'globalForce')

print("Finished creating all recorders...")

# create the loading
#----------------------------------------------------------

op.setTime(10.0)

# apply point load at the uppermost pile node in the x-direction
values = [0.0, 0.0, 1.0, 1.0]
time = [0.0, 10.0, 20.0, 10000.0]
nodeTag = 200+nNodePile
loadValues = [3500.0, 0.0, 0.0, 0.0, 0.0, 0.0]
op.timeSeries('Path', 1, '-values', *values, '-time', *time, '-factor', 1.0)
op.pattern('Plain', 10, 1)
op.load(nodeTag, *loadValues)

print("Finished creating loading object...")

# create the analysis
#----------------------------------------------------------
op.integrator('LoadControl', 0.05)
op.numberer('RCM')
(continued from previous page)

```python
op.system('SparseGeneral')
op.constraints('Transformation')
op.test('NormDispIncr', 1e-5, 20, 1)
op.algorithm('Newton')
op.analysis('Static')

print("Starting Load Application...")
op.analyze(201)

print("Load Application finished...")
# print("Loading Analysis execution time: [expr $endT-$startT] seconds.")
# op.wipe

op.reactions()
Nodereactions = dict()
Nodedisplacements = dict()
for i in range(201, nodeTag+1):
    Nodereactions[i] = op.nodeReaction(i)
    Nodedisplacements[i] = op.nodeDisp(i)
print('Node Reactions are: ', Nodereactions)
print('Node Displacements are: ', Nodedisplacements)
```

1.12.5 Thermal Examples

1. **Restrained beam under thermal expansion**

**Restrained beam under thermal expansion**

1. The original model can be found here.
2. The Python source code is shown below, which can be downloaded [here](#).
3. Make sure the numpy and matplotlib packages are installed in your Python distribution.
4. Run the source code in your favorite Python program and should see
from openseespy.opensees import *

import numpy as np
import matplotlib.pyplot as plt

# define model
model('basic', '-ndm', 2, '-ndf', 3)

# define node
node(1, 0.0, 0.0)
node(2, 2.0, 0.0)
node(3, 1.0, 0.0)

# define boundary condition
fix(1, 1, 1, 1)
fix(2, 1, 1, 1)
fix(3, 0, 1, 1)

# define an elastic material with Tag=1 and E=2e11.
matTag = 1
uniaxialMaterial('Steel01Thermal', 1, 2e11, 2e11, 0.01)

# define fibred section Two fibres: fiber $yLoc $zLoc $A $matTag
secTag = 1
section('FiberThermal', secTag)
fiber(-0.025, 0.0, 0.005, matTag)
fiber(0.025, 0.0, 0.005, matTag)

#define coordinate transformation
transfTag = 1
geomTransf('Linear', transfTag)

#define beam integration
np = 3
biTag = 1
beamIntegration('Lobatto', biTag, secTag, np)

#define beam element
element('dispBeamColumnThermal', 1, 1, 3, transfTag, biTag)
element('dispBeamColumnThermal', 2, 3, 2, transfTag, biTag)

#define time series
tsTag = 1
timeSeries('Linear', tsTag)

#define load pattern
maxtemp = 1000.0
pattern('Plain', patternTag, tsTag)
eleLoad('-ele', 1, '-type', '-beamThermal', 1000.0, -0.05, 1000.0, 0.05)
eleLoad('-ele', 2, '-type', '-beamThermal', 0, -0.05, 0, 0.05)

#define analysis
incrtemp = 0.01
system('BandGeneral')
constraints('Plain')
numberer('Plain')
test('NormDispIncr', 1.0e-3, 100, 1)
algorithm('Newton')
integrator('LoadControl', incrtemp)
analysis('Static')

# analysis
nstep = 100
temp = [0.0]
disp = [0.0]
for i in range(nstep):
    if analyze(1) < 0:
        break
temp.append(getLoadFactor(patternTag)*maxtemp)
disp.append(nodeDisp(3,1))

plt.plot(temp, disp, '-o')
plt.xlabel('Temperature')
plt.ylabel('Nodal displacement')
plt.grid()
plt.show()
1.12.6 Parallel Examples

1. **Hello World Example 1**
2. **Hello World Example 2**
3. **Parallel Truss Example**
4. **Parallel Tri31 Example**
5. **Parallel Parametric Study Example**

**Hello World Example 1**

1. The source code is shown below, which can be downloaded here.
2. Run the source code with 4 processors

```python
import openseespy.opensees as ops

pid = ops.getPID()
np = ops.getNP()

print('Hello World Process:', pid)
if pid == 0:
    print('Total number of processes:', np)
```

the outputs look like

```
Hello World Process: 1
Hello World Process: 2
Hello World Process: 0
Total number of processes: 4
Hello World Process: 3
Process 1 Terminating
Process 2 Terminating
Process 0 Terminating
Process 3 Terminating
```

The script is shown below

```python
import openseespy.opensees as ops

pid = ops.getPID()
np = ops.getNP()

print('Hello World Process:', pid)
if pid == 0:
    print('Total number of processes:', np)
```

**Hello World Example 2**

1. The source code is shown below, which can be downloaded here.
2. Run the source code with 4 processors

```bash
mpiexec -np 4 python hello2.py
```

the outputs look like

```
Random:
Hello from 2
Hello from 1
```
Hello from 3

Ordered:
Hello from 1
Hello from 2
Hello from 3

Broadcasting:
Hello from 0
Hello from 0
Process 3 Terminating
Process 2 Terminating
Process 1 Terminating
Process 0 Terminating

The script is shown below

```python
import openseespy.opensees as ops

pid = ops.getPID()
np = ops.getNP()

# datatype = 'float'
# datatype = 'int'
datatype = 'str'

if pid == 0:
    print('Random: ')
    for i in range(1, np):
        data = ops.recv('-pid', 'ANY')
        print(data)
else:
    if datatype == 'str':
        ops.send('-pid', 0, 'Hello from {}' .format(pid))
    elif datatype == 'float':
        ops.send('-pid', 0, float(pid))
    elif datatype == 'int':
        ops.send('-pid', 0, int(pid))

ops.barrier()

if pid == 0:
    print('Ordered: ')
    for i in range(1, np):
        data = ops.recv('-pid', i)
        print(data)
else:
    if datatype == 'str':
        ops.send('-pid', 0, 'Hello from {}' .format(pid))
    elif datatype == 'float':
        ops.send('-pid', 0, float(pid))
    elif datatype == 'int':
        ops.send('-pid', 0, int(pid))
```

(continues on next page)
ops.barrier()
if pid == 0:
    print('nBroadcasting: ')
    if datatype == 'str':
        ops.Bcast('Hello from {}'.format(pid))
    elif datatype == 'float':
        ops.Bcast(float(pid))
    elif datatype == 'int':
        ops.Bcast(int(pid))
else:
    data = ops.Bcast()
    print(data)

Parallel Truss Example

1. The source code is shown below, which can be downloaded here.
2. Run the source code with 2 processors

```
mpiexec -np 2 python paralleltruss.py
```

the outputs look like

```
Node 4: [[72.0, 96.0], [0.5300927771322836, -0.17789363846931772]]
Node 4: [[72.0, 96.0], [0.5300927771322836, -0.17789363846931772]]
Node 4: [[72.0, 96.0], [1.530092777132284, -0.19400676316761836]]
Node 4: [[72.0, 96.0], [1.530092777132284, -0.19400676316761836]]
opensees.msg: TIME(sec) Real: 0.208238

opensees.msg: TIME(sec) Real: 0.209045
```

Process 0 Terminating
Process 1 Terminating

The script is shown below

```
import openseespy.opensees as ops
pid = ops.getPID()
np = ops.getNP()
ops.start()
if np != 2:
    exit()

ops.model('basic', '-ndm', 2, '-ndf', 2)
ops.uniaxialMaterial('Elastic', 1, 3000.0)

if pid == 0:
    ops.node(1, 0.0, 0.0)
    ops.node(4, 72.0, 96.0)
    ops.fix(1, 1, 1)
    ops.element('Truss', 1, 1, 4, 10.0, 1)
    ops.timeSeries('Linear', 1)
```

(continues on next page)
Parallel Tri31 Example

1. The source code is shown below, which can be downloaded here.
2. Run the source code with 4 processors

```python
ops.pattern('Plain', 1, 1)
ops.load(4, 100.0, -50.0)
else:
    ops.node(2, 144.0, 0.0)
    ops.node(3, 168.0, 0.0)
    ops.node(4, 72.0, 96.0)
    ops.fix(2, 1, 1)
    ops.fix(3, 1, 1)
    ops.element('Truss', 2, 2, 4, 5.0, 1)
    ops.element('Truss', 3, 3, 4, 5.0, 1)
    ops.constraints('Transformation')
    ops.numberer('ParallelPlain')
    ops.system('Mumps')
    ops.test('NormDispIncr', 1e-6, 6, 2)
    ops.algorithm('Newton')
    ops.integrator('LoadControl', 0.1)
    ops.analysis('Static')
ops.analyze(10)
print('Node 4: ', [ops.nodeCoord(4), ops.nodeDisp(4)])
ops.loadConst('-time', 0.0)
if pid == 0:
    ops.pattern('Plain', 2, 1)
    ops.load(4, 1.0, 0.0)
ops.domainChange()
ops.integrator('ParallelDisplacementControl', 4, 1, 0.1)
ops.analyze(10)
print('Node 4: ', [ops.nodeCoord(4), ops.nodeDisp(4)])
ops.stop()
```

the outputs look like

```
opensees.msg: TIME(sec) Real: 0.177647
opensees.msg: TIME(sec) Real: 0.187682
opensees.msg: TIME(sec) Real: 0.193193
```
The script is shown below

```python
import openseespy.opensees as ops

pid = ops.getPID()
np = ops.getNP()
ops.start()

ops.model('basic', '-ndm', 2, '-ndf', 2)

L = 48.0
H = 4.0
Lp = L / np
ndf = 2
meshsize = 0.05

ops.node(pid, Lp * pid, 0.0)
ops.node(pid + 1, Lp * (pid + 1), 0.0)
ops.node(np + pid + 2, Lp * (pid + 1), H)
ops.node(np + pid + 1, Lp * pid, H)

sid = 1
ops.setStartNodeTag(2 * np + 2 + pid * int(H / meshsize + 10))
ops.mesh('line', 3, 2, pid, np + pid + 1, sid, ndf, meshsize)
ops.setStartNodeTag(2 * np + 2 + (pid + 1) * int(H / meshsize + 10))
ops.mesh('line', 4, 2, pid + 1, np + pid + 2, sid, ndf, meshsize)
ops.setStartNodeTag(int(2 * L / meshsize + (np + 1) * H / meshsize * 2) + pid * int(H * L / meshsize ** 2 * 2))
ops.mesh('line', 1, 2, pid, pid + 1, sid, ndf, meshsize)
ops.mesh('line', 2, 2, np + pid + 1, np + pid + 2, sid, ndf, meshsize)

ops.nDMaterial('ElasticIsotropic', 1, 3000.0, 0.3)

eleArgs = ['tri31', 1.0, 'PlaneStress', 1]
ops.mesh('quad', 5, 4, 1, 4, 2, 3, sid, ndf, meshsize, *eleArgs)
```

(continues on next page)
if pid == 0:
    ops.fix(pid, 1, 1)
    ops.fix(np+pid+1, 1, 1)
if pid == np-1:
    ops.timeSeries('Linear', 1)
    ops.pattern('Plain', 1, 1)
    ops.load(np + pid + 2, 0.0, -1.0)

ops.constraints('Transformation')
ops.numberer('ParallelPlain')
ops.system('Mumps')
ops.test('NormDispIncr', 1e-6, 6)
ops.algorithm('Newton')
ops.integrator('LoadControl', 1.0)
ops.analysis('Static')
ops.stop()
ops.start()
ops.analyze(1)

if pid == np-1:
    print('Node', pid+1, ops.nodeDisp(pid+1))

ops.stop()

Parallel Parametric Study Example

1. The source code is shown below, which can be downloaded here.

2. Run the source code with 2 processors

   mpiexec -np 2 python paralleltruss2.py

   the outputs look like

   Processor 0
   Node 4 (E = 3000.0 ) Disp : [0.5300927771322836, -0.17789363846931766, -0.17789363846931766]  
   Processor 1

   Node 4 (E = 6000.0 ) Disp : [0.2650463885661418, -0.08894681923465883, -0.08894681923465883]

   Process 1 Terminating
   Process 0 Terminating

   The script is shown below

   import openseespy.opensees as ops
   pid = ops.getPID()
   np = ops.getNP()
   ops.start()
   if np != 2:

   (continues on next page)
    exit()

ops.model('basic', '-ndm', 2, '-ndf', 2)

if pid == 0:
    E = 3000.0
else:
    E = 6000.0

ops.uniaxialMaterial('Elastic', 1, E)

ops.node(1, 0.0, 0.0)
ops.node(2, 144.0, 0.0)
ops.node(3, 168.0, 0.0)
ops.node(4, 72.0, 96.0)

ops.fix(1, 1, 1)
ops.fix(2, 1, 1)
ops.fix(3, 1, 1)

ops.element('Truss', 1, 1, 4, 10.0, 1)
ops.timeSeries('Linear', 1)
ops.pattern('Plain', 1, 1)
ops.load(4, 100.0, -50.0)

ops.element('Truss', 2, 2, 4, 5.0, 1)
ops.element('Truss', 3, 3, 4, 5.0, 1)

ops.constraints('Transformation')
ops.numberer('ParallelPlain')
ops.system('Umfpack')
ops.test('NormDispIncr', 1e-6, 6)
ops.algorithm('Newton')
ops.integrator('LoadControl', 0.1)
ops.analysis('Static')

ops.analyze(10)

if pid == 0:
    print('Processor 0')
    print('Node 4 (E =', E, ') Disp :', ops.nodeDisp(4))

ops.barrier()

if pid == 1:
    print('Processor 1')
    print('Node 4 (E =', E, ') Disp :', ops.nodeDisp(4))

ops.stop()